EMERGENCY ACTION PLAN FOR PETIT LAKE DAM



STATE ID NO. 112-009-00462; NID NO. GA00685 Revision 3

PETIT LAKE – BIG CANOE PICKENS COUNTY, GEORGIA

Submitted to

BIG CANOE PROPERTY OWNER'S ASSOCIATION

> 10586 Big Canoe Jasper, Georgia 30143

> > Submitted by



engineers | scientists | innovators



TABLE OF CONTENTS

1.	INT]	RODUCTION1
	1.1	Statement of Purpose
	1.2	History of Document1
2.	NOT	TIFICATION FLOWCHART2
3.	PRO	JECT DESCRIPTION13
4.	EME	ERGENCY DETECTION, EVALUATION, AND CLASSIFICATION 16
5.	GEN	IERAL RESPONSIBILITIES UNDER THE PLAN20
	5.1	Dam Owner's Responsibilities:
	5.2	Responsibility for Notification:
	5.3	Responsibility for Evacuation:
	5.4	Responsibility for Duration, Security Termination, and Follow-Up:21
	5.5	EAP Coordinator Responsibility22
6.	PRE	PAREDNESS23
7	REF	ERENCES24
		LIST OF TABLES
Table	1	Dam Failure Notification of Big Canoe Critical Infrastructure Just Below the Dam
Table:	2	Dam Failure Notification of Big Canoe Residents and Other Facilities
Table	3	Dam Failure Notification of Properties Downstream of Big Canoe
Table -		Detection of Dam Failure and Classification of Emergency
Table.	5	Preplanned Operations for a Condition B (Level 2) Emergency
		LIST OF FIGURES
Figure	1	Dam Failure Notification Flowchart for Conditions B (Level 2) & C (Level 1)
		Emergencies
Figure		Dam Failure Notification Flowchart for Condition A (Level 3) Emergencies
Figure 3		Evacuation Routes Prior to Dam Failure

LIST OF APPENDICES

Appendix A	Definitions
Appendix B	Inundation Maps and Calculations
Appendix C	Dam Features and Photographs
Appendix D	EAP Distribution and Acceptance
Appendix E	EAP Review, Revision, and Testing Documentation
Appendix F	Locally Available Resources

1. INTRODUCTION

1.1 Statement of Purpose

The purpose of the Emergency Action Plan (EAP) is to establish procedures for warning, evacuating and protecting persons and property which would be endangered in the event of a failure of the Petit Lake Dam, in Big Canoe, Georgia, as well as taking timely action to notify the appropriate emergency management agencies, law enforcement bodies, and/or governing officials of possible, impending, or actual failure of the dam.

1.2 History of Document

The EAP for Petit Lake Dam was originally created and revised by another consultant (Jordan, Jones, and Goulding) in 1998 and 2007, respectively. A summary of known dates of revision of this EAP is provided in Appendix E and future revisions and modifications should be added to this table.

This EAP was revised by Geosyntec Consultants, Inc. (Geosyntec), and relies upon information and analyses developed by others as identified in the Plan. Geosyntec has developed this plan in good faith for use by the Big Canoe Property Owner's Association (POA) and the Emergency Management Agencies responsible for Petit Lake Dam. This plan may not be relied upon by other parties and may not be modified without the express written permission of Geosyntec.

Geosyntec updated the EAP for Petit Lake Dam based on guidance from the Georgia Safe Dams Program document *Engineer Guidelines* (2015), and templates referred to in *Engineer Guidelines* created by the North Carolina Department of Environmental Quality (2016) and the Association of State Dam Safety Officials (2010).

The inundation mapping and calculations provided in Appendix B were original created by Jordan, Jones, and Goulding and have not been updated by Geosyntec. The Figures provided of inundation maps and Big Canoe property parcels, and the names and addresses provided in Tables 1 and 2 were provided by Big Canoe. The property parcels and addresses for properties downstream of Big Canoe in Table 3 were provided by the Pickens County Emergency Management Agency (EMA).

2. NOTIFICATION FLOWCHART

The Notification Flowcharts (Figure 1 and 2) summarize the following information which is applicable during an impending or imminent failure of the Petit Lake Dam:

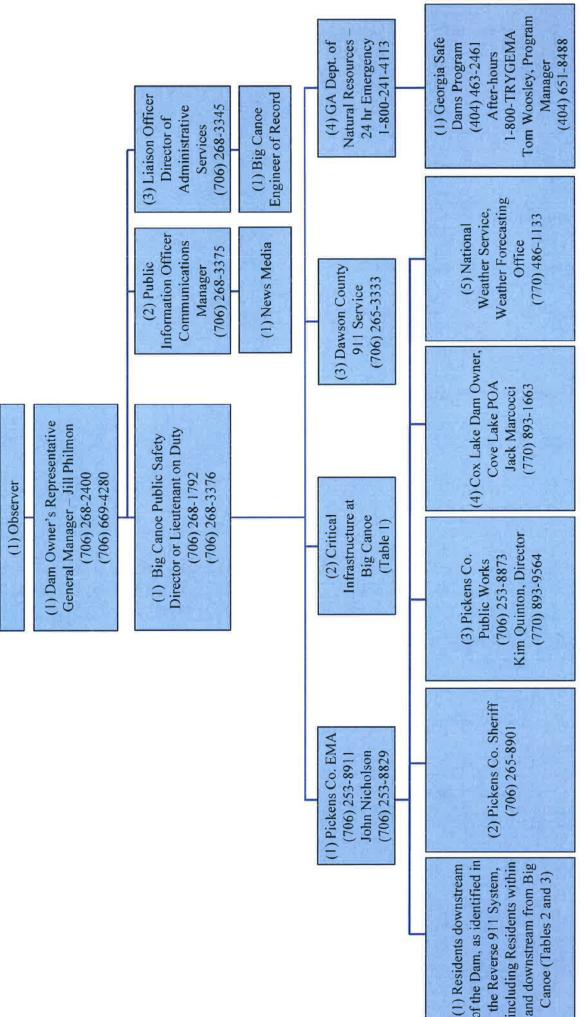
- Who is responsible for notifying Petit Lake Dam owner representative(s) and/or emergency management official(s);
- Who is to be notified; and
- What is the priority order in which individuals are to be notified.

All residents and employees of Big Canoe can and should be observers of unusual events at Petit Lake Dam. This observer group will be educated through the local newsletter regarding what are symptoms of impeding or imminent failure, and who at the POA should be contacted to initiate the Notification Process.

The Dam Owner's Representative or his designee is responsible for initiating the Notification Process. The Dam Owner's Representative or his designee will verify the condition of Petit Lake Dam which the observer has identified and initiate the notification. The Dam Failure Notification Flow Charts (Figures 1 and 2 and Tables 1, 2, and 3) identify the residents, emergency management agencies, government agencies and Big Canoe Property Owners Association that should be contacted immediately and in what order. Subsequent contacts by each of these individuals are shown in priority order.

Figure 1: retit Lake Dam

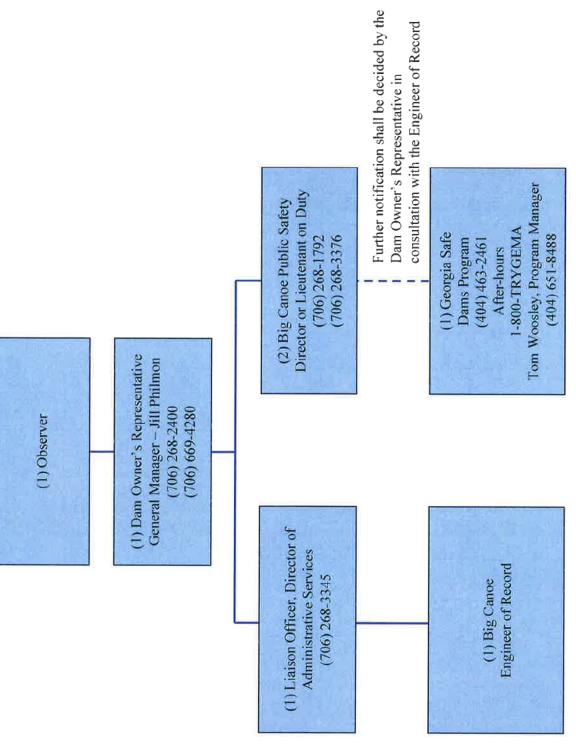
Dam Failure Notification Flowchart for Conditions C (Level 1) & B (Level 2) Emergencies



(#) = Order of Priority of Notification

Note: Flowchart and contact information provided by Big Canoe Property Owner's Association on 9, 19 and 19 June 2017.

Dam Failure Notification Flowchart for Condition A (Level 3) Emergencies



(#) = Order of Priority of Notification

Note: Flowchart and contact information provided by Big Canoe Property Owner's Association.

TABLE 1⁽¹⁾ DAM FAILURE NOTIFICATION OF BIG CANOE CRITICAL INFRASTRUCTURE JUST BELOW THE DAM

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

FACILITY	ADDRESS/LOT NUMBER	PHONE NUMBER
Utilities Office	Highland Trail/Wolfscratch Drive	706/268-3400
Swim Club	Wolfscratch Drive	706/268-3317
Fitness Center	Wolfscratch Drive	706/268-3441
Chapel	Wolfscratch Drive	706/268-3203
Tennis Center	Wolfscratch Drive	706/268-3367
IGA Express	Wolfscratch	706/268-3326
Lakewatch Village	Golf Club Drive	706/268-3326
Clubhouse	Golf Club Drive	706/268-3326
Fire Station #3	Wolfscratch Drive	706/268-1792
Duffers	Golf Club Drive	706/268-3273
Golf Shop	Golf Club Drive	706/268-3323
Cart Barn	Golf Club Drive	706/268-3323

Note:

⁽¹⁾ Facilities and contact information provided by Big Canoe Property Owner's Association on 26 April 2017 based on Inundation Mapping conducted by Jones, Jordan, and Goulding

TABLE $2^{(1)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	Phone Number 3
2569	Wilderness Parkway	Campbell, Douglas & Shannon						
5888	Wilderness Parkway	Williams, Sally						
87	Tanager Way	Farias, Fran & Phillip						
216	Indigo Bunting Trail	Turner, Jeffrey						
254	Indigo Bunting Trail	Harper, David						
270	Indigo Bunting Trail	Risoldi, Ciro						
288	Indigo Bunting Trail	Honeycutt, Boyce						
289	Indigo Bunting Trail	Lacey, Claude						
255	Indigo Bunting Trail	Rozeboom, Loren						
0	Bluebird Court	Reynolds. David & Patsy						
18	Bluebird Court	Reynolds, David & Patsy						
172	Canada Geese Point	Pepper, Ralph						
230	Canada Geese Point	Ebbeskotte, Carolyn						
205	Canada Geese Point	May, Randy						
121	Goldfinch Point	Perkel, Karen and Ralph						
36	Petit Ridge Drive	Huffstetler, Mark & Rachel						
84	Flycatcher Point	Clough, Gerald & Anne						
190	Swallow Point	Leff, Steven						
181	Swallow Point	Malgeri, Linda						
115	Swallow Point	Arthurs. Lee						
95	Swallow Point	Kelly, David and Laura						
0	Swallow Point	Bradley Family Trust						
18	Thrush Turn	Coleman, William						
56	Thrush Turn	Reisinger, Ralf & Daniela						
74	Thrush Turn	#47 Holding LLC						
40	Holly Point	Young, Alden						
287	Willow Drive	Roberson, Ellen						

TABLE $2^{(1)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	Phone Numb
135	Chestnut Rise	Collins, Norma	N.				Si u	
135	Chestnut Rise	Abernathy, MaryAnn						
135	Chestnut Rise	Scott, Norman & Jonny						
145	Chestnut Rise	Milles, Purtell						
145	Chestnut Rise	Faust, Richard & Wendy						
145	Chestnut Rise	Moser, Nancy						
145	Chestnut Rise	Ferrini, Joseph						
155	Chestnut Rise	House, Jerry						
155	Chestnut Rise	Eaves, Dennis						
155	Chestnut Rise	Doll, Herman						
20	Sconti Ridge	Manning, Joyce						
20	Sconti Ridge	Parks, William						
20	Sconti Ridge	Collier, Daryl & Linda						
20	Sconti Ridge	Hanson/Shaw						
34	Sconti Ridge	Brown, Donald & MaryAnn						
34	Sconti Ridge	Hayes, Jr., Mark						
34	Sconti Ridge	Young, Shelia						
34	Sconti Ridge	Verell, William & Susan						
48	Sconti Ridge	Wagner, William & Patricia						
48	Sconti Ridge	Sherman, Karen						
48	Sconti Ridge	Hacker & Ramsey						
86	Sconti Ridge	Campbell,Mark						
86	Sconti Ridge	McCann, Debra and Ronald						
86	Sconti Ridge	Captan, Mohamed & Marcia						
86	Sconti Ridge	Wood, Barbara						
116	Sconti Ridge	Mullins, Jr., B.A.						
116	Disharoon	Floyd, Greg						

TABLE $2^{(1)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number I	Phone Number 2	Phone Number 3
116	Sconti Ridge	Capone & Rutledge						
140	Sconti Ridge	Riley, Ronald						
40	Treetop Lane	Chait, Susan						
104	Treetop Lane	Paynter, Fredrick						
309	Buckskull Hollow Drive	Brown, Carolyn						
301	Buckskull Hollow Drive	Warren, Claude						
57	Buckskull Hollow Drive	Henil & Brennan						
41	Buckskull Hollow Drive	Dollar, Dari & David						
11	Buckskull Hollow Drive	Bassett, Robert & Jolyne						
22	Buckskull Hollow	McCardel, James & Dora Jean						
26	Buckskull Hollow	Davis, James						
84	Highland Court	Nicholson						
110	Highland Court	Lundstrom						
0	Highland Court	BC LLC						
196	Sconti Knoll Drive	Cody, Gail						
200	Sconti Knoll Drive	Horsley, Rosemarie						
193	Sconti Knoll Drive	Kowalski, George						
191	Sconti Knoll Drive	Tanner, Caroll						
60	Sconti Ridge	Hackney						
86	Sconti Point	Crawford, Howard Wayne						
0	Sconti Point	Rhodus, Gary and Marsha						
0	Sconti Point	Bhold, Adriana						
0	Sconti Point	Bhold, Adriana						
62	Sconti View Drive	Slade, David						
34	Sconti Ct	Landreth, Porter						
21	Chestnut Knoll Point	Rees, James						
5	Chestnut Knoll Point	Moon, Jacqueline						

TABLE $2^{(l)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	
7	Chestnut Rise	Bowers, Joanne						I
9	Chestnut Knoll Point	Leardini, Paolo						
П	Chestnut Knoll Point	Gurusamy & Subbannan						
31	Chestnut Knoll	Genone, Terry and Loretta						
41	Chestnut Knoll Point	Nelson, Ronald						
28	Tanager Way	Beraha, Dan & Ellen						
226	Wolfscratch Village Cir.	Chapel						
51	Clubhouse Drive	Apgar, Cindy & Dan						
79	Clubhouse Drive	Fargason, Charles & Gail						
65	Clubhouse Drive	Dubose Calvin Jr. & Mary ann						
1944	Wildemess PKWY	Mcclure, William & Nancy	**					
	Clubhouse Drive	Bowers-Mcclure, Mark & Joanne						
29	Clubhouse Drive	Bell, Pamela & Кепу						
1944	Wilderness Parkway	Hecht, Bruce and Rosemary						
1944	Wilderness Parkway	Tillery, Roger & Patty						
1944	Wilderness Parkway	Prince, Bruce & Judith						
1944	Wilderness Parkway	Chu, Kathleen and Norman						
1944	Wilderness Parkway	Gregg, Fredrick & Sue						
1944	Wilderness Parkway	Kadechuk, Barry & Linda						
1944	Wilderness Parkway	Delashmutt, R. E.						
1944	Wilderness Parkway	Hyland, Keith and Lauren	T					
1944	Wilderness Parkway	Lehmberg, Bill & Peggy						
1944	Wilderness Parkway	Zamborsky, Joe and Lis						
1944	Wilderness Parkway	Salcedo, Cesar & Anna						
1944	Wilderness Parkway	Dicus, Patti & Frank						
1944	Wilderness Parkway	Farias, Fran & Phillip	1					
315	Choctaw Pass	Anne Bopp						

TABLE $\mathbf{2}^{(l)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	Phone Number 3
333	Choctaw Pass	Auchter, Barbara and Thome						
126	Sinti trail	Tiller, Micheal D. & Valerie						
140	Sinti Trail	Albritton, Jeffery						
190	Sinti trail	Fellows, Henry & Pamela						
204	Sinti trail	Heller, Steve & Carol						
220	Sinti trail	Morris, William and Margie						
75	Twin Creek Drive	Back, Stephen and Susan						
101	Twin Creek Drive	Psalmond, Phillip and Tami						
115	Twin Creek Drive	Clark, Brandon and Monica						
131	Twin Creek Drive	Boudreau, Bill and Cindy						
151	Twin Creek Drive	Creel, Lynda and Micheal						
185	Twin Creek Drive	Santoro, Ann and Peter						
104	Twin Creek Drive	Willett, Roy & Weslie						
80	Twin Creek Drive	Cetrino, Carol/Piccirillo, Mark						

Note:

⁽¹⁾ Contact information provided by Big Canoe Property Owner's Association on 25 April 2017

TABLE $\mathbf{3}^{(1)}$ DAM FAILURE NOTIFICATION OF PROPERTIES DOWNSTREAM OF BIG CANOE

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Parcel ID
295	Blackwell Creek Way	049A 064
121	Chastain Court	049 024
59	Clint Court	050 022
916	Cove Lake Drive	049 090 112
976	Cove Lake Drive	049 090 113
968	Cove Lake Drive	049 090 114
903	Cove Lake Drive	049 090 115
141	Cove Lake Drive	049 090 116
779	Cove Lake Drive	049 090 117
661	Cove Lake Drive	049 090 118
543	Cove Lake Drive	049 090 119
112	Cove Lake Drive	049 090 120
459	Cove Lake Drive	049 090 121
391	Cove Lake Drive	049 090 122
351	Cove Lake Drive	049 090 123
303	Cove Lake Drive	049 090 124
249	Cove Lake Drive	049 090 125
8822	Cove Road	049 090
385	Del Moore Road	049 021
11117	Highway 53 East	050 023
10322	Highway 53 East	050 029
100	Hunters Trace	049A 003
1065	Hunters Trace	049A 031
1109	Hunters Trace	049A 032

Note

⁽¹⁾ Property information provided by the Pickens County EMA on 21 February 2018. Personal names and contact information for residents outside of Big Canoe are not accessible outside of the Pickens County EMA CodeRED system.

TABLE 3⁽¹⁾ DAM FAILURE NOTIFICATION OF PROPERTIES DOWNSTREAM OF BIG CANOE

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Parcel ID		
30	Justice Way	049 030		
43	Justice Way	049 031 001		
16	Limestone Trail	050 028 001		
44	MacKenzie Lane	049 089		
533	Old Mill White Road	049 013		
1493	Old Mill White Road	049 017		
1543	Old Mill White Road	049 020		
1985	Old Mill White Road	050 026		
2192	Old Mill White Road	050 027		
2164	Old Mill White Road	050 027 002		
40	Overlook Court	049 090 128		
27	Overlook Court	049 090 129		
520	Partain Road	049 119		
638	Partain Road	049 121		
54	Pendley Woods Road	049 124		
24	Pony Lane	049A 017		
584	Redbud Pass	049A 051		
451	Redbud Pass	049A 053		
874	Sparkling Springs Road	049 019 001 & 049 019 002		
189	Timber Creek	049 089 008		
87	Trotters Lane	049A 001		
125	Trotters Lane	SEWER PLANT		

Note:

⁽¹⁾ Property information provided by the Pickens County EMA on 21 February 2018. Personal names and contact information for residents outside of Big Canoe are not accessible outside of the Pickens County EMA CodeRED system.

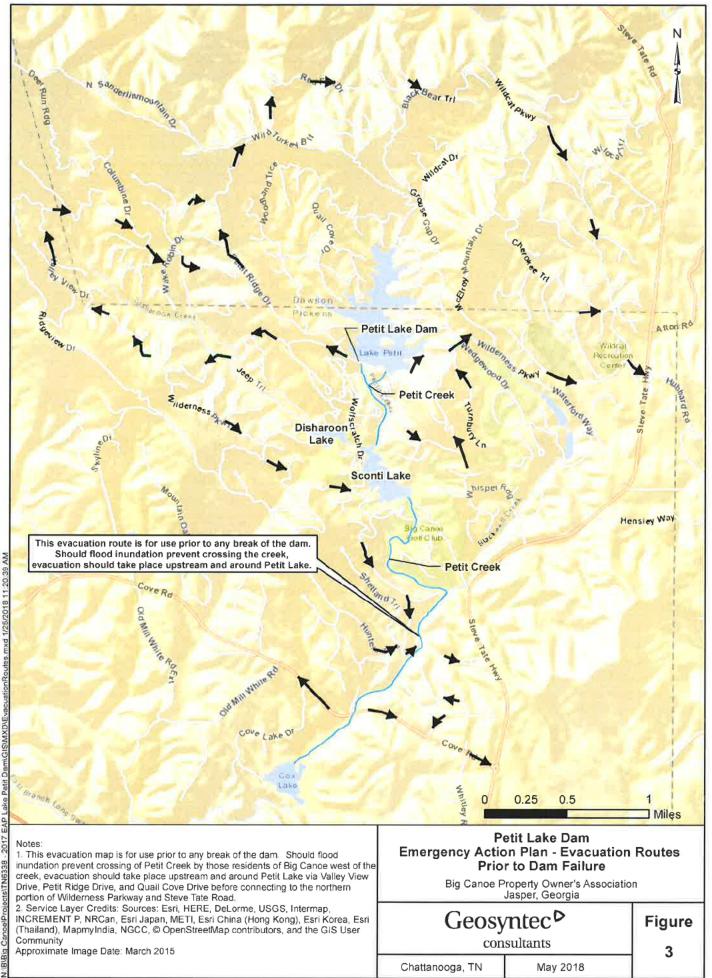
3. PROJECT DESCRIPTION

Petit Lake Dam is located within the Big Canoe Development on Petit Creek about 5.8 miles upstream of Marble Hill, in Pickens County, north central Georgia. The reservoir formed by the dam has a surface area of 104 acres at a normal pool elevation of 1635.0 and extends up Petit Creek approximately 0.70 miles. The total storage for the reservoir is approximately 7,500 ac-ft, according to the Georgia Safe Dams database. The drainage area upstream of Big Canoe Dam is 1.53 square miles. The topography around the dam consists of very steep, wooded, mountainous foothills.

Petit Lake Dam is listed in the Georgia State Safe Dams Program and the National Inventory of Dams under the following identification numbers, respectively: 112-009-00462 and GA00685. The earth dam has a maximum height of 126 feet according to the Georgia Safe Dams database, a length of 880 feet, and a top width of 35 feet. The dam has a 15-foot wide concrete cascading channel spillway on the east side of the earth dam's abutment. The spillway discharge is controlled by a concrete crest underneath a bridge located on the roadway (i.e. Wilderness Parkway) running along the crest of the dam (See Dam Features Map provided in Appendix C). The dam has a 36-inch low level drain which is the only permanent means of lowering the lake level short of structural excavation of an abutment area or the use of temporary pumps or siphons. The low-level drain can be operated by a sluice gate operator located near the center top of the dam. Lake Sconti, owned by Big Canoe, is located some 1.0 miles downstream of Petit Lake. This earth dam has a maximum height 40 feet, a length of 190 feet and a top width of 20 feet. The dam has a 10-foot wide concrete spillway on the eastern abutment. Cox Lake Dam is located some 3.5 miles downstream from Petit Lake Dam south of Cove Road, and is not owned or operated by Big Canoe.

Big Canoe has two permanent roadway access points: (1) the main entrance off Steve Tate Highway is Wilderness Parkway, which crosses Petit Creek some 2.3 miles downstream of Petit Lake Dam, and (2) the secondary entrance off Steve Tate Highway (also Wilderness Parkway) which crosses over the top of Petit Lake Dam, as well as connecting to the main entrance road via a road at the base of Petit Lake Dam (See Inundation Map provided in Appendix B). A third access route utilizes a gravel maintenance road adjacent to the Sconti Golf Clubhouse. These three (3) connections to Steve Tate Highway are the only evacuation routes from the impacted areas of Big Canoe. The use of these evacuation routes will be controlled by the Big Canoe Department of Public Safety to prevent endangerment of evacuees in case of an imminent failure. As shown in Figure 3, two (2) of these evacuation routes would be cut off west of Petit Creek in case of a dam failure of Petit Lake Dam, and evacuation would need to take place via roads upstream of Petit Lake (i.e., Valley View Drive, Petit Ridge Drive, and Quail Cove Drive) and connecting back to the northern intersection of Wilderness Parkway and Steve Tate Road. A small portion of Steve Tate Highway could also be inundated in the event of a major dam failure.

An additional road downstream and outside of the Big Canoe Development would be impacted by failure of Petit Lake Dam. Cove Road is some 3.0 miles downstream of the dam. Petit Creek flows through three (3) 10-foot x 10-foot box culverts under Cove Road.



4. EMERGENCY DETECTION, EVALUATION, AND CLASSIFICATION

Detection and Evaluation of Petit Lake Dam Failure:

Once the observer notifies the Dam Owner's Representative or his designee about a condition of concern, the Dam Owner's Representative or his designee will immediately proceed to Petit Lake Dam and personally inspect the dam for any signs of failure condition. The Dam Owner's Representative or his designee will evaluate the severity of the condition of concern utilizing the guidelines in Table 4 to determine the appropriate Emergency Action Classification. Petit Lake Dam Failure Emergencies will be classified according to their severity and urgency. For the purposes of this EAP, three emergency classifications are provided. Conditions C, B, and A are consistent with other Big Canoe action plans, while Levels 1, 2, and 3 are consistent with Georgia Department of Natural Resources (DNR) Safe Dams Program. For clarity, both nomenclatures are provided throughout this EAP and are defined as follows:

Condition C (Level 1): Failure is imminent or has occurred

This is a situation where a failure either has occurred, is occurring, or obviously is just about to occur. It is impossible to determine how long it will take for a failure to occur or for a complete breach to occur once failure begins. Therefore, once the Dam Owner's Representative determines that there is no longer time available to attempt corrective measures to prevent failure, the "failure is imminent or has occurred" warning, as suggested in Section 5.1, should be issued. Emergency management officials will interpret the phrase "failure is imminent" to mean that the dam is failing. For evacuation purposes, "failure is imminent" and "failure has occurred" should be interpreted as the same condition.

Condition B (Level 2): Potential failure situation is developing

This is a situation where a failure may eventually occur, but preplanned actions taken during certain events (such as major floods, earthquakes, evidence of piping, etc.) may alleviate failure. Recommended methods and steps to alleviate the risk of failure are provided in Table 5. Even if failure is inevitable, more time is generally available than in a "failure has occurred" situation to issue warnings and/or take preparedness actions. When a dam safety situation is observed that may lead to a failure if left unattended, but there is no immediate danger, the Dam Owner's Representative will issue a warning that a "potential failure situation is developing". The Dam Owner's Representative will assess the situation and determine the urgency of the emergency situation. Based on the Dam Owner's Representative's assessment, the first series of notifications should be made and it is up to the Pickens County EMA to determine the subsequent course of action to follow.

NOTE: It should be remembered that it may be appropriate to declare a Condition C (Level 1). However, there should be a smooth transition from Condition B (Level 2) to Condition C (Level 1) when using both conditions.

Condition A (Level 3): Unusual, slowly developing event

This is a condition where a situation is developing, but has not yet threatened the operation or the structural integrity of the dam. The situation should be investigated by the Dam Owner's Representative for development of a potential or imminent dam failure situation. The Dam Owner's Representative will assess the situation and determine a path forward approach. Warnings shall not be issued unless the situation develops into a Condition B (Level 2) situation.

To assist the Emergency Management Agency in selecting their appropriate course of action and to provide a proper transition from Condition A (Level 3) to Condition B (Level 2) or Condition B (Level 2) to Condition C (Level 1), the Big Canoe Public Safety Director will clearly communicate the situation to the EMA. For Condition A (Level 3) and Condition B (Level 2) situations, the Big Canoe Public Safety Director will place the first series of notifications on initial alert, and provide periodic updates on the situation as it develops so that the EMA can assess when they should implement their evacuation procedures. For example, the Dam Owner's Representative will issue an initial warning and periodic updates on the lake level as it rises during flooding conditions and eventually overtops the dam. As the lake rises, a "potential failure situation is developing" warning should be issued with periodic updates on how much time is available before overtopping occurs. Once the dam overtops, a "failure is imminent or has occurred" warning should be issued, as suggested in Section 5.1.

Preplanned actions will be initiated once a Condition B (Level 2) Emergency has been declared and the initial notifications have been completed. The preplanned operations that should be undertaken are shown in Table 5.

TABLE 4(1) DETECTION OF DAM FAILURE AND CLASSIFICATION OF EMERGENCY

EVENT	DAM FAILURE MECHANISM	EVALUATION OF FAILURE	CLASSIFICATION OF FAILURE MECHANISM(2)
Unexpected Failure	Unknown	Dam unexpectedly and without warning begins to fail	Condition C (Level 1)
Major Flood/		Erosion and removal of the road and embankment occurring	Condition C (Level 1)
Embankment Overtopping	Overtopping of dam	Flood pool rapidly approaching top of dam and embankment still intact	Condition B (Level 2)
Overtopping		Settlement of more than a few inches	
		Slope movement larger than the size of a car	Condition C (Level 1)
		Flowing water from downstream face of dam	00.00.00.00.00.77
Global	Settlement of dam crest	Settlement of less than a few inches	
Earthquake or	Slope movement	Slope movement of less that the size of a car	0 12 10
Seismic Activity	Evidence of seepage or piping	Wet areas on downstream face of dam that continue to increase in size and intensity of flow	Condition B (Level 2)
		Measurable earthquake felt or reported near the dam and dam appears to be stable.	Condition A (Level 3)
		Settlement of more than a few inches	Condition C (Lovel 1)
Embankment	Settlement of dam crest	Slope movement larger than the size of a car	Condition C (Level 1)
Movement	Settlement of dam crest Slope movement	Settlement of less than a few inches	Condition B (Level 2)
		Slope movement of less than the size of a car	Condition D (Level 2)
		New cracks in the embankment greater than 1/4-inch wide without seepage	Condition A (Level 3)
Embankment		Flowing water greater than 10 gallons per minute from downstream face of dam	Condition C (Level 1)
Seepage	Evidence of seepage of piping	Wet areas with cloudy discharge on downstream face of dam that continue to increase in size and intensity of flow	Condition B (Level 2)
		New seepage areas in or near the dam, water flowing clear	Condition A (Level 3)
	0.70	Spillway overflowing with an advancing head cut that is threatening the control section or that is already flooding people downstream	
Spillway Flow	Spillway overflow Spillway erosion	Spillway overflowing with active gully erosion	Condition B (Level 2)
		Spillway overflowing with no active erosion	Condition A (Level 3)
		Normal flow with erosion under, beneath, or at edges of the spillway	Condition A (Level 3)
		Rapidly enlarging sinkhole on dam or appurtenances	Condition C (Level 1)
Sinkholes	 Observed sinkhole 	Observation of new sinkhole in reservoir area or on embankment	Condition B (Level 2)
		Observation of sinkhole downgradient of the dam	Condition A (Level 3)
		Increase in piezometer readings of more than 10 feet and flowing water from downstream face of dam	Condition C (Level 1)
		Rapid decrease in lake level and flowing water from downstream face of dam	Condition C (Level 1)
Routine Instrumentation Readings	 Significant change in piezometer readings Rapid decrease in lake level 	Increase in piezometer readings of more than 10 feet and no flowing water from downstream face of dam	Condition P. (Lovel 2)
7701011185	rapia doctate in falle te te	Rapid decrease in lake level with no apparent reason and no flowing water from downstream face of dam	Condition B (Level 2)
		Piezometer readings vary beyond predetermined values and no flowing water from downstream face of dam	Condition A (Level 3)
		Detonated bomb that has results in damage to the dam or appurtenances	Condition C (Level 1)
Security Threat	Bomb threat	Verified bomb threat that, if carried out, could result in damage to the dam or appurtenances with no impacts to the functioning of the dam	Condition B (Level 2)
		Reported bomb threat, unverified	Condition C (Level 1)
		Damage to dam or appurtenances that has resulted in uncontrolled water release	Condition A (Level 3)
Sabotage/ Vandalism	 Damage to dam or appurtenances 	Damage to dam or appurtenances that has resulted in seepage flow	Condition B (Level 2)
Vandalism	appurtenances	Damage or modification to the dam or appurtenances with no impacts to the	0 12 00 11
		function of the dam	Condition C (Level 1)

Notes:

(1) This table was based on a similar table from the North Carolina Department of Environmental Quality, 2016.

(2) This table was based on a similar table from the North Carolina Department of Environmental Quality, 2016. (2) Conditions C. B. and A are consistent with other Big Canoe action plans while Levels 1, 2, and 3 are consistent with Georgia Department of Natural Resources (GA DNR) Safe Dams Program. For clarity, both nomenclatures are provided.

TABLE 5 PREPLANNED OPERATIONS FOR CONDITION B (LEVEL 2) EMERGENCY

EVENT	IMPENDING DAM	PREPLANNED OPERATIONS IN PRIORITY ORDER
EVENT	FAILURE MECHANISM	
		Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.
Major Flood/Embankment Overtopping	Overtopping of dam	 Open the bypass valve on the water supply line to the water treatment plant (if available). If lake levels continue to rise dangerously close to top of the dam crest, excavate emergency channel in abutment area adjacent to concrete chute spillway. Also bring in emergency pumps and discharge outflow to spillway or directly into Petit Creek.
		Do not excavate channel on top of dam, or discharge pump outflow on face of dam
Earthquake or Seismic Activity	Slope Failure	 Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water treatment plant (if available). Monitor piezometer readings daily and plot readings to identify significant changes in readings. Also record lake levels. Survey elevation along top of dam daily at 50' intervals. Plot elevations of
		each point to identify significant changes in readings. 5. Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).
Embankment Movement, Embankment Seepage, Spillway Flow, or Sinkhole	Slope Failure	 Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water treatment plant (if available). Monitor piezometer readings daily and plot reading to identify significant changes in readings. Also record lake levels. Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings. Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).
Routine Instrumentation	Slope Failure	 Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water. Monitor piezometer readings daily and plot reading to identify significant changes in readings. Also record lake levels. Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings. Obtain input from Georgia Safe Dams program or Owner's Engineer as to emergency repairs to be constructed (if any).
Security Threat or Sabotage	Slope Failure	Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water treatment plant (if available).

5. GENERAL RESPONSIBILITIES UNDER THE PLAN

The EAP contains preplanned actions designed to prevent a failure to dam structures or to minimize the impact of a dam failure on life and property. It establishes and documents procedures for notifying state and local emergency management agencies, law enforcement bodies, and downstream residents affected by a dam failure. All responsible parties responsible for implementing the EAP shall verify their responsibilities with their signature in Appendix D.

5.1 <u>Dam Owner's Responsibilities:</u>

The person responsible for performing the tasks required under the EAP is the Dam Owner's Representative and the Public Safety Director. If the Public Safety Director is absent, the responsible person will be the Public Safety Director Lieutenant on duty at that time.

The duties of the Public Safety Director under the EAP should be as follows:

- 1. The responsibility for making the initial assessment of the emergency Condition C (Level 1), Condition B (Level 2), or Condition A (Level 3).
- 2. The responsibility for initiating the notification process (see Notification Flowchart). All warning messages should be brief and to the point. For example, for a Condition C (Level 1) Emergency, the message should be as follows: "This is Public Safety Director at Big Canoe. I have verified overtopping of Petit Lake dam and believe dam failure is imminent. I request that you immediately activate the Petit Lake Dam Emergency Action Plan. This is an actual emergency and not a drill." Note that a warning message shall not be issued for a Condition A (Level 1) emergency.
- 3. The responsibility for initiating the preplanned actions in case of a Condition B (Level 2) emergency.

5.2 Responsibility for Notification:

For a Condition C (Level 1) or Condition B (Level 2) emergency, the Dam Owner's Representative, the General Manager of the Big Canoe POA, is responsible for contacting the Public Safety Director at Big Canoe, the Public Information Officer Communications Manager, and the Liaison Officer, Director of Administrative Services.

The Public Safety Director at Big Canoe is responsible for notifying Pickens County EMA, Critical Infrastructure at Big Canoe (Table 1), Dawson County 911 Services, and the Georgia DNR 24-Hour Emergency Contact in the event of an emergency. If time allows, he should seek advice and assistance.

The Public Information Officer, Communications Manager is responsible for notifying the News Media (including radio and television media). The Liaison Officer, Director of Administrative Services is responsible for notifying Big Canoe's Engineer of Record.

The Pickens County EMA is responsible for notification of the residents below the dam as identified in the Reverse 911 System, including: (i) Big Canoe residents (Table 2) and residents downstream from Big Canoe (Table 3), (ii) Pickens County Sheriff's Department, (iii) Pickens County Public Works, (iv) the Cox Lake Dam Owner (Cove Lake Property Owner's Association, Inc.), and the (v) National Weather Service Weather Forecasting Office.

The Georgia DNR 24 Hour Emergency Contact is responsible for notification of the Georgia Safe Dams Program.

For a Condition A (Level 3) emergency, the Dam Owner's Representative, the General Manager of the Big Canoe POA, is responsible for contacting the Public Safety Director at Big Canoe and the Liaison Officer, Director of Administrative Services.

The Liaison Officer, Director of Administrative Services is responsible for contacting the Big Canoe Engineer of Record. The Public Safety Director at Big Canoe is responsible for contacting the Georgia Safe Dams Program, if required by the Dam Owner's Representative.

5.3 Responsibility for Evacuation:

Warning and evacuation planning are the responsibilities of the Pickens County EMA who have the statutory obligation. Under the EAP, the Big Canoe Public Safety Director is responsible for notifying the Pickens County EMA when a failure is imminent or has occurred (Condition C or Level 1), or a potential failure situation is developing (Condition B or Level 2). Big Canoe will not assume the responsibility of government entities for evacuation of people. This procedure should be coordinated with the appropriate public officials prior to an emergency situation developing.

5.4 Responsibility for Duration, Security Termination, and Follow-Up:

The Pickens County EMA is responsible for monitoring the situation at the dam and keeping local authorities informed of developing conditions at the dam from the time that an emergency starts until the emergency has been terminated. Security measures at the dam should be implemented by the Pickens County Sheriff's Department.

The Pickens County EMA is responsible for declaring that the emergency at the dam is terminated.

A follow-up evaluation after an emergency by all participants will be conducted. The Big Canoe POA is responsible for coordinating this effort and generating a written report which will detail the extent to which the EAP was followed, and what changes, additions or improvements need to be made to the EAP.

5.5 EAP Coordinator Responsibility

The Dam Owner's Representative or his designee will be the designated EAP coordinator who will be responsible for EAP related activities, including preparing revisions to the EAP, establishing training activities, coordinating EAP exercise, etc. They will also be the EAP contact if any of the involved parties have questions about the plan. The EAP will be reviewed annually with contacts, phone numbers, verified for accuracy. Documentation of the annual review is provided in Appendix E. As infrastructure and homes are built in the inundation map hazard areas, contact information (i.e., resident's names, addresses, and phone numbers) will be added to Tables 1, 2, and 3, as appropriate and available.

6. PREPAREDNESS

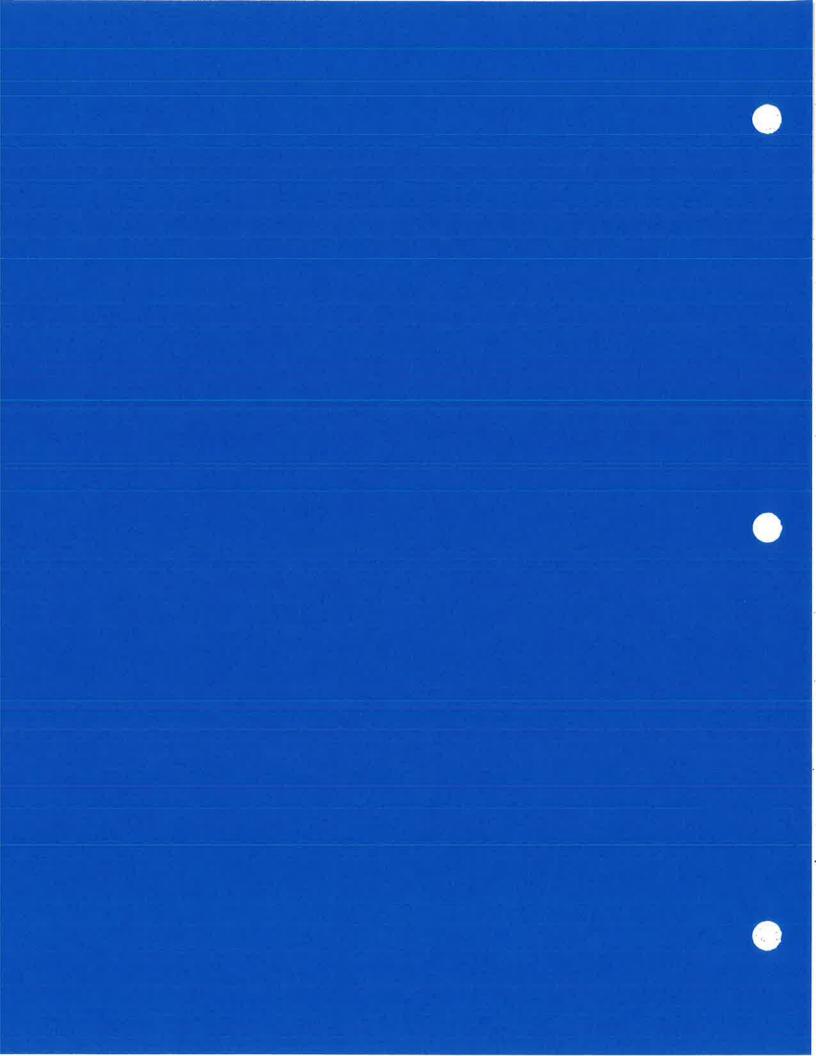
The following actions should be taken in preparation of an emergency:

- Initiation of a dam inspection and surveillance program. The dam should be formally inspected annually, at minimum, and readings from the dam instrumentation should be taken and interpreted quarterly, at minimum. During quarterly readings, basic observations of the dam can also be made.
- Preparation of a systematic warning and evacuation plan. A formal notification system should be coordinated with residents and businesses in the inundation area, and evacuation routes should be discussed/provided to those residents.
- Preparations should be made for evacuation on weekends, weekdays, and any time of
 day or night, including holidays. The emergency responders should have backup ways
 of communicating and a way to respond to the emergency in case of power outages.
- Community awareness programs for emergency response procedures. The community should be made aware of the possible emergencies and procedures associated with the possible failure of the dam.
- Establishment of emergency flood operating procedures.
- Organization of equipment, labor, and materials for use in emergency situations. A
 list of locally available resources that could be used in the event of an emergency are
 provided in Appendix F. These businesses can supply pumps, power generators, divers
 for inspections, and materials for temporary repair, depending on the emergency. Also
 the Pickens County EMA has provided a list of equipment that may be available during
 a dam failure.
- Conduct emergency exercises. At minimum, an orientation meeting should be held with key people so that those playing key roles and those having responsibilities outlined in this EAP can become familiar with it. A drill or tabletop exercise can be held, and coordinated with the Pickens County EMA. Documentation of reviews, tests, and revisions of this EAP should be documented in the forms provided in Appendix E.

7. REFERENCES

- Association of State Dam Safety Officials [2010]. "EAPWG Final EAP Template," available online at http://www.damsafety.org/community/owners/?p=3a95437d-1876-46d6-843b-d65d45beb46a, accessed March 2017.
- Georgia Safe Dams Program [2015]. "Engineer Guidelines, 2015 Edition," Georgia Department of Natural Resources, Version 4.0.
- Jordan, Jones, and Goulding [1998]. "Emergency Action Plan for Dam Failure, Petit Lake Dam, Big Canoe, Georgia," prepared for Big Canoe Property Owner's Association, September.
- Jordan, Jones, and Goulding [2007]. "Emergency Action Plan for Dam Failure, Petit Lake Dam, Big Canoe, Georgia," prepared for Big Canoe Property Owner's Association, April.
- North Carolina Department of Environmental Quality [2016]. "Guidelines for Owner Completion of the Emergency Action Plan (EAP) Template," Division of Energy, Mineral and Land Resources, available online at https://deq.nc.gov/about/divisions/energy-mineral-land-permits/dam-safety, accessed March 2017.

APPENDIX A DEFINITIONS

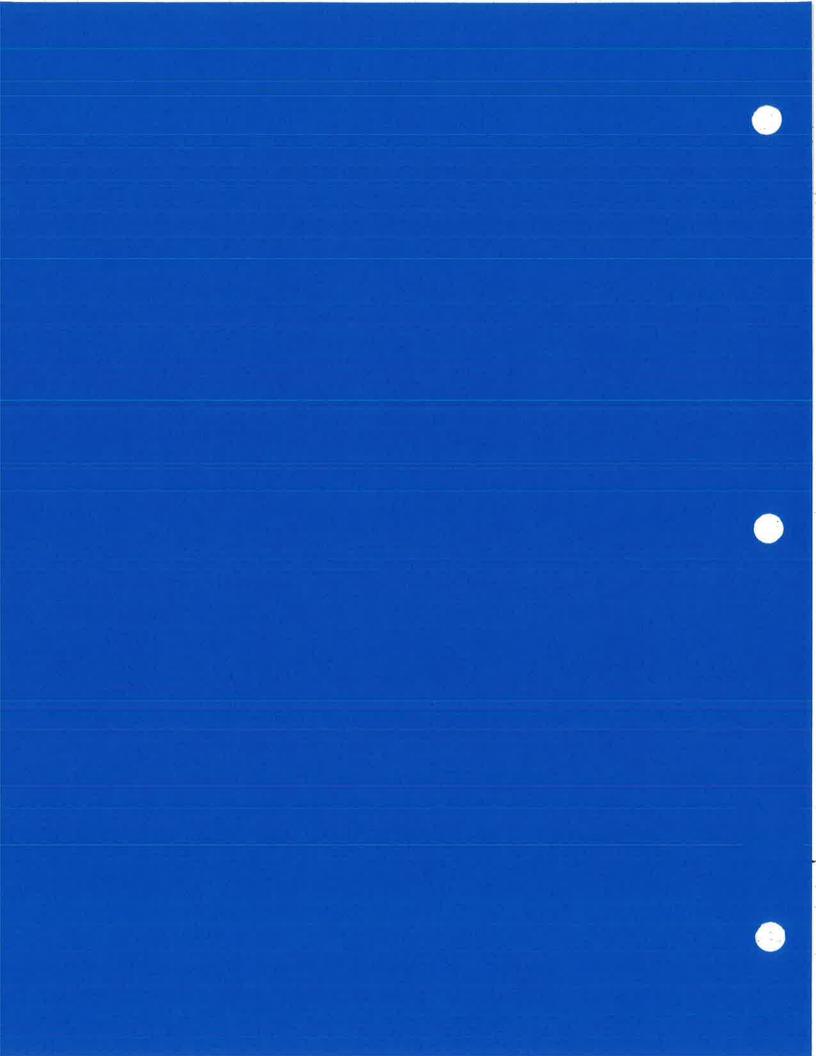


APPENDIX A - DEFINITIONS

ABUTMENT	The part of the valley side against with the dam is constructed. Right and left abutments are those on respective sides as an observer when looking downstream.
APPURTENANCES	Structures around a dam that are necessary to the operation of the dam project (i.e., spillways).
BREACH	An opening through the dam resulting in partial or total failure of the dam.
CATEGORY I DAM	A dam that is either 25 feet tall or impounds 100 acre-feet of water and where improper operation or dam failure would result in the probable loss of human life.
COMPREHENSIVE EAP EXERCISE	An in-depth exercise of an EAP that involves the interaction of the dam owner with the state and local emergency management agencies in a stressful environment with time constraints. Functional and full scale EAP exercises are considered comprehensive EAP exercises.
DAM FAILURE	Catastrophic type of failure characterized by the sudden, rapid, and uncontrolled release of impounded water. It is recognized that there are lesser degrees of failure and that any malfunction or abnormality outside the design assumptions and parameters which adversely affect a dam's primary function of impounding water is properly considered a failure. Such lesser degrees of failure can progressively lead to or heighten the risk of a catastrophic failure. They are, however, normally amendable to corrective action.
EMBANKMENT	Fill material, usually earth or rock, placed with sloping sides.
EMERGENCY ACTION PLAN (EAP) EXERCISE	An activity designed to promote emergency preparedness; test or evaluate emergency action plans, procedures or facilities; train personnel in emergency management duties; and demonstrate operational capability. Exercises consist to the performance of duties, tasks or operations very similar to the way they would be performed in a real emergency. However, the exercise performance is
EMERGENCY	in response to a simulated event.
MANAGEMENT AGENCY	The State and local agencies responsible for emergency operations, planning,
EMERGENCY OPERATIONS	mitigation, preparedness, response, and recovery for all hazards.
CENTER	The location or facility where responsible officials gather during an emergency to direct and coordinate emergency operations, to communicate with other jurisdictions and with field emergency forces, and to formulate protective action decisions and recommendations during an emergency.
FLOOD ROUTING	A process of determining progressively over time the amplitude of a flood wave as it moves past a dam or downstream to successive points along a river or stream.
HAZARD	A situation which creates the potential for adverse consequences such as loss of life, property damage or other adverse impacts. Impacts may be for a defined area downstream of a dam from flood-waters released through spillways and outlet works of the dam or waters released by partial or complete failure of the dam.
HEADWATER	The water immediately upstream from a dam. The water surface elevation varies due to fluctuations in inflow and the amount of water passed through the dam.
NUNDATION MAP	A map delineating areas that would be flooded as a result of a dam failure or other unusually large spillway release.
OVERTOP	Flow of an embankment dam beyond its spillway capacity and over the top of the dam crest, or containment elevation.

PIPING	Internal erosion caused by seepage.
PROJECT DESIGN FLOOD	The maximum rate of rainfall in which the dam could safely pass or store without overtopping.
SEEPAGE	The continuous movement of water from the upstream face of the dam toward its downstream face.
SETTLEMENT	The downward movement of the ground due to forces (i.e., buildings and other structures) applied to the surface.
SPILLWAY	A structure over or through which flood flows are discharged.
TAIL WATER	The water downstream from the dam.

APPENDIX B INUNDATION MAPS AND CALCULATIONS PREPARED BY JORDAN, JONES, AND GOULDING AND BIG CANOE POA



APPENDIX B Page 1

Area Description

Petit Lake is a 115 foot high earthen embankment in the upper reaches of the Blackwell Creek drainage basin in northeastern Pickens County. It impounds approximately 6,000 acre-feet of water, according to National Inventory of Dams data (1992). The contributing drainage area is approximately 1.53 square miles consisting primarily of heavily wooded residential development and heavily wooded undeveloped land.

Approximately 1.0 mile downstream of Petit Lake on Blackwell Creek is Sconti Lake. The embankment here is approximately 40 feet high and only impounds approximately 260 acre-feet according to the National Inventory of Dams data (1992). The drainage area to Sconti, minus the area into Petit, is approximately 3.3 square miles, again consisting primarily of heavily wooded residential and heavily wooded undeveloped land.

Between the two lakes, the valley is partially cleared and serves primarily as a golf course and for other recreational activities. Downstream of Sconti the valley is relatively narrow, typical of North Georgia undeveloped streams, and is generally heavily wooded.

Approximately 1.33 miles downstream of Sconti is a two-lane roadway (Wilderness Parkway) crossing of Blackwell Creek consisting of a covered bridge approximately 100 feet in length. The roadway connects to the south main entrance to the Big Canoe community.

Approximately 0.6 miles further downstream is another two-lane roadway crossing named Cove Road. This is a county maintained road. This crossing consists of three 10' x 10' box culverts.

Downstream of Cove Road, according to the U.S.G.S. Nelson Quadrangle Map, is Cox Lake. The embankment is approximately 97 feet high

Downstream of Cox Lake, the stream is again relatively narrow and heavily wooded. There are several homes at various points along the stream in this area and the stream adds some additional tributaries. Just east of the Marblehill community and parallel to the north of Georgia Hwy. 53, the stream widens into a small floodplain consisting of cultivated fields.

From here, the stream curls to the northwest and then south around a small rise and goes under Hwy. 53 in Marblehill. It then turns to the northeast to travel through Marblehill, going under a railroad bridge in the center of Marblehill.

From Marblehill the stream enters a relatively wide floodplain extending to the east of Marblehill and parallel to the south of Hwy. 53.

APPENDIX B Page 2

Modeling

The stream from Petit Lake to the floodplain east of Marblehill has been modeled using the National Weather Service program, DAMBRK. The dam breach flood volume and peak downstream times, elevations and flows have been calculated using this program.

Cox Lake has been modeled using design plans provided by the NCRS office in Jasper, Georgia. It was initially hoped that the flood wave would be stopped and held in this structure. However, the magnitude of the flood wave is such that the dam is overtopped with substantial flow occurring through the emergency spillway and over the top of the dam. The height of the modeled overtopping is approximately 1.25 feet or 0.75 feet below the breach elevation (two feet above top of dam). It is assumed since this is an engineered structure and has been periodically inspected through the years, it would be able to withstand this overtopping without breaching.

The covered bridge road and Cove Road were both modeled as dams with minimal initial flow-through for initial conditions computations. The area of opening capacity for both roads below the roadway elevation is insignificant compared to the magnitude of the breach flood peak, therefore it is not necessary to explicitly model the bridge openings.

In contrast, the Hwy. 53 and railroad bridge in Marblehill were modeled as bridges with the area of opening below the embankment defined. The two crossings were field measured and both are in good structural condition. However, the breach wave height exceeds the embankment height by more than two feet for both crossings resulting in breaching of both crossings.

Worst Case Scenario

Two scenarios are generally appropriate for modeling a dam breach wave. One scenario would consist of a sunny day breach which have the model beginning with a full reservoir to the top of dam elevation with all outflow structures blocked. The other would consist of a project storm breach consisting of water level beginning at normal pool, normal outflow occurring, and a project storm hydrograph entering the reservoir with the breach occurring at the peak time of the inflow hydrograph. The two scenarios would be compared and the worst case used to define the dam breach flood zone. For dams with large storage capacities and large drainage areas, the two scenarios are generally necessary for determining the worst case for downstream flooding.

Petit Lake has a large storage capacity and small inflow drainage basin. Only one breach scenario has been modeled for this EAP, the sunny day breach. The storm breach analysis was limited because the excess storage capacity before the dam is overtopped minus the spillway outflow is adequate to contain the storm runoff from this relatively small sparsely developed drainage basin. The peak breach wave from this model at Petit Lake is 225,253 cubic feet per second. With the starting water surface level at the top of dam elevation, the freeboard is completely filled.

Preliminary project storm hydrographs have been developed for both Petit Lake and Sconti Lake using HEC-1. The hydrographs are for PMP rainfall, taken from Hydrometeorological Report No. 51 from the National Weather Service, consisting of 30.2 and 41 inches for the 6-Hour and 24-Hour duration storms, respectively. A Curve Number and Lag Time have been determined using Soil Conservation Service (now National Resource Conservation Service) techniques. The peak flow for Petit Lake for the two storms is 14,624 cfs (6-Hour) and 7,100 cfs (24-Hour). This is less than 10% of the modeled initial peak breach flow. For Sconti Lake (not including the Petit Lake drainage area) the two peaks are 18,830 cfs (6-Hour) and 13,294 cfs (24-Hour), again less than 10% of the breach flow.

APPENDIX B Page 3

During the project storm scenario, the freeboard storage and outflow capacity of Petit Lake would cause the dam breach to begin at a level substantially below the top of dam elevation, thus producing a lowered volume and peak when compared to the sunny day scenario. The project storm hydrograph into Sconti Lake would not raise a total project storm breach flow peak to above the level of the sunny day scenario.

Because of the large storage capacity of Petit Lake and the relatively small PMP inflows to both Petit and Sconti Lakes, we are confident the sunny day scenario represents the worst case. The mapped extents of the sunny day model represents the maximum area of flood that would occur for any breach scenario.

Modeling Parameters

Modeling parameters have been developed in accordance with the Georgia Safe Dams Program Engineering Guidelines, 1998 Edition (1). The following breach parameters have been used:

Petit Lake

Starting Water Surface Elevation = Top of Dam (1648 msl)

(1) Breach Base Width = Height of Dam (118 feet)

Breach Side Slope = 1:1

(1) Time to Complete Failure = 0.5 hours

Sconti Lake

Starting Water Surface Elevation = Normal Pool (1464.3 msl, approx.)

(1) Breach Base Width = Height of Dam (40 feet)

Breach Side Slope = 1:1

(1) Time to Complete Failure = 0.5 hours

Covered Bridge

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (31 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

Cove Road

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (30 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

Cox Lake

Starting Water Surface Elevation = Normal Pool (1276.2 msl)

(1) Breach Base Width = Height of Dam (97 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.5 hours

Hwy. 53 in Marblehill

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (22 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

Railroad Bridge in Marblehill

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (22 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

APPENDIX B Page 4

Manning's values and expansion/contraction coefficients were based on engineering judgment after visual inspection of the Blackwell Creek valley. Sections were obtained from a digital terrain model developed from U.S.G.S. quad topography and were adjusted to reflect visual inspection of the stream valley channel. Minor adjustments were made in stream reach distances to promote numerical stability in the model.

Areas of off-channel storage have been incorporated at appropriate locations in the model.

Additional parameters were included or modified based on requirements for calculating initial conditions and for promoting numerical stability through a complete peak assessment for all sections. These additional parameters include: section smoothing, delta x distances, constant turbine flow, additional reservoir inflow, lateral inflow, and others as required. These additional parameters serve only to promote the numerical stability of the model and do not minimize the magnitude of the peak elevation and flow results.

Results

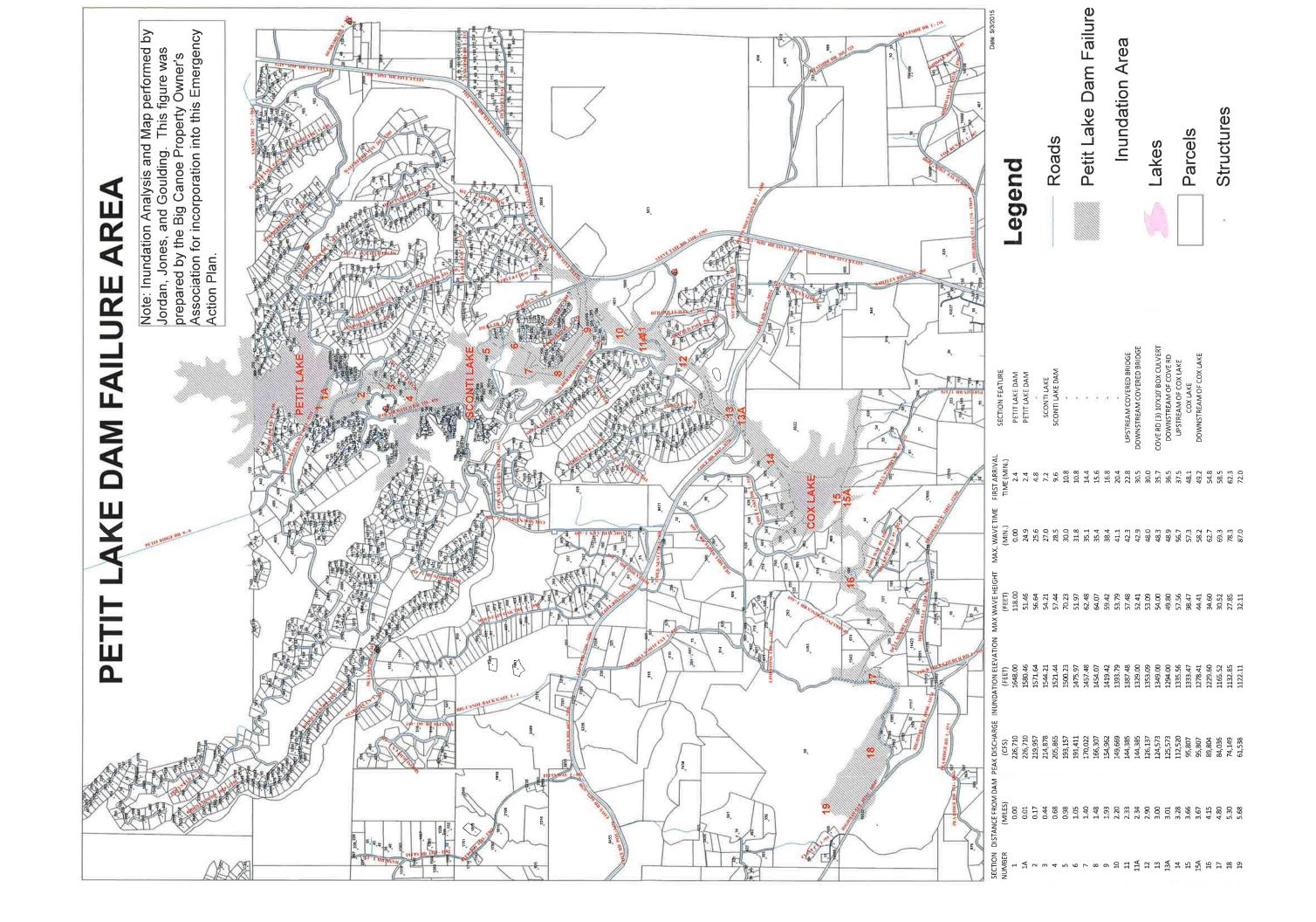
The modeling results indicate that Sconti Lake Dam, the covered bridge road, and Cove Road would all be breached under a substantial flood wave.

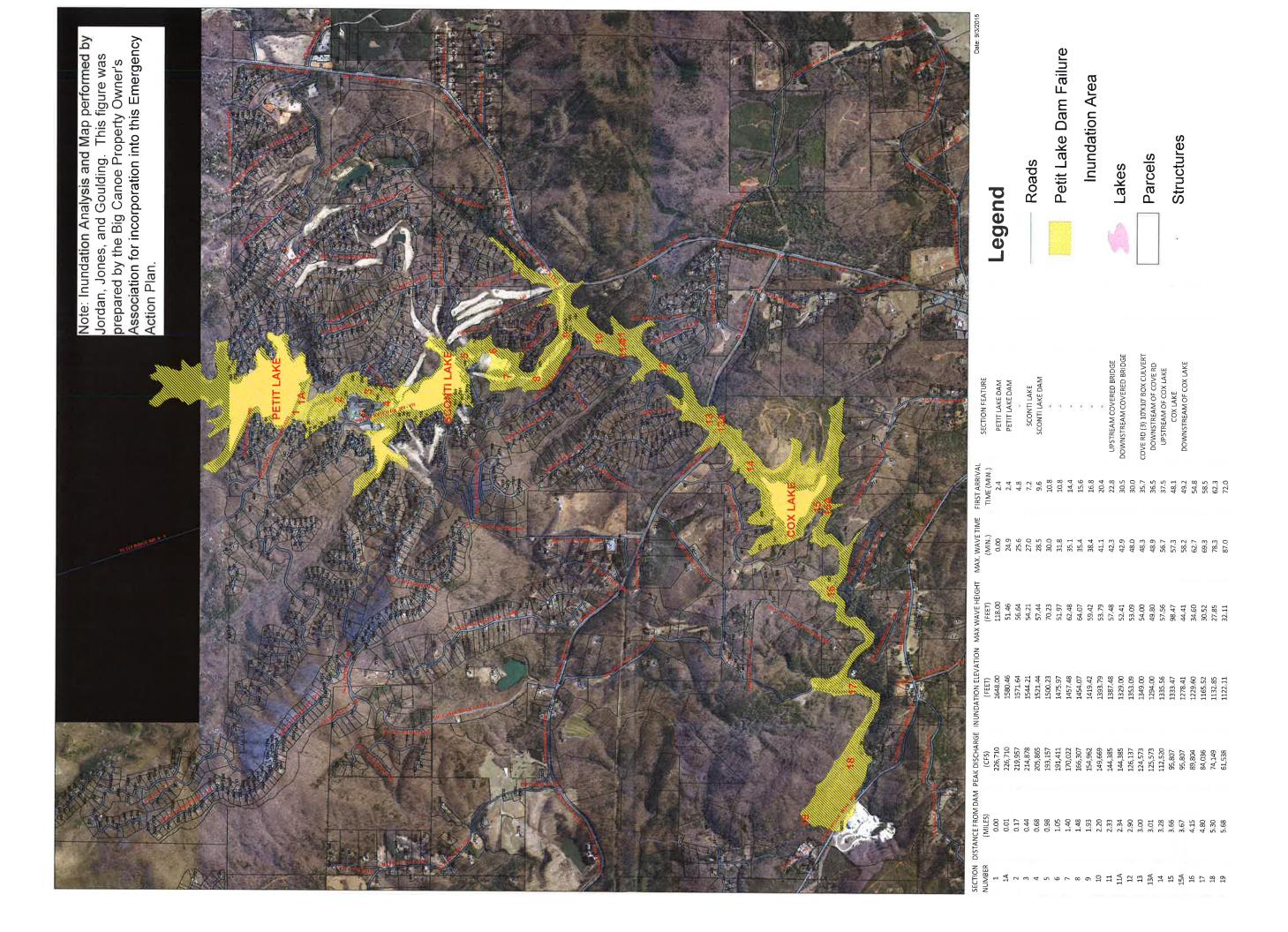
Peak elevations indicate that a small portion of Steve Tate Road would be underwater.

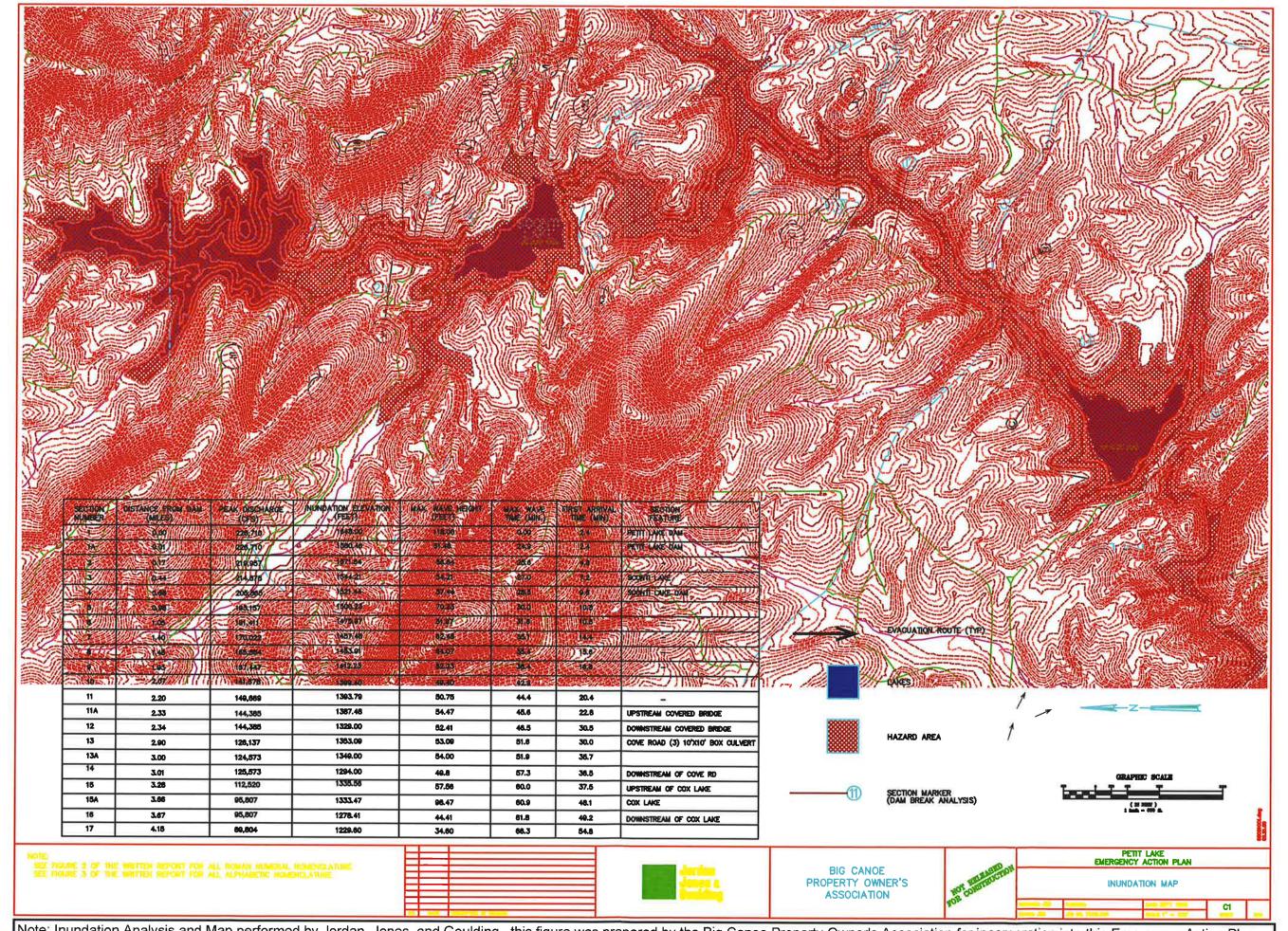
The model indicates that Cox Lake would be overtopped but not breached. Portions of Hwy. 53 to the east and to the west of Marblehill would be temporarily submerged but are on the edge of the main flood channel. The crossings in Marblehill would be breached and areas in Marblehill within the floodplain south of Hwy. 53 would be inundated and in the direct path of the breach wave.

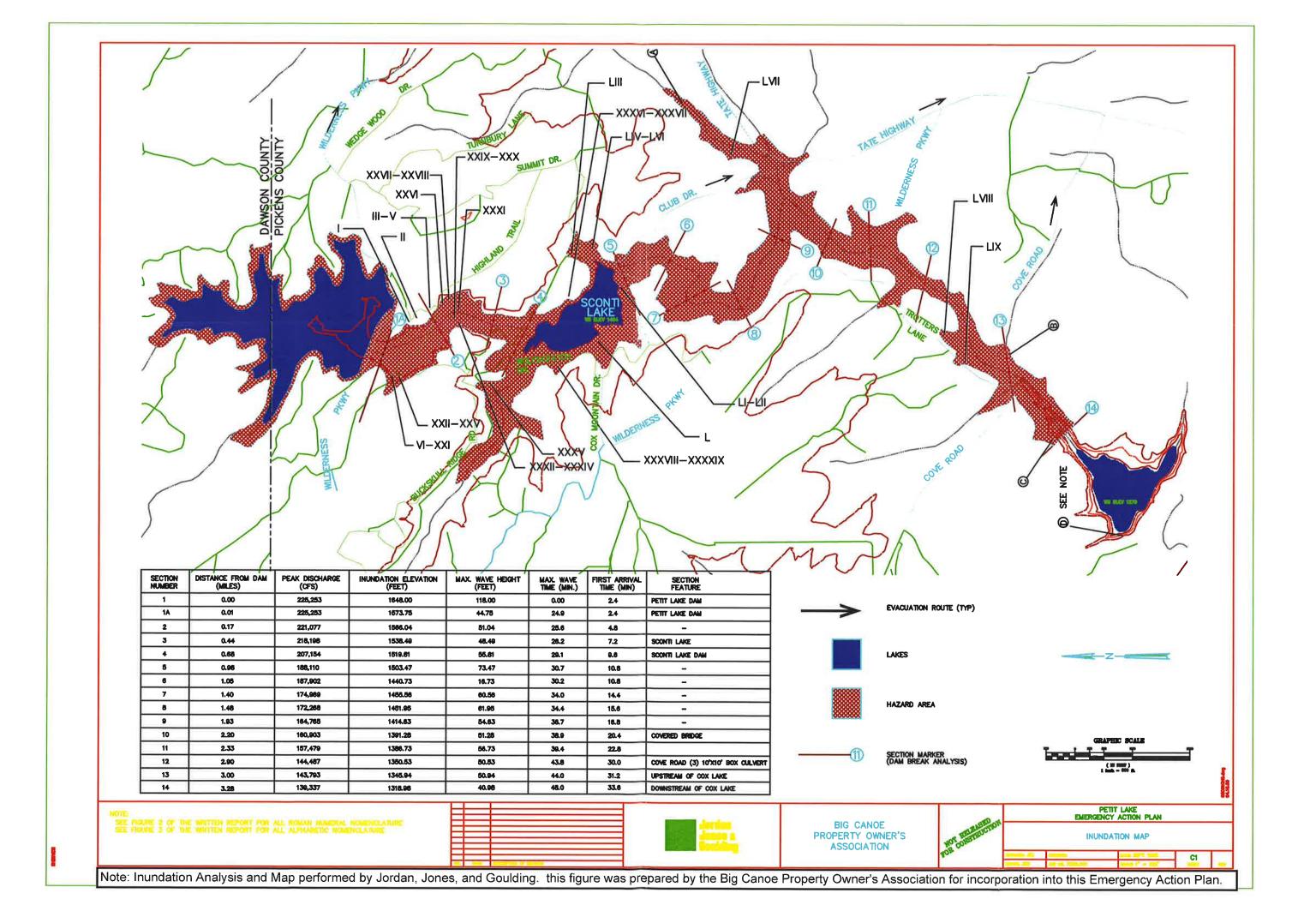
The model indicates that the breach wave is dissipated to a height of approximately 22 feet just upstream of where Hwy. 53 again crosses the stream, east of Marblehill. The height of the bridge/embankment here is approximately 27 feet. It is therefore assumed this crossing serves to break the flood wave and dissipate the volume in the upstream floodplain.

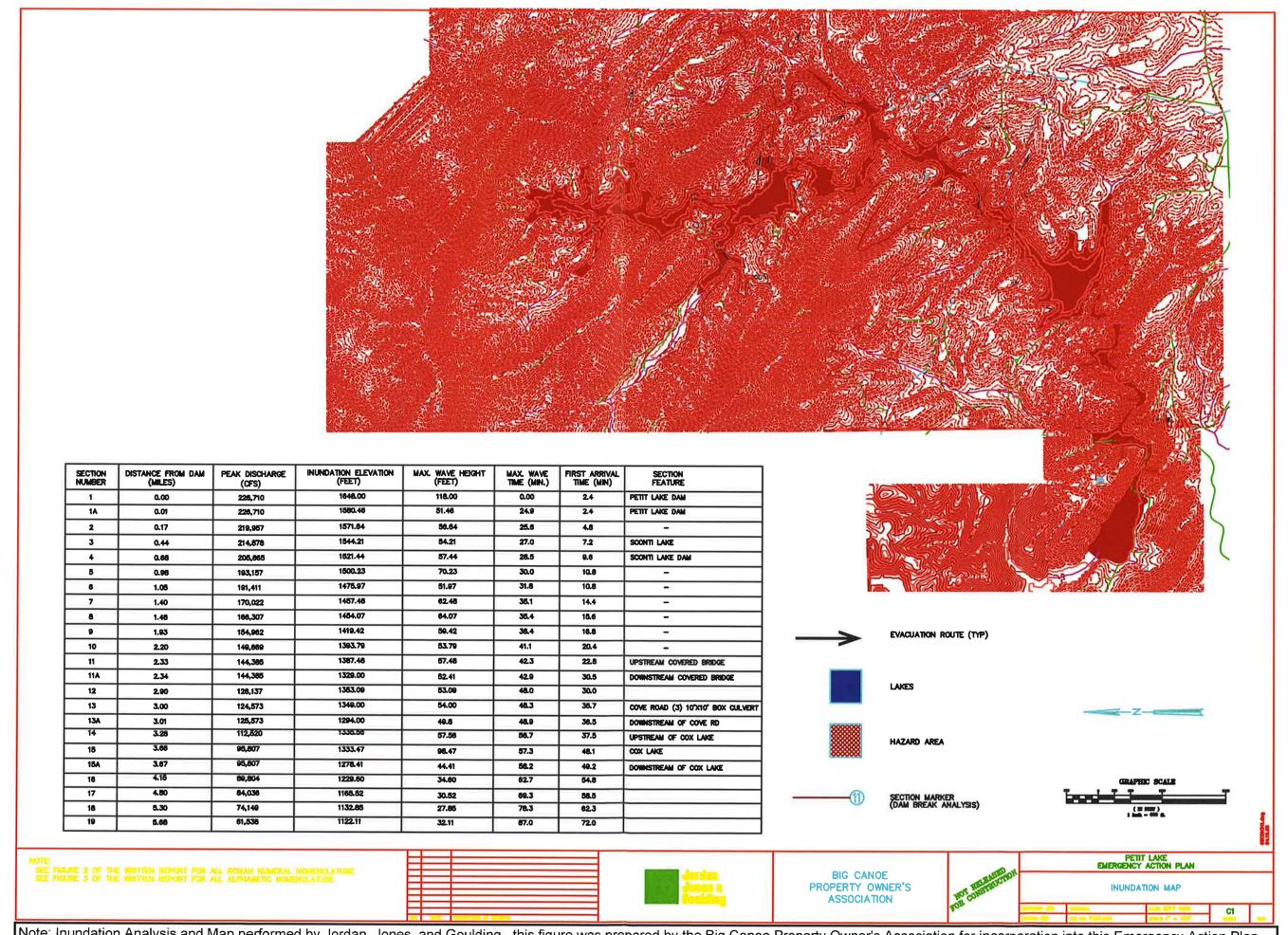
The extent of the breach flooding is shown on the attached maps. DAMBRK and HEC-1 input and output is attached.

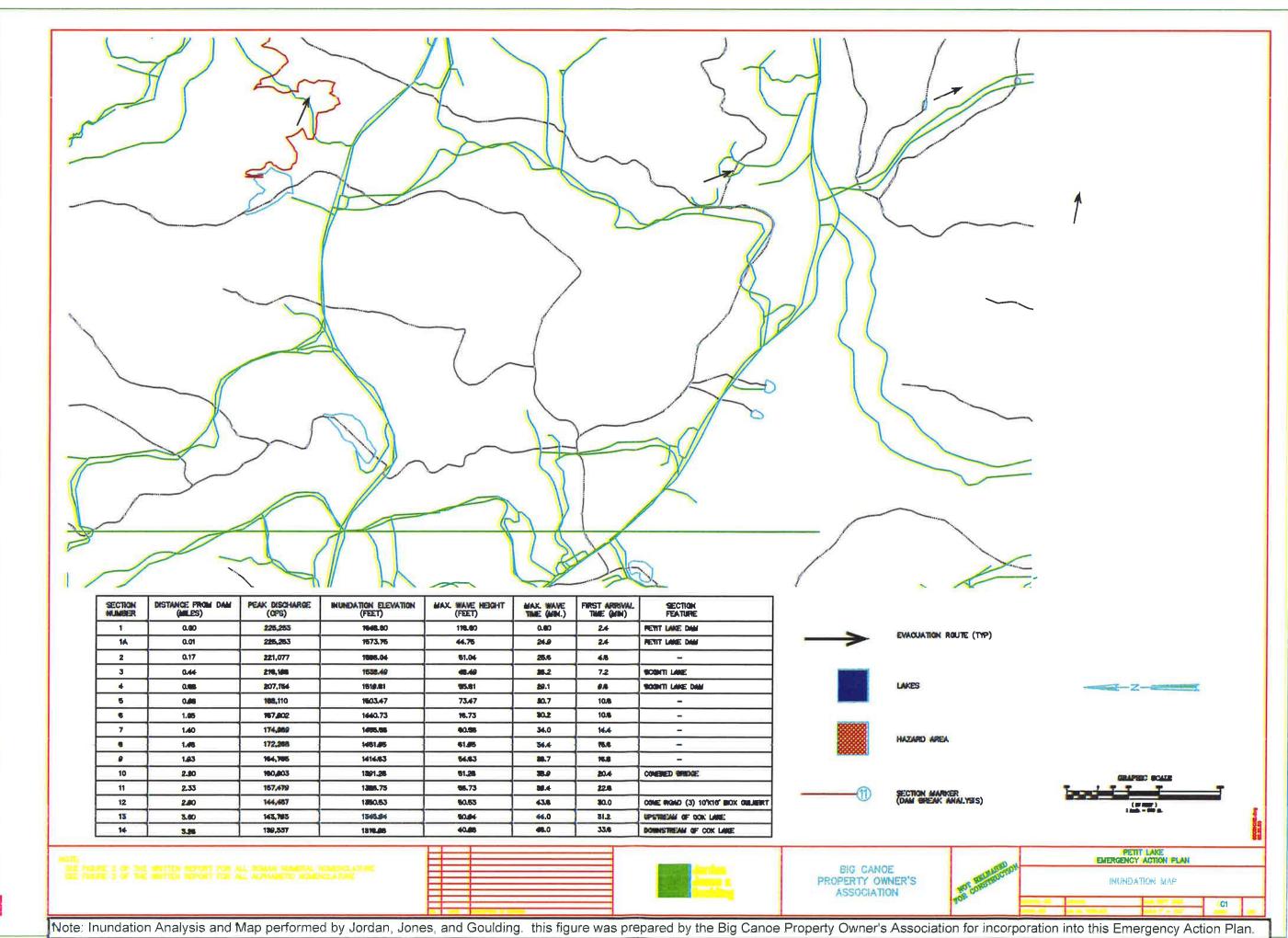












Emerg.	Action Plan	1 Lake Pet	it Dam	Jordan,	Jones, &	Goulding		
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Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's Association for incorporation into this Emergency Action Plan.

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0.050 0.050 0.050	0.06 0.06 0.06	0.10 0.10 0.10	0.12 0.12 0.12	0.15 0.15 0.15	0.15 0.15 0.15	0.18 0.18 0.18	0.18 0.18 0.18
0.050 0.055	0.06	0.10 0.10	0.12	0.15 0.15	0.15 0.15	0.18 0.18	0.18 0.18
0.060 0.060 0.050	0.07 0.07 0.06	0.10 0.10 0.10	0.12 0.12 0.12	0.15 0.15 0.15	0.15 0.15 0.15	0.18 0.18 0.18	0.18 0.18 0.18
0.050 0.050	0.06 0.06	0.10 0.10 0.10	0.12	0.15 0.15 0.15	0.15 0.15	0.18 0.18	0.18
0.055 0.050 0.055	0.06 0.06 0.06	0.10 0.10 0.10	0.12 0.12 0.12	0.15 0.15 0.15	0.15 0.15 0.15	0.18 0.18 0.18	0.18 0.18 0.18
0.055 0.055	0.06 0.06	0.10 0.10	0.12 0.12	0.15 0.15	0.15 0.15	0.18 0.18	0.18 0.18
0.055 0.055 0.05	0.06 0.06 0.00	0.10 0.10 0.05	0.12 0.12 0.05	0.15 0.15 0.05	0.15 0.15 0.00	0.18 0.18 0.05	0.18 0.18 0.00
0.05 0.00	0.05	0.05	0.00	0.00	0.00	0.05	0.00
0.0 -0.4 0.00	0.00 0.25	0.25 0.00	0.0 0.00	-0.4 0.00	0.00 0.00	0.00	0.25 -0.3
0.0 2	0.0 7	-27.0	0.0	0.0	0.0	0.00	0.0
50.0 00.0	500.0 000.0	1000.0 1000.0					

ANALYSIS OF THE DOWNSTREAM FLOOD HYDROGRAPH PRODUCED BY THE DAM BREAK OF

Emerg. Action Plan

ON

Lake Petit Dam

ANALYSIS BY

Jordan, Jones, & Goulding

BASED ON PROCEDURE DEVELOPED BY DANNY L. FREAD, PH.D., SR. RESEARCH HYDROLOGIST

QUALITY CONTROL TESTING AND OTHER SUPPORT BY JANICE M. LEWIS, RESEARCH HYDROLOGIST

HYDROLOGIC RESEARCH LABORATORY W23, OFFICE OF HYDROLOGY NOAA, NATIONAL WEATHER SERVICE SILVER SPRING, MARYLAND 20910

1

***	*****	***	****	****	***
****	*****	***	*****	*****	****
***					***
***	SUMMARY	OF	INPUT	DATA	***
***					***
***	*****	***	*****	****	****
***	*****	***	*****	****	****

INPUT CONTROL PARAMETERS FOR Emerg. Action Plan

PARAMETER ***********************************	VARIABLE *****	VALUE ******
NUMBER OF DYNAMIC ROUTING REACHES	KKN	1
TYPE OF RESERVOIR ROUTING	KUI	1
MULTIPLE DAM INDICATOR	MULDAM	4
PRINTING INSTRUCTIONS FOR INPUT SUMMARY	KDMP	3
NO. OF RESERVOIR INFLOW HYDROGRAPH POINTS	ITEH	3
INTERVAL OF CROSS-SECTION INFO PRINTED OUT WHEN JN	C=9 NPRT	0
FLOOD-PLAIN MODEL PARAMETER	KFLP	0
METRIC INPUT/OUTPUT OPTION	METRIC	0

IDAM= 1

IDAM= 6

IDAM= 13

IDAM= 16

Emerg. Action Plan RESERVOIR

TABLE OF ELEVATION VS SURFACE AREA

> 148.8 103.9

1650.00 1635.00

34.7	1600.00
14.6	1580.00
4.2	-1560.00
1.1	1540.00
.0	1530.00

DAM NUMBER 1

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER	UNITS V	ARIABLE	VALUE ******
ELEVATION OF WATER SURFACE	FEET	YO	1648.00
SIDE SLOPE OF BREACH		z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1530.00
WIDTH OF BASE OF BREACH	FEET	BB	118.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.50
ELEVATION OF WATER WHEN BREACHED	FEET	KF	1648.00
ELEVATION OF TOP OF DAM	FEET	HD	1648.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		CS	.00
DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	.00
DISCHARGE THRU TURBINES	CFS	QT	1100.00

CDO SHOULD NOT BE 0.0 IF OVERTOPPING MAY OCCUR

DAM NUMBER 2

PARAMETER ***********************************	UNITS V	ARIABLE	VALUE *****
ELEVATION OF WATER SURFACE	FEET	YO	1464.30
SIDE SLOPE OF BREACH		Z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1430.00
WIDTH OF BASE OF BREACH	FEET	ВВ	40.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.50
ELEVATION OF WATER WHEN BREACHED	FEET	НF	1472.00
ELEVATION OF TOP OF DAM	FEET	HD	1470.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	1464.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		CS	318.00

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's Association for incorporation into this Emergency Action Plan.

DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	570.00
DISCHARGE THRU TURBINES	CFS	QT	975.00

DAM NUMBER 3

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER *************************		ARIABLE	VALUE *******
ELEVATION OF WATER SURFACE	FEET	YO	1335.00
SIDE SLOPE OF BREACH		Z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1330.00
WIDTH OF BASE OF BREACH	FEET	ВВ	31.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.10
ELEVATION OF WATER WHEN BREACHED	FEET	HF	1349.00
ELEVATION OF TOP OF DAM	FEET	HD	1347.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	1330.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		CS	45.00
DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	450.00
DISCHARGE THRU TURBINES	CFS	QT	.00

DAM NUMBER 4

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER *********************************	UNITS V	ARIABLE	VALUE *******
ELEVATION OF WATER SURFACE	FEET	YO	1300.00
SIDE SLOPE OF BREACH		Z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1295.00
WIDTH OF BASE OF BREACH	FEET	ВВ	30.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.10
ELEVATION OF MATER AND PROPERTY.			4707.00
ELEVATION OF WATER WHEN BREACHED	FEET	HF	1327.00
ELEVATION OF TOP OF DAM	FEET	HD	1325.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	1295.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		cs	45.00
DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	750.00

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's Association for incorporation into this Emergency Action Plan.

DHF(INTERVAL BETWEEN INPUT HYDROGRAPH ORDINATES) = .00 HRS.

TEH(TIME AT WHICH COMPUTATIONS TERMINATE)= .8100 HRS.

BREX(BREACH EXPONENT) = 1.000

MUD(MUD FLOW OPTION) = 0

IWF(TYPE OF WAVE FRONT TRACKING) = 0

KPRES(WETTED PERIMETER OPTION) = 0

KSL(LANDSLIDE PARAMETER) = 0

DFR(WINDOW FOR CRITICAL FROUDE NO. IN MIX FLOW ALGORITHM)= .050

INFLOW HYDROGRAPH TO Emerg. Action Plan

100.00 1300.00 10000.00

TIME OF INFLOW HYDROGRAPH ORDINATES

1

.0000 2.0000 24.0000

CROSS-SECTIONAL PARAMETERS FOR Lake Petit Dam BELOW Emerg. Action Plan

PARAMETER ***********************************	VARIABLE	VALUE *****
NUMBER OF CROSS-SECTIONS	NS	18
MAXIMUM NUMBER OF TOP WIDTHS	NCS	8
NUMBER OF CROSS-SECTIONAL HYDROGRAPHS TO PLOT	NTT	6
TYPE OF OUTPUT OTHER THAN HYDROGRAPH PLOTS	JNK	0
CROSS-SECTIONAL SMOOTHING PARAMETER	KSA	-1
DOWNSTREAM SUPERCRITICAL OR NOT	KSUPC	2
NO. OF LATERAL INFLOW HYDROGRAPHS	LQ	2
NO. OF POINTS IN GATE CONTROL CURVE	KCG	0

NUMBER OF CROSS-SECTION WHERE HYDROGRAPH DESIRED (MAX NUMBER OF HYDROGRAPHS = 6)

1 2 6 13 16 18

SMF= .90 NTSM= 5 NSMR= 7

NDSM(K)	NUSM(K)
3	2
4	3
6	5
8	7
9	8
10	9
11	10

CROSS-SECTIONAL VARIABLES FOR Lake Petit Dam BELOW Emerg. Action Plan

PARAMETER	UNITS	VARIABLE
LOCATION OF CROSS-SECTION	MILE	XS(I)
ELEVATION(MSL) OF FLOODING AT CROSS-SECTION	N FEET	FSTG(I)
ELEV CORRESPONDING TO EACH TOP WIDTH	FEET	HS(K,I)
TOP WIDTH CORRESPONDING TO EACH ELEV	FEET	BS(K,I)
(ACTIVE FLOW PORTION)		
TOP WIDTH CORRESPONDING TO EACH ELEV	FEET	BSS(K,I)
(OFF-CHANNEL PORTION)		
NUMBER OF CROSS-SECTION		I
NUMBER OF ELEVATION LEVEL		K

XS(I) = .000 FSTG(I) = .00

HS ... 1530.0 1540.0 1560.0 1580.0 1600.0 1620.0 1640.0 1650.0 BS ... 300.0 450.0 630.0 840.0 1070.0 1600.0 1940.0 2300.0

BSS ... 0. 0. 0. 0. 0. 0

CROSS-SECTION NUMBER 2

XS(I) = .010 FSTG(I) = .00

HS 1529.0 1540.0 1560.0 1580.0 1600.0 1620.0 1640.0 1650.0 BS 300.0 450.0 630.0 840.0 1070.0 1600.0 1940.0 2300.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 3

 $XS(I) = .170 ext{ FSTG}(I) = .00$

1515.0 1520.0 1540.0 1560.0 1580.0 1590.0 1600.0 1610.0 HS 680.0 850.0 980.0 1190.0 BS 40.0 105.0 300.0 770.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 4

1

XS(I) = .440 FSTG(I) = .00

HS 1490.0 1495.0 1500.0 1510.0 1520.0 1540.0 1560.0 1564.0 BS 13.0 35.0 165.0 250.0 315.0 432.0 560.0 620.0 BSS0 .0 .0 .0 .0 .0 .0 .0

XS(I) = .680 FSTG(I) = .00

1464.0 1470.0 1480.0 1490.0 1500.0 1510.0 1520.0 1540.0 HS 10.0 180.0 470.0 1060.0 1180.0 1230.0 1450.0 BS .0 BSS0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 6

XS(I) = .980 FSTG(I) = .00

1430.0 1460.0 1470.0 1480.0 1490.0 1500.0 1505.0 1540.0 HS ... 5.0 100.0 276.0 356.0 506.0 680.0 960.0 2395.0 BS ... BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 7

XS(I) = .990 FSTG(I) = .00

HS 1429.0 1470.0 1480.0 1490.0 1500.0 1505.0 1540.0 1460.0 5.0 100.0 276.0 356.0 506.0 680.0 960.0 2395.0 BS BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 8

1

XS(I) = 1.050 FSTG(I) = .00

1424.0 1425.0 1430.0 1440.0 1450.0 1460.0 1470.0 1484.0 HS 19.0 39.0 180.0 350.0 637.0 785.0 978.0 BS ... 263.0 BSS0 .0 .0 .0 .0 .0 .0 .0

XS(1) = 1.400 FSTG(1) = .00

НS 1395.0 1400.0 1410.0 1420.0 1425.0 1430.0 1440.0 1488.0 BS 29.0 695.0 65.0 240.0 420.0 510.0 590.0 1185.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 10

XS(I) = 1.480 FSTG(I) = .00

HS 1390.0 1395.0 1400.0 1405.0 1420.0 1430.0 1440.0 1462.0 BS 21.0 30.0 55.0 170.0 290.0 380.0 455.0 590.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 11

XS(I) = 1.930 FSTG(I) = .00

1360.0 1365.0 1370.0 1390.0 1400.0 1420.0 1440.0 1450.0 HS 125.0 BS 18.0 41.0 255.0 320.0 435.0 560.0 650.0 .0 BSS0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 12

XS(I) = 2.200 FSTG(I) = .00

1340.0 1345.0 1350.0 1360.0 1380.0 1390.0 1420.0 1430.0 BS 28.0 46.0 140.0 ... 240.0 425.0 520.0 710.0 790.0 .0 BS\$0 .0 .0 .0 .0 .0 .0

XS(I) = 2.330 FSTG(I) = .00

HS 1330.0 1340.0 1350.0 1360.0 1380.0 1390.0 1400.0 1410.0 470.0 790.0 50.0 90.0 330.0 BS 700.0 845.0 1020.0 BSS0 .0 ٠0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 14

XS(1) = 2.340 FSTG(1) = .00

HS 1329.0 1340.0 1350.0 1360.0 1380.0 1390.0 1400.0 1410.0 BS 17.0 90.0 330.0 470.0 700.0 790.0 845.0 1020.0 .0 .0 BSS0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 15

XS(1) = 2.900 FSTG(1) = .00

1300.0 1305.0 1310.0 1320.0 1330.0 1340.0 1360.0 1390.0 HS 285.0 BS 23.0 40.0 380.0 450.0 630.0 145.0 850.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 16

1

XS(1) = 3.000 FSTG(1) = .00

1295.0 1330.0 HS 1325.0 1340.0 1350.0 1360.0 1370.0 1380.0 BS 30.0 250.0 540.0 830.0 985.0 1290.0 1450.0 1135.0 .0 BSS0 .0 .0 .0 .0 .0 .0

XS(I) = 3.010 FSTG(I) = .00

1294.0 1325.0 1330.0 1340.0 1350.0 1360.0 1370.0 1380.0 HS 30.0 250.0 540.0 830.0 985.0 1135.0 1290.0 1450.0 BS .0 .0 .0 .0 .0 .0 BSS0

CROSS-SECTION NUMBER 18

XS(I) = 3.280 FSTG(I) = .00

HS 1278.0 1280.0 1300.0 1320.0 1340.0 1360.0 1380.0 1400.0 BS 35.0 115.0 300.0 530.0 665.0 780.0 915.0 1095.0 BSS0 .0 .0 .0 .0 .0 .0 .0

1	BST		300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
1	BSST		.00	.00	.00	-00	.00	.00	.00	.00
2	BST		300.00	450.00	630.00		1070.00	1600.00	1940.00	2300.00
2	BSST		.00	.00	.00	.00	.00	.00	.00	.00
3	BST		40.00	105.00	300.00	680.00	770.00	850.00	980.00	1190.00
3	BSST		.00	.00	.00	.00	.00	.00	.00	.00
4	BST		13.00	35.00	165.00	250.00	315.00	432.00	560.00	620.00
4	BSST		.00	.00	.00	.00	.00	.00	.00	.00
5	BST		5.00	10.00	180.00	470.00	1060.00	1180.00	1230.00	1450.00
5	BSST		.00	.00	.00	.00	.00	.00	.00	.00
6	BST		5.00	100.00	276.00	356.00	506.00	680.00	960.00	2395.00
6	BSST		.00	.00	.00	.00	.00	.00	.00	.00
7	BST		5.00	100.00	276.00	356.00	506.00	680.00	960.00	2395.00
7	BSST		.00	.00	.00	.00	.00	.00	.00	.00
8	BST		19.00	39.00	180.00	263.00	350.00	637.00	785.00	978.00
8		• • •	.00	.00	.00	.00	.00	.00	.00	.00
9	BST		29.00	65.00	240.00	420.00	510.00	590.00	695.00	1185.00
9	BSST		.00	.00	.00	.00	.00	.00	.00	.00
10			21.00	30.00	55.00	170.00	290.00	380.00	455.00	590.00
				-			-			. 00
		•••								650.00
										00
		• • •								790.00
							-			.00
		• • •								1020.00
										.00
										1020.00
										.00
										850.00
		•••								- 00
										1450.00
										.00
		•••							77	1450.00
										.00
								1.3		1095.00
18	BSST		.00	.00	.00	.00	.00	.00	.00	. 00
	1 2 2 3 3 4 4 5 5 6 6 7 7 8 8 9 9	1 BSST 2 BSST 2 BSST 3 BSST 3 BSST 4 BSST 5 BSST 6 BSST 6 BSST 7 BSST 7 BSST 7 BSST 10 BSST 10 BSST 11 BSST 11 BSST 12 BSST 11 BSST 12 BSST 14 BSST 15 BSST 16 BSST 17 BSST 17 BSST 18 BST 17 BSST 18 BST 17 BSST 18 BST 18 BST	1 BSST 2 BST 2 BSST 3 BSST 3 BSST 4 BSST 5 BSST 6 BSST 6 BSST 6 BSST 7 BSST 8 BSST 9 BSST 10 BSST 11 BSST 12 BSST 13 BSST 14 BSST 15 BSST 16 BSST 17 BSST 18 BST 18 BST 19 BSST 11 BSST 11 BSST 12 BSST 13 BSST 14 BSST 15 BSST 16 BSST 17 BSST 18 BST 18 BST 19 BSST 11 BSST 12 BSST 13 BSST 14 BSST 15 BSST 16 BSST 17 BSST 18 BST 18 BST 19 BSST 10 BSST 11 BSST 12 BSST 13 BSST 14 BSST 15 BSST 16 BSST 17 BSST 18 BST 18 BST	1 BSST .00 2 BST 300.00 2 BSST .00 3 BSST .00 4 BST .13.00 4 BSST .00 5 BSST .00 5 BSST .00 6 BSST .00 7 BSST .00 8 BSST .00 9 BSST .00 9 BSST .00 10 BSST .00 11 BSST .00 12 BSST .00 13 BSST .00 14 BSST .00 15 BSST .00 15 BSST .00 15 BSST .00 16 BSST .00 17 BSST .00 16 BSST .00 17 BSST .00 16 BSST .00 17 BSST .00	1 BSST .00 .00 2 BSST .300.00 .450.00 2 BSST .00 .00 3 BSST .00 .00 3 BSST .00 .00 4 BSST .00 .00 5 BSST .00 .00 5 BSST .00 .00 6 BSST .00 .00 6 BSST .00 .00 7 BSST .00 .00 8 BSST .00 .00 9 BSST .00 .00 9 BSST .00 .00 10 BSST .00 .00 10 BSST .00 .00 10 BSST .00 .00 11 BSST .00 .00 12 BSST .00 .00 13 BSST .00 .00 14 BST .00 .00 15 BST	1 BSST .00 .00 .00 2 BST .300.00 450.00 630.00 2 BSST .00 .00 .00 3 BST .40.00 105.00 300.00 3 BSST .00 .00 .00 4 BSST .00 .00 .00 4 BSST .00 .00 .00 5 BST .500 10.00 180.00 5 BSST .00 .00 .00 6 BSST .00 .00 .00 6 BSST .00 .00 .00 7 BSST .00 .00 .00 8 BST 19.00 39.00 180.00 9 BSST .00 .00 .00 9 BSST .00 .00 .00 10 BST .29.00 65.00 240.00 9 BSST .00 .00 .00 10 BST .20 .0	1 BSST .00 .00 .00 .00 2 BSST .300.00 450.00 630.00 840.00 2 BSST .00 .00 .00 .00 3 BSST .00 .00 .00 .00 4 BSST .00 .00 .00 .00 4 BSST .00 .00 .00 .00 5 BSST .500 10.00 180.00 470.00 5 BSST .500 10.00 180.00 470.00 5 BSST .500 100.00 276.00 356.00 6 BSST .500 100.00 276.00 356.00 6 BSST .500 100.00 276.00 356.00 7 BST 5.00 100.00 276.00 356.00 8 BST .00 .00 .00 .00 9 BST .500 100.00 276.00 356.00 9 BST .500 100.00 .00	1 BSST .00 .00 .00 .00 .00 .00 2 BST .300.00 450.00 630.00 840.00 1070.00 2 BSST .00 .00 .00 .00 .00 3 BSST .40.00 105.00 .300.00 680.00 .770.00 3 BSST .00 .00 .00 .00 .00 4 BST 13.00 35.00 165.00 250.00 .315.00 4 BSST .00 .00 .00 .00 .00 .00 5 BST .500 10.00 180.00 470.00 1060.00 .00	BSST	BSST

Inchines amount	4		1944 <u>-</u>							
CROSS-SECTION	1	BST	300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
CROSS-SECTION	1	BSST	.00	.00	.00	00	.00	.00	.00	.00
CROSS-SECTION	2	BST	300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
CROSS-SECTION	2	BSST	.00	,00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	3	BST	40.00	105.00	300.00	680.00	770.00	850.00	980.00	1190.00
CROSS-SECTION	3	BSST	.00	.00	-00	.00	.00	.00	.00	.00
CROSS-SECTION	4	BST	13.00	35.00	165.00	250.00	315.00	432.00	560.00	620.00
CROSS-SECTION	4	BSST	-00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	5	BST	5.00	10.00	180.00	470.00	970.00	1180.00	1230.00	1450.00
CROSS-SECTION	5	BSST	.00	.00	.00	_00	90.00	.00	.00	.00
CROSS-SECTION	6	BST	5.00	100.00	276.00	356.00	506.00	680.00	930.00	2395.00
CROSS-SECTION	6	BSST	-00	.00	.00	.00	.00	.00	30.00	.00
CROSS-SECTION	7	BST	5.00	100.00	276.00	356.00	506.00	680.00	930.00	2395.00
CROSS-SECTION	7	BSST	.00	.00	.00	.00	.00	.00	30.00	.00
CROSS-SECTION	8	BST	19.00	39.00	180.00	263.00	350.00	637.00	785.00	978.00
CROSS-SECTION	8	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	9	BST	29.00	65.00	240.00	420.00	510.00	590.00	695.00	1185.00
CROSS-SECTION	9	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	10	BST	21.00	30.00	55.00	170.00	290.00	380.00	455.00	590.00
CROSS-SECTION	10	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	11	BST	18.00	41.00	125.00	255.00	320.00	435.00	560.00	650.00
CROSS-SECTION	11	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	12	BST	28.00	46.00	140.00	240.00	425.00	520.00	710.00	790.00
CROSS-SECTION	12	BSST	. 00	.00	.00	.00	.00	00	00	-00
CROSS-SECTION	13	BST	50.00	90.00	330.00	470.00	700.00	790.00	845.00	1020.00
CROSS-SECTION	13	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	14	BST	17.00	90.00	330.00	470.00	700.00	790.00	845.00	1020.00
CROSS-SECTION	14	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	15	BST	23.00	40.00	145.00	285.00	380.00	450.00	630.00	850.00
CROSS-SECTION	15	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	16	BST	30.00	250.00	540.00	830.00	985.00	1135.00	1290.00	1450.00
CROSS-SECTION	16	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	17	BST	30.00	250.00	540.00	830.00	985.00	1135.00	1290.00	1450.00
CROSS-SECTION	17	BSST	.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	18	BST	35.00	115.00	300.00	530.00	665.00	780.00	915.00	1095.00
CROSS-SECTION	18	BSST	.00	.00	.00	.00	.00	.00	.00	.00

CROSS-SECTION	1	HST	 1530.00	1540.00	1560.00	1580.00	1600.00	1620.00	1640.00	1650.00
CROSS-SECTION	2	HST	 1529.00	1540.00	1560.00	1580.00	1600.00	1620.00	1640.00	1650.00
CROSS-SECTION	3	HST	 1515.00	1520.00	1540.00	1560.00	1580.00	1590.00	1600.00	1610.00
CROSS-SECTION	4	HST	 1490.00	1495.00	1500.00	1510.00	1520.00	1540.00	1560.00	1564.00
CROSS-SECTION	5	HST	 1464.00	1470.00	1480.00	1490.00	1500.00	1510.00	1520.00	1540.00
CROSS-SECTION	6	HST	 1430.00	1460.00	1470.00	1480.00	1490.00	1500.00	1505.00	1540.00
CROSS-SECTION	7	HST	 1429.00	1460.00	1470.00	1480.00	1490,00	1500.00	1505.00	1540.00
CROSS-SECTION	8	HST	 1424.00	1425.00	1430.00	1440.00	1450.00	1460.00	1470.00	1484.00
CROSS-SECTION	9	HST	 1395.00	1400.00	1410.00	1420.00	1425.00	1430.00	1440.00	1488.00
CROSS-SECTION	10	HST	 1390.00	1395.00	1400.00	1405.00	1420.00	1430.00	1440.00	1462.00
CROSS-SECTION	11	HST	 1360.00	1365.00	1370.00	1390.00	1400.00	1420.00	1440.00	1450.00
CROSS-SECTION	12	HST	 1340.00	1345.00	1350.00	1360.00	1380.00	1390.00	1420.00	1430.00
CROSS-SECTION	13	HST	 1330.00	1340.00	1350.00	1360.00	1380.00	1390.00	1400.00	1410.00
CROSS-SECTION	14	HST	 1329.00	1340.00	1350.00	1360.00	1380.00	1390.00	1400.00	1410.00
CROSS-SECTION	15	HST	 1300.00	1305.00	1310.00	1320.00	1330.00	1340.00	1360.00	1390.00
CROSS-SECTION	16	HST	 1295.00	1325.00	1330.00	1340.00	1350.00	1360.00	1370.00	1380.00
CROSS-SECTION	17	HST	 1294.00	1325.00	1330.00	1340.00	1350.00	1360.00	1370.00	1380.00
CROSS-SECTION	18	HST	 1278.00	1280.00	1300.00	1320.00	1340.00	1360.00	1380.00	1400.00

MANNING N ROUGHNESS COEFFICIENTS FOR THE GIVEN REACHES (CM(K,I),K=1,NCS) WHERE I = REACH NUMBER

REACH	H 1	•••	.050	.060	.100	.120	.150	.150	.180	.180
REACH	1 2		.050	.060	.100	.120	.150	.150	.180	.180
REACH	1 3	***	.050	.060	.100	.120	.150	.150	.180	.180
REACH	1 4		.050	.060	.100	.120	.150	.150	.180	.180
REACH	1 5		.055	.060	.100	.120	.150	.150	.180	_180
REACH	۱ 6		.060	.070	.100	.120	.150	.150	.180	.180
REACH	7	•••	.060	.070	.100	.120	.150	.150	.180	.180
REACH	8	•••	.050	.060	.100	.120	.150	.150	.180	.180
REACH	9	***	.050	.060	.100	.120	.150	.150	.180	.180
REACH	10	•••	.050	.060	.100	.120	.150	.150	.180	.180
REACH	11		.055	.060	.100	.120	.150	.150	.180	.180
REACH	12	•••	.050	.060	.100	.120	.150	.150	.180	.180
REACH	13	•••	.055	.060	.100	.120	.150	.150	.180	.180
REACH	14		.055	.060	.100	.120	.150	.150	.180	.180
REACH	15		.055	.060	.100	.120	.150	.150	.180	.180
REACH	16	•••	.055	.060	.100	_120	.150	. 150	.180	.180
REACH	17		.055	.060	.100	.120	.150	.150	.180	.180

CROSS-SECTIONAL VARIABLES FOR Lake Petit Dam BELOW Emerg. Action Plan

BETWEEN CROSS-SECTIONS

15

16

17

REACH NUMBER DXM(I) FKC(I) ***** .050 1 .000 2 .000 .000 3 .250 .050 4 .050 .000 5 .050 -.400 .000 6 .000 7 .000 .050 8 .000 .250 9 .050 -.400 10 .050 .250 11 .050 .000 12 .000 .000 13 .000 .000 14 .000 .000

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DOWNSTREAM FLOW PARAMETERS FOR Lake Petit Dam BELOW Emerg. Action Plan

PARAMETER		/ARIABLE	VALUE *******
MAX DISCHARGE AT DOWNSTREAM EXTREMITY	CFS	QMAXD	.0
MAX LATERAL OUTFLOW PRODUCING LOSSES	CFS /FEET	T QLL	.000
INITIAL SIZE OF TIME STEP	HOUR	DTHM	-27.0000
DOWNSTREAM BOUNDARY PARAMETER	FEET	YDN	.000000
SLOPE OF CHANNEL DOWNSTREAM OF DAM	FPM	SOM	.00
THETA WEIGHTING FACTOR		THETA	.00
CONVERGENCE CRITERION FOR STAGE	FEET	EPSY	.000000
TIME AT WHICH DAM STARTS TO FAIL	HOUR	TFI	.00

AT REACH=	2	DXM WAS CHANGED TO .016 DU	JE TO EXP/CONTRACT CRITERIA
AT REACH=	3	DXM SHOULD BE CHANGED TO .C	043 DUE TO EXP/CONTRACT CRITERIA
AT REACH=	5	DXM SHOULD BE CHANGED TO .C	009 DUE TO EXP/CONTRACT CRITERIA
AT REACH=	7	DXM SHOULD BE CHANGED TO .C	001 DUE TO EXP/CONTRACT CRITERIA
AT REACH=	8	DXM WAS CHANGED TO .048 DL	JE TO EXP/CONTRACT CRITERIA
AT REACH=	9	DXM SHOULD BE CHANGED TO .C	017 DUE TO EXP/CONTRACT CRITERIA
AT REACH=	12	DXM WAS CHANGED TO .039 DL	JE TO EXP/CONTRACT CRITERIA
AT REACH=	14	DXM WAS CHANGED TO .166 DU	JE TO EXP/CONTRACT CRITERIA
AT REACH=	15	DXM SHOULD BE CHANGED TO .C	004 DUE TO EXP/CONTRACT CRITERIA
AT REACH=	17	DXM WAS CHANGED TO .010 DU	JE TO EXP/CONTRACT CRITERIA

COMPUTATIONS WILL USE THE FOLLOWING DXM VALUES

101.000 .016 .050 .050 .050 101.000 .050 .048 .050 .050 .050 .039 101.000 .166 .050 101.000 .010

LATERAL INFLOW REACH NUMBER

LQX(I)

2 7

(QL(L, 1),L=1,ITEH) 50. 500. 1000.

(QL(L, 2),L=1,ITEH)
0. 0. 1000.

RE-NUMBERED VALUES FOR IDAM

IDAM(1) = 1

IDAM(2) = 27

IDAM(3) = 54

IDAM(4) = 60

INITIAL CONDITIONS

INITIAL CONDITIONS

100. 150. 1150. 1027. 1027. 1027. 503. 503. 503.	QDI(I),I=1 1100. 1150. 1150. 1150. 1027. 1027. 1027. 503. 503.	,N) 1150. 1150. 1150. 1027. 1027. 1027. 503. 503. 503.	1150. 1150. 1150. 1027. 1027. 1027. 1027. 503. 503. 503.	1150. 1150. 1150. 1027. 1027. 1027. 1027. 503. 503.	1150. 1150. 1150. 1027. 1027. 1027. 503. 503. 503. 503.	1150. 1150. 1150. 1027. 1027. 1027. 503. 503. 503. 503.	1150. 1150. 1150. 1027. 1027. 1027. 503. 503. 503.
1648.00 1521.12 1496.62 1464.31 1411.50 1382.34 1352.32 1313.26 1294.26 1289.43 1284.55	(I(I), I=1, I 1530.02 1520.18 1490.84 1464.30 1407.59 1378.99 1348.63 1303.22 1293.65 1288.82 1283.93	1528.68 1519.40 1485.01 1464.30 1402.37 1375.64 1343.63 1300.44 1293.05 1288.22 1283.31	1527.35 1518.18 1479.68 1437.41 1399.78 1372.26 1340.51 1300.00 1292.45 1287.61 1282.68	1526.04 1513.41 1473.72 1428.06 1395.64 1368.80 1335.86 1296.67 1291.84 1287.00 1282.03	1524.75 1508.86 1465.83 1424.11 1392.33 1364.99 1335.00 1296.06 1291.24 1286.39 1281.36	1523.48 1504.18 1464.26 1420.04 1389.00 1360.75 1332.67 1295.46 1290.64 1285.78 1280.65	1522.26 1500.67 1464.31 1415.96 1385.67 1356.71 1322.70 1294.86 1290.03 1285.17 1280.04

TIME PARAMETERS OF OUTFLOW HYDROGRAPH IMMEDIATELY DOWNSTREAM OF DAM

PARAMETER ************************	UNITS	VARIABLE	VALUE ******
TIME TO FAILURE	HR	TFH	.500
TIME TO START OF RISING LIMB OF HYDROGRAPH	HR	TFO	.810
TIME TO PEAK	HR	TP	24.000
TIME STEP SIZE	HR	DTHI	.019

ROUTING COMPLETED.

KTIME= 139 ALLOWABLE KTIME= 699 TT= .8

PROFILE OF CRESTS AND TIMES FOR Lake Petit Dam BELOW Emerg. Action Plan

DISTANCE FROM DAM MILE	MAX ELEV FEET *****	MAX FLOW CFS *****	TIME MAX ELEV-HRS *****	MAX VEL FPS ******	FLOOD ELEV FEET	TIME FLOOD ELEV-HRS
.000	1647.99	225253	.000	4.85	.00	.00
.010	1573.75	225253	.415	10.47	.00	.00
.026	1573.30	224588	.419	10.13	.00	.00
.042	1572.80	224134	.419	9.72	.00	.00
.058	1572.26	223785	.419	9.87	-00	.00
.074	1571.66	223415	.419	10.06	.00	.00
.090	1571.00	223027	.419	10.28	.00	.00
.106	1570.25	222618	.422	10.53	.00	.00
.122	1569.42	222188	.422	10.84	-00	.00
.138	1568.46	221823	.422	11.24	.00	.00
.154	1567.35	221441	.426	11.79	-00	.00
.170	1566.04	221077	.426	12.78	.00	.00
.224	1562.29	220031	.430	12.07	.00	.00
.278	1558.11	219241	.430	11.86	.00	.00
.332	1553.61	218685	- 433	12.35	.00	.00
.386	1548.18	218370	.437	13.56	.00	.00
-440	1538.49	218198	.437	17.69	.00	.00
.500	1530.55	217791	-448	14.21	.00	.00
.560	1524.87	216161	.470	11.78	.00	.00
.620	1521.66	212547	.481	9.53	.00	.00
.680	1519.81	207154	.485	8.04	.00	.00
.730	1518.17	202213	.493	15.41	.00	.00
.780	1516.32	198214	.496	678.21	.00	.00
.830 .880	1514.17 1511.56	194569 191643	.500 .504	23.92 12.35	.00	.00 .00
.930	1508.17	189187	.504	14.82	.00	.00
.980	1503.47	188110	.511	52.90	.00	.00
.990	1498.02	188110	.515	*****	.00	.00
1.050	1440.73	187902	.504	263.91	.00	.00
1.100	1474.10	187009	.533	33.16	.00	.00
1.150	1472.01	185210	.537	98.88	.00	.00
1.200	1429.45	184534	.511	64.78	.00	.00
1.250	1460.64	183271	.556	9.45	.00	.00
1.300	1458.53	180525	.563	9.61	.00	.00
1,350	1456.87	177649	.567	9.04	.00	.00
1.400	1455.56	174969	.567	6.53	.00	.00
1.480	1451.95	172268	.574	10.01	.00	.00
1.530	1448.51	171083	.581	10.35	.00	.00
1.580	1444.83	169890	.585	10.68	.00	.00
1.630	1440.84	168703	.589	10.98	.00	.00
1.680	1436.56	167644	.593	11.28	.00	.00
1.730	1432.29	166790	.596	11.58	.00	.00
1.780	1428.02	166109	.600	11.90	.00	.00
1.830	1423.73	165584	.604	12.24	.00	.00
1.880	1419.42	165159	.607	12.63	.00	.00
1.930	1414.63	164765	.611	13.22	.00	.00
1.984	1409.65	164317	.619	12.95	.00	.00
2.038	1404.72	163797	.622	12.67	.00	.00

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PROFILE OF CRESTS AND TIMES FOR Lake Petit Dam BELOW Emerg. Action Plan

MILE FEET CFS ELEV-HRS FPS	FLOOD ELEV FEET ******	TIME FLOOD ELEV-HRS ******
2.092 1399.91 163110 .633 12.40	.00	.00
2.146 1395.38 162188 .641 12.11	.00	.00
2.200 1391.28 160903 .648 11.97	.00	.00
2.243 1389.18 159642 .652 10.28	.00	.00
2.287 1387.74 158460 .656 8.44	.00	.00
2.330 1386.73 157479 .656 6.90	-00	.00
2.340 1379.03 157479 .674 9.62	.00	.00
2.527 1369.62 151707 .696 9.29	.00	.00
2.713 1361.17 147058 .715 9.27	.00	.00
2.900 1350.53 144487 .730 9.83	.00	.00
2.950 1348.55 144036 .730 7.36	-00	.00
3.000 1345.94 143793 .733 7.93	.00	.00
3. 010 1324.73 143793 .622 50.52	.00	.00
3.020 1339.38 143748 .748 21.29	•00	.00
3.030 1338.41 143615 .752 20.58	.00	.00
3.040 1337.45 14 3 469 .752 20.11	.00	.00
3.050 1336.51 143334 .756 19.75	.00	.00
3.060 1335.59 143185 .756 19.36	.00	.00
3.070 1334.69 143040 .759 19.01	.00	.00
3.080 1333.80 142892 .759 18.64	.00	.00
3.090 1332.93 142730 .763 18.24	.00	.00
3.100 1332.08 142582 .763 17.77	.00	.00
3.110 1331.25 142422 .767 17.42	.00	.00
3.120 1330.44 142254 .767 16.96	.00	.00
3.130 1329.64 142096 .770 16.51	.00	.00
3.140 1328.86 141924 .770 16.04	.00	.00
3.150 1328.09 141751 .774 15.57	.00	.00
3.160 1327.34 141583 .774 15.06	.00	.00
3.170 1326.60 141402 .778 14.56	.00	.00
3.180 1325.87 141225 .778 14.05	.00	.00
3.190 1325.16 141048 .781 13.57	.00	.00
3.200 1324.45 140859 .781 13.20	.00	.00
3.210 1323.76 140678 .785 12.98	.00	.00
3.220 1323.07 140494 .785 12.82	.00	.00
3.230 1322.38 140299 .789 12.82	.00	.00
3.240 1321.70 140113 .789 12.83	.00	.00
3.250 1321.02 139923 .793 12.85	.00	.00
3.260 1320.34 139722 .796 12.90	.00	.00
3.270 1319.66 139533 .796 12.98	.00	.00
3.280 1318.98 139337 .800 13.10	.00	.00

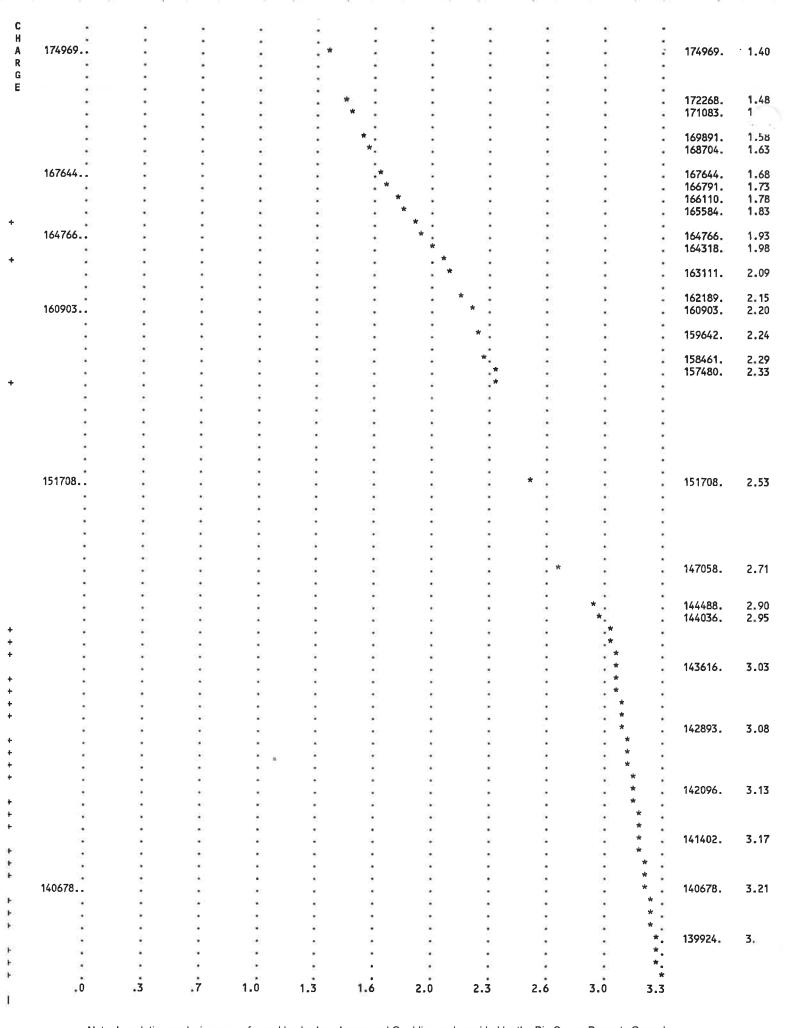
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	::•1:	•%					1.0	. W		(.	.1386.7	2.3
	•	•	9		•	•		*	3 4		•	
	3*1			:				*			.1379.0	2.3
				*		(6)						
	47/0 /2	(• ()				5905	(*)			S • S	. 17/0 /	2 -
	1369.62.	•			3	•	•	•	•	•	.1369.6	2.5
		300				160	8.40	· ·		0. 0 0	ž.	
	3		*	8		(*)			* *	*	.1361.2	2.7
	1.00		•			90	0.00			(0 . 00)		
				:							*	
		3.00	*	*		*				*	.1350.5	2.9
	7.	•		•		•	(*)	•		*	.1348.6	2.9
	**	9.6			*	80	127			:•:"	.1345.9	3.0
	1339.38.		ž.,							*	.1339.4	3.0
		•	•	×	36	36		•	4	:w: ★	*	
i			•		•				*	*	.1337.5	3.0
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	1334.69.			*		•				*	.1334.7	3.1
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	1326.60.	***			*	((♠)); 20-17	0.00	*			* .1326.6 *	3.2
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	•	•	(0)	•		• ,					* .1324.5	3.2
		((*));	S•0	*	*	(*)	7000 1024	*	*		* * *	
1	:			:	÷		•	:	:	•	* *	
		346	1000		ě	×				9.00	* * .1321.7	3.2
	*	•	(*)	•			•	8		•	*.	
		7 . C	15 € 5 5 ≙ 0		*		(A)			500 526	*	
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	.0	.3	.7	1.0	1.3	1.6	2.0	2.3	2.6	3.0	3.3	

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221441			*		•	•		•	. €	•	•			222189.	.12
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* 19821478 * 19821478 * 19457083 * 19164488 18918793 18811198 * 187905. 1.05 187005. 1.10 185211. 1.15 184534. 1.20 * 183272. 1.25			*		0	*	78.2	**	•	9 6 12	5 8 0				
* 19821478 * 19821478 * 19457083 * 19164488 189187		207155.	•		9 9	÷).).	•	:).5.1 (*)	A. 10 (1) (1 (4))			207155.	.68
* 19821478 * 19457083 * 19164488 18918793 18811198 * 187903. 1.05 187009. 1.10 185211. 1.5 * 185212. 1.25			G.		Al S	•		•	•	•			3		
* 19821478 * 19457083 * 19164488 18918793 18811198 * 187903. 1.05 187009. 1.10 185211. 1.5 * 185212. 1.25				1.5	9). 9).			190		u t i Nam	%•5 ?%₹				
* 19821478 * 19457083 * 19164488 18918793 18811198 * 187903. 1.05 187009. 1.10 185211. 1.5 * 185212. 1.25			•			ě	•	ı.B		•	y•				
* 19821478 * 19457083 * 19164488 18918793 18811198 * 187903. 1.05 187009. 1.10 185211. 1.5 * 185212. 1.25					K. N	*	:	3.€3 7.€3		4	(e) 78	: :	7.0		
189187. * 19164488 189187. * 18918793 18811198 * 187903. 1.05 187009. 1.10 * 185211. 1.15 184534. 1.20 * 180526. 1.30						*	•	(%) (*)	*		(10) (10)			202213.	.73
189187. * 19164488 189187. * 18918793 18811198 * 187903. 1.05 187009. 1.10 * 185211. 1.15 184534. 1.20 * 180526. 1.30						2	•	449	•	*	100	•	•		
189187. * 19164488 189187. * 18918793 18811198 * 187903. 1.05 187009. 1.10 * 185211. 1.15 184534. 1.20 * 180526. 1.30								3.€C	*	*					
189187. * 19164488 189187. * 18918793 18811198 * 187903. 1.05 187009. 1.10 * 185211. 1.15 184534. 1.20 * 180526. 1.30			•	•		* *	*		*	®			•	108214	78
189187 *								•	i	•				770214.	.,0
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189187 *			:				:	•	:	;	•	: :	:		
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189187							•	•		•					
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*								:		:		: :		191044.	.88
*						•	<u>*</u>	•	•	•	•				
*		189187.				. *		•			1.00 1.00			189187.	. 93
185211 *	-41		•			•	k .				50) 3 1 :			188111.	.98
185211 1.15 184534. 1.20 * 183272. 1.25	Ħ						* - *	*	•		1 6 5 GR			187903	1 05
185211 1.15 184534. 1.20 * 183272. 1.25							. *				(5) 18)			187009.	1.10
*		185211				•	· *	•	•	•	•		•	185211	
*						•	÷ *			:		: :		184534.	1.20
D : 180526. 1.30			•	*		•)	*	•	•	•	•				
						···			ing:	•		* *		103212.	1.40
			•	•		•	•	•		•	•9 5:				
				:		•v •v			•			: :	:	180526.	1.30
D			•			•		•	(10)	*	7 (
I	D	5 5		:		•		·	•						
	I	i i	•0				(*)	.*	•	•	• 1	•	•	177649.	1.35



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1.2

1455.6

1460.6

GAGE ZERO = 1530.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1647.98 FEET FLOOD STAGE NOT AVAILABLE

		MAX STAGE =	225253 CFS	AT TIME = AT TIME =	.000 HOURS
E	STAGE	FLOW			

TIME	STAGE	FLOW	_					
HR	FEET	CFS	0	50000	100000	150000	200000	250000
.00	118.0	1100	*		•	•		•
.02	118.0	1400	*	•	•			0.00
.04	117.9	2635						1161
.06	117.9	5366	.*.			•	•	
.08	117.8	9664	. "	3.0	•	•	540	(•)
.10	117.7	15858	. *				•	
.12	117.4	24096	•	**:			1,000	.(*:
.14	117.1	34207	•	T	•		1.0	•
.16	116.6	46266	•	*,	*		(*)	9.5
.18	116.0	60097	•	∞ *			3.60	
.20	115.2	75855	•		* .*			
.22	114.1	93104	•	3.€6			(*)	(*)
.24	112.8	111586	•	•	• *	. 9		
.26	111.2	130717	•	**	160	* •	(*)	(9€)
.28	109.3	150167	•	•		*		
.30	106.9	169111	•				*	
.32	104.1	186788	•		10.00	*	* *	
.34	100.8	202388	•	•	(*)		*	
.36	96.9	214908	•		8.01	* :		
.38	92.3	222881	•	•	•	9		*
.40	86.7	224884	•		3.5	•	2.	*
.42	80.2	220766	•	· ·	(9€)		•	* :00
-44	72.3	206897	•				**	•
.46	62.3	180378	•		0.€3		* .	10.00
.48	49.2	136350	•	•		* *		
.50	33.6	54294	•	×*	9.5	•		9.0
.52	22.6	14978	*		300	€	•	200
54	15.7	7954	* *		3.6	9		•
8	9.9	3870	.*		(9•0)	*/		
.58	5.7	1660	*	*	•	•		•
.60	3.4	1100	*		0.00			3.63
.62	1.6	1100	*		9.0	146		•
.64	.7	1100	*		163	. 10		(*)
.66	.7	1100	*		360			
.68	.7	1100	*		•			· ·
.70	.7	1100	*		13€);	(*)		3€3
.72	.7	1100	*	€	1			•
.74	.7	1100	*			(*)		3 .€ 3:
.76	.7	1100	*	¥	₹	1160		
.78	.7	1100	*					
.80	.7	1100	*			(*)		200

GAGE ZERO = 1529.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1573.75 FEET FLOOD STAGE NOT AVAILABLE

MAX STAGE = 44.75 FEET AT TIME = .415 HOURS MAX FLOW = 225253 CFS AT TIME = .389 HOURS

		MAX FL	.UW =	227273	CFS	ΑΙ	IIME =	.389 HU	JUKS .
TIME	STAGE	FLOW							
HR	FEET	CFS	0	50000	100000		150000	200000	250000
.00	1.0	1100	*		100000		150000	200000	250000
.02	1.2	1400	*						200
.04	1.7	2635	.*		- 2		- 0		-
.06	2.6	5366	*					- 1	95.5
.08	3.8	9664	. *						120
.10	5.2	15858	. *						(*)
.12	6.9	24096		*					
.14	9.1	34207		*					
. 16	12.0	46266		*					
.18	15.6	60097		ingi 🔻					
.20	19.5	75855			W		*		
.22	23.5	93104		3.60	* .				
.24	27.1	111586	•			*		÷	· .
.26	30.5	130717			(€)		* .		90
.28	33. 5	150167					*	•	
.30	36.1	169111					•	*	
.32	38.5	186788		0.00	×		•	*	
.34	40.6	202388						*	
.36	42.4	214908		:•:	0.00				*
.38	43.7	222881							*
.40	44.5	224884							*
.42	44.7	220766						*	*
.44	44.2	206897	•	•				. *	
.46	42.4	180378		: * :	0.00			* :	
.48	39.0	136350					*		
.50	31.5	54294		1 m 🕏			(*)	*	*
.52	22.7	14978	. *	91					
.54	16.0	7954	. *		(*)		10.00		
.56	10.5	3870	*	:•	(*)		(₩)	¥1	· ·
.58	5.9	1660	*						
.60	2.5	1100	*		(*)			*:	*
.62	1.0	1100	*		37		•	•	
.64	1.0	1100	*		9.0		3.00	•	
.66	1.0	1100	*						
.68	1.0	1100	*						
.70	1.0	1100	*					**	
.72	1.0	1100	*		9		(*)	•	•
.74	1.0	1100	*		(*		(0.0)		
.76	1.0	1100	*	•					
.78	1.0	1100	*					•	*
.80	1.0	1100	-	*			(.€0)	¥:	*

GAGE ZERO = 1430.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1503.47 FEET FLOOD STAGE NOT AVAILABLE

MAX	STAGE =	73.47	FEET	AT TIME	=	.511 HOURS
MAX	FLOW =	188111	CFS	AT TIME	Ξ	.507 HOURS

TIME	STAGE	FLOW										
HR	FEET	CFS	0		50000	•	100000	150	000	200	000	250000
.00	33.4	2613	.*				*					
.02	30.3	5121	.*						20			
.04	25.7	4811	.*									
.06	23.6	2317	*				100		4		8	7
.08	20.8	2631	, w						7.0			
.10	16.6	2779	_*				741		100			
.12	13.6	1726	*		4						-	1.5
- 14	11.6	1523	*								ĵ.	-
.16	10.0	1801	*						. 5			
.18	29.0	69005				*			1000			
.20	43.8	15079	700	*			4		020		- 0	5 7
.22	42.4	16540		*	·				West of the second		-	
.24	44.2	17191		*	- 0		-		terior		•	
.26	45.8	23296			* :				-		2	/2
.28	47.6	31563			* 0				1850 1960			
.30	49.5	41665			₩ ੂ				525		•	ě
.32	51.7	54403			· w		75		3.50		•	
.34	54.6	71766	3.0		•	*			1992			•
.36	58.3	94191					*		•			·
.38	62.5	118573	(40)				•	*	8.8		*	
.40	66.0	139640	020					,	A on			
.42	68.6	155628	•		•		•		*		•	•
.44	70.3	166552			*		*		:*0	k	•	
.46	71.6	175582					•		•	*	•	
.48	72.7	183127			•				0.00	*	•	
.50	73.4	187740			- 20				•			•
.52	73.4	186728			*1		•		•		*	
54	72.5	178620	•		•		•		•		•	•
6	70.7	164887			**		*			 k	•	
8د.	68.4	148320					*		***	-		•
.60	65.1	126451			•		•		-		•	•
.62	61.6	106542			100		*_	•			190	*
.64	58.2	89562	•		2.40		* "		•		•	
.66	55.3						•		•		•	
		76051			:(*)	. ~	*		•		(*)	•
.68 .70	52.5 50.0	65301	•		1	-			•			
		55438	3.0				*		9.			
.72	47.4	45864	•		. *.		•				•	
.74	44.6	36657	•		. (1)		•		2			
.76	41.7	28345	*	4	2.00		*		*			
.78	38.6	21912	٠	_ 	•		•		•		•	
- 80	35.7	17148		*								

GAGE ZERO = 1330.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1386.73 FEET FLOOD STAGE NOT AVAILABLE

MAX	STAGE =	56.73	FEET	AT TIME	=	.656	HOURS
MAX	FLOW =	157480	CFS	AT TIME	=	-644	HOURS

		PIAK I COM -	137400 (ars Ar	IIME -	.044 110	UNS
TIME	STAGE	FLOW					
HR	FEET	CFS 0	50000	100000	150000	200000	250000
.00	7.5	915 *	2000		150000	200000	250000
.02	8.6	1132 *					
.04	9.1	1227 *	120	- 5	- 5	- 1	
.06	9.3	1286 *				52.0	350
.08	9.5	1326 *			- 0		
.10	9.6	1330 *		-			100
.12	9.5	1326 *					500
. 14	9.5	1325 *					700
.16	9.5	1313 *					
.18	9.5	1326 *					-
.20	9.7	1351 *					
.22	9.7	1353 *					3.00
.24	9.8	1373 *	7.0				38
.26	9.9	1393 *	•				
.28	10.0	1420 *	(4)		•		100
.30	10.2	1471 *					300
.32	10.8	1600 *					
.34	12.1	1889 *					
.36	14.3	2430 *	100				
.38	17.3	3846 .*	(*)	0			
.40	20.7	7632 . *		7.0			(*)
.42	23.8	13680 . *	340				
.44	26.7	21861 .	k 🧠				
.46	29.6	33046 .	*				
.48	32.9	47341 .	*.		ş.		
.50	36.7	64808 .	*	0.	•		
.52	41.0	84332 .	190	*			
.54	45.3	104049 .		*	•		
.56	49.0	121940 .			*		
.58	51.9	136965 .	1.		* :		
.60	54.2	147251 .			*.		•
.62	55.7	154076 .			.*		
.64	56.5	157351 .		1000	.*		
.66	56.7	156324 .			.*		(4)
.68	56.4	151808 .			*		
.70	55.6	144971 .			*		70•00
.72	54.6	136606 .		(*)	* *		100
.74	53.3	127580 .			*		
.76	51.8	118358 .		540	*		
.78	50.2	109405 .	9	* *			
.80	48.6	100733		*	•	*	/ * 2

GAGE ZERO = 1295.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1345.94 FEET FLOOD STAGE NOT AVAILABLE

		1141/ 67		D SINGE	NOT AVAIL				
		MAX ST		50.94	FEET	AT TIME =		HOURS	
		MAX FL	- WO.	143793	CFS	AT TIME =	· .733	HOURS	
TIME	STAGE	FLOW							
HR	FEET	CFS	0	50000	100000	150000	200000	250000	
.00	5.0	505	*				ii		
.02	5.0	507	*				6		
.04	5.1	51 7	*						
.06	5.3	550	*			7	8		
.08	5.7	615	*	3					
.10	6.3	718	*				70		
.12	7.0	837	*	9					
- 14	7.6	951	*				SO 15		
. 16	8.1	1045	*		- ŝ			•	
.18	8.5	1121	*		•				
.20	8.8	1178	*		•	fh 6*		•	
.22	9.0	1219	*	•	•	/•		·	
.24	9.2	1247	*	•			(*		
.26	9.3	1267	*	•	•				
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GAGE ZERO = 1278.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1318.98 FEET FLOOD STAGE NOT AVAILABLE

MAX STAGE = 40.98 FEET AT TIME = MAX FLOW = 139337 CFS AT TIME = _800 HOURS .767 HOURS

		MAX FLOW	=	137337	CFS	Al lime =	./6/ HU	UK5
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.16	2.3	754 *		190	(4)			
.18	2.4	873 *			7.			
.20	2.6	980 *			(*)			
.22	2.7	1073 *				*		
.24	2.7	1141 *			35			
.26	2.8	1195 *					•	
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.30	2.9	1257 *		9.0	343			*
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* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104

Full Microcomputer Implementation

by

Haestad Methods, Inc.

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RIIMP- AND -RIIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRANZ7 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

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U.S. ARMY CORPS OF ENGINEERS HYDROLOGIC ENGINEERING CENTER 609 SECOND STREET DAVIS, CALIFORNIA 95616 (916) 756-1104

Big Canoe - PMP Flood Hydrographs

PRINT CONTROL PLOT CONTROL HYDROGRAPH PLOT SCALE	MINUTES IN COMPUTATION INTERVAL STARTING DATE STARTING TIME NUMBER OF HYDROGRAPH ORDINATES ENDING DATE ENDING TIME CENTURY MARK	0.10 HOURS 6.90 HOURS	SQUARE MILES INCHES FEET CUBIC FEET PER SECOND ACRE-FEET ACRES DEGREES FAHRENHEIT
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TIME DATA FOR INPUT TIME SERIES
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SUBBASIN AREA SUBBASIN CHARACTERISTICS TAREA 1.53

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BASIN TOTAL PRECIPITATION 41.00 PRECIPITATION DATA STORM

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UNITGRAPH DIMENSIONLESS SCS

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PERCENT IMPERVIOUS AREA

INITIAL ABSTRACTION

CURVE NUMBER

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CRVNBR RT I MP

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807. 164. 27. 5.

Pettit Lake 6-Hour PMP ***** *********** 궃

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TIME INTERVAL IN MINUTES SCALE PLOT CONTROL HYDROGRAPH PLOT PRINT CONTROL SERIES TIME OUTPUT CONTROL VARIABLES 40 DATA FOR INPUT JXMIN JXDATE JXTIME IPLOT QSCAL TIME

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SUBBASIN RUNOFF DATA

STARTING DATE STARTING TIME

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SUBBASIN CHARACTERISTICS

BA

38

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's Association for incorporation into this Emergency Action Plan.

| SUBBASIN RUNOFF DATA | SUBBASIN CHARACTERISTICS
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0.00 0.00
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0.00 0.00 | LOSS RATE 1.33 STRTL 1.33 CRVNBR 60.00 RTIMP 0.00 | DIMENSIONLESS UNITGRAPH
TLAG 0.85 LA | | 189.
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JXTIME 0 | SUBBASIN RUNOFF DATA | SUBBASIN CHARACTERISTICS |
|----------------------|--|--------------------|---------------------------|--|--|---|-----|--|---|------------------------|--|--|----------------------|--------------------------|
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| | AREA | | BASIN TOTAL PRECIPITATION | TERN 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0. | INITIAL ABSTRACTION
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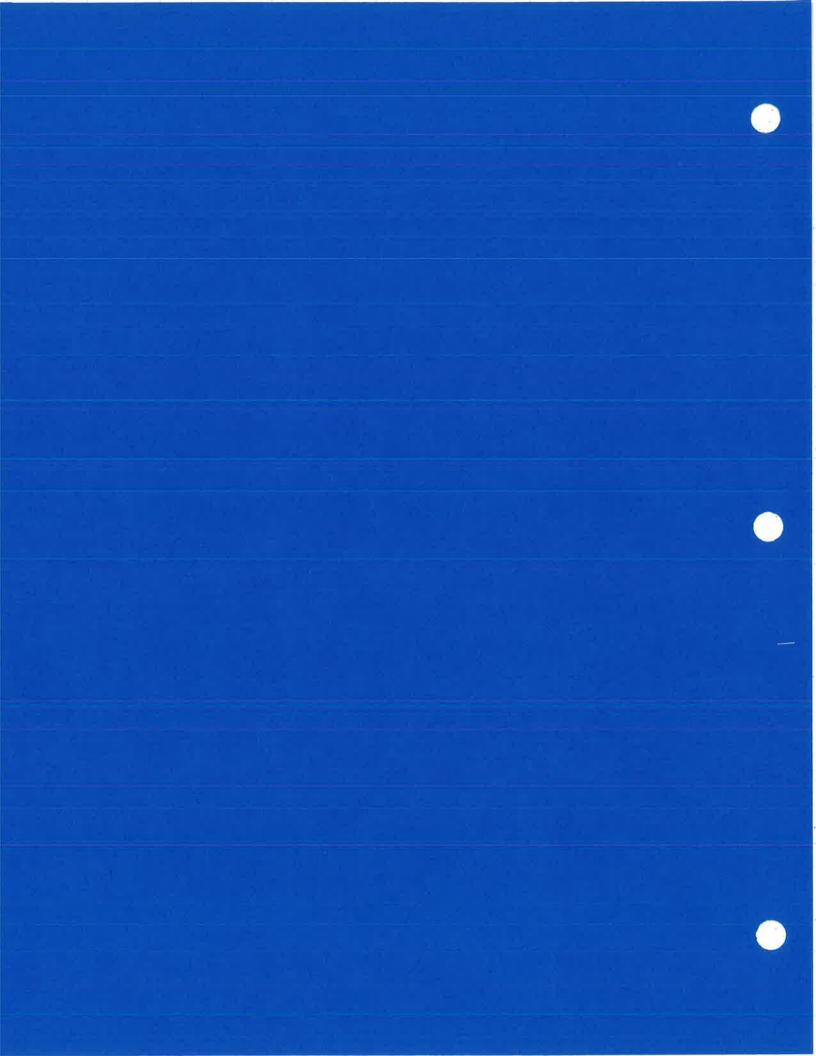
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0.08 | 0.01 | 0.01 | 0.01 | 0.02 | 0.02 | 0.02 | 0.03 | 0.05 | 0.39 |
|--------|------|--------------------|--|-------------|---|----------------|-----------|------|------|-------|------|
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LAG | | | | | | | |
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| | 16. | 39. | - 77 | 122. | 171. | 234. | 303. | | | | |
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| | 849- | 810. | 768. | 724. | . 699 | 611. | 546. | | | | |
| | 356. | 324. | 294. | 266. | 245. | 245. 225. 205. | 205. | 187. | 170. | 154. | |
| | 138. | 127. | 116. | 105. | 95. | 87. | 62 | | | | |
| | 53. | 48. | 44- | . 04 | 36. | 33. | 30. | | | | |
| | 21. | 19. | 17. | 16. | 14. | 13. | 12. | | | | |
| | 6 | ω | 7. | 7. | 9. | 5. | 5. | | | | |
| | 2. | 2. | <u>.*</u> | - | ° | | | | | | |

RUNOFF SUMMARY FLOW IN CUBIC FEET PER SECOND TIME IN HOURS, AREA IN SQUARE MILES

| TIME OF
MAX STAGE | 1.53 | 1.53 | 3.30 | 3.30 |
|-------------------------|---------------|---------------|---------------|---------------|
| MAXIMUM
STAGE | 3657. | 3354. | 5848. | 6941. |
| BASIN
AREA | 3657. | 3354. | 5848. | 6941. |
| FLOW FOR MAXIMUM PERIOD | 4203. | 3857. | 6724. | 7982. |
| OW FOR M | 06.9 | 2.90 | 06.9 | 3.80 |
| AVERAGE FL | 7100. | 14624. | 13294. | 18830. |
| TIME OF
PEAK | PT24 | PT06 | SC24 | 9008 |
| PEAK
FLOW | | | | |
| STATION
72-HOUR | - | _ | - | _ |
| OPERATION
24-HOUR | HYDROGRAPH AT | HYDROGRAPH AT | HYDROGRAPH AT | HYDROGRAPH AT |
| 6-HOUR | | | | |

*** NORMAL END OF HEC-1 ***

APPENDIX C DAM FEATURES AND PHOTOGRAPHS





N:\B\Big Canoe\Projects\TN6338 - 2017 EAP Lake Petit Dam\GIS\MXD\Dam_Features_mxd 5/16/2017 8:22:44 AM

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Geosyntec consultants

PHOTOGRAPH LOG

| PI | ROJECT NAME: Emergency Action Plan for Dam Failure, Lake Petit Dam | PROJECT NO.: TN6338 |
|----|--|----------------------------|
| С | LIENT: Big Canoe Property Owner's Association | FILE NAME: AppD_Photos.ppt |



Photograph 1: View of the low-level valve box



Photograph 2: View of the low-level valve box.

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Geosyntec consultants

PHOTOGRAPH LOG

| PROJECT NAME: Emergency Action Plan for Dam Failure, Lake Petit Dam | PROJECT NO.: TN6338 |
|---|----------------------------|
| CLIENT: Big Canoe Property Owner's Association | FILE NAME: AppD Photos.ppt |



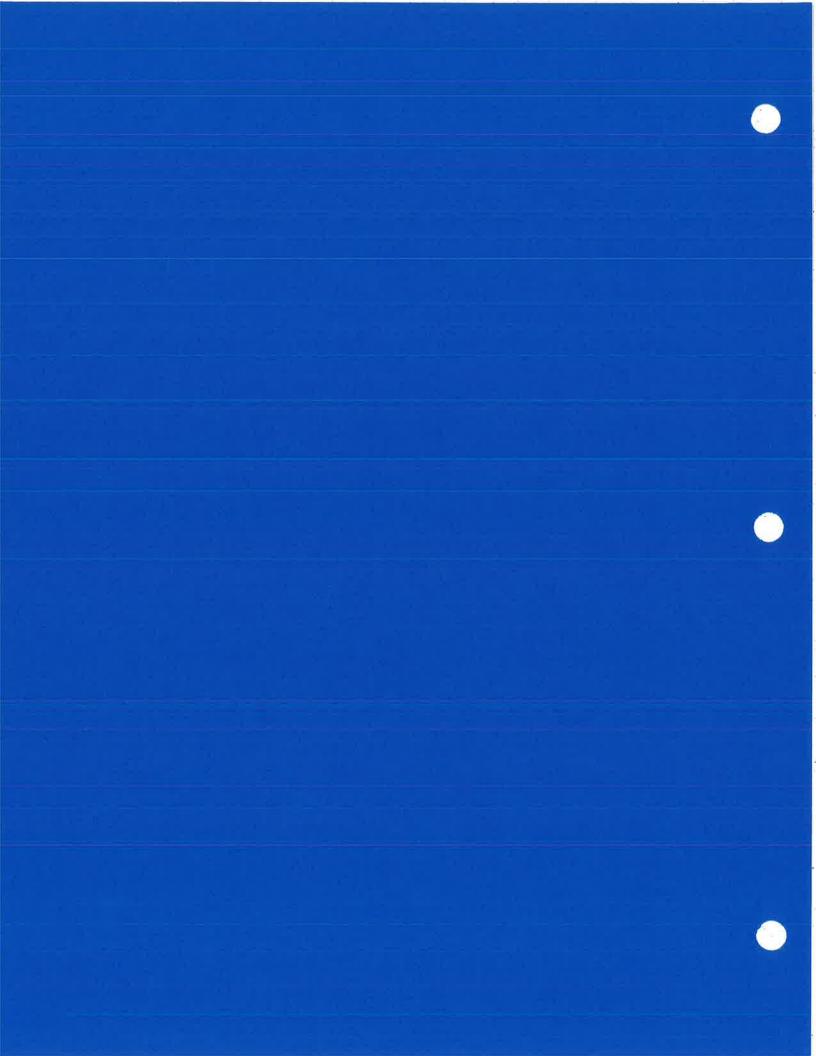
Photograph 3: Tail Water Creek, 2016 – Low-level drain outlet.



Photograph 4: Spillway, 2016 – general view with moderate flow during observation

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APPENDIX D EAP DISTRIBUTION AND ACCEPTANCE



APPENDIX D EAP DISTRIBUTION

The following entities maintain a copy of the EAP. Contact information can be found on Figure 1. The EAP should be signed by all parties, including those listed below, involved in plan implementation on the following pages.

- 1. Big Canoe Property Owner's Association
- 2. Big Canoe Public Safety Director
- 3. Pickens County Emergency Management Agency (EMA)
- 4. Dawson County 911 Service
- 5. Big Canoe Engineer of Record
- 6. Georgia Department of Natural Resources (GA DNR)
- 7. Georgia Safe Dams Program
- 8. Pickens County Sheriff
- 9. Pickens County Public Works
- 10. Cox Lake Dam Owner, Cove Lake Property Association

| Organizatio | Sn: 0 0 1 0 |
|-------------|--------------------------------------|
| B | ig Canoe Proporty Owners Association |
| Name: _ | Jason Brownell |
| Title: |): rector of Operations |
| Signature: | Jason Souroll |
| Date: | 0/16/18 |

| Organization: Fig Cause 7 | Eble Salty |
|----------------------------|----------------------|
| Name: | Jordan |
| Title: Ruble Safety | Manager / Fire Chief |
| Signature: | |
| Date: 10/25/2018 | |

| Organization: PICHENS County EMA | |
|----------------------------------|--|
| Name: John Nicholson | |
| Title: EMA DIRECTOR | |
| Signature: July inholse | |
| Date: 007, 24, 2018 | |

| 0 / 1. 5. 15. |
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| Dawson County Fire & Emergancy Services |
| Name: |
| Dammy Thompson |
| Title: |
| Fire Chief EMA Director |
| Signature: |
| Date: 3-18-15 |

| Organization: GEOSYNTEC CONSULTANTS |
|--------------------------------------|
| Name: WESLEY MACDONALD |
| Title: PROJECT ENGINEER OF RECORD |
| Signature: July 172 |
| Date: 9/25/2019 |

| Organiza | ation: | | |
|-----------|-----------------------|-----------|--|
| | EPD- GEORGIA SAFE DAM | 3 PROGRAM | |
| Name: | Tom Woosley | | |
| Title: | PROGRAM MANAGEL | | |
| Signature | Jon Word | | |
| Date: | 812212018 | | |

| Organizat | ion: PILISENS SHEETERS OFFICE |
|------------|-------------------------------|
| Name: | DONALD E CANO |
| Title: | Sheniff. |
| Signature: | Dmulde fing |
| Date: | 05-29-19. |

EAP ACCEPTANCE

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake Dam and understand the agreed-upon responsibilities.

| 3043 CAMP ROAD JASPER, GA 30143 |
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EAP ACCEPTANCE

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake Dam and understand the agreed-upon responsibilities.

| Organizati | on: Core Lak | ie Pr | operty | Duneus | Assoc |
|------------|--------------|-------|--------|--------|-------|
| | Robert 1 | | | | |
| Title: | Board | 0 | | | |
| Signature: | Robet L | Kene | w | ı | |
| Date: | 10/26 | 1/ | | | |

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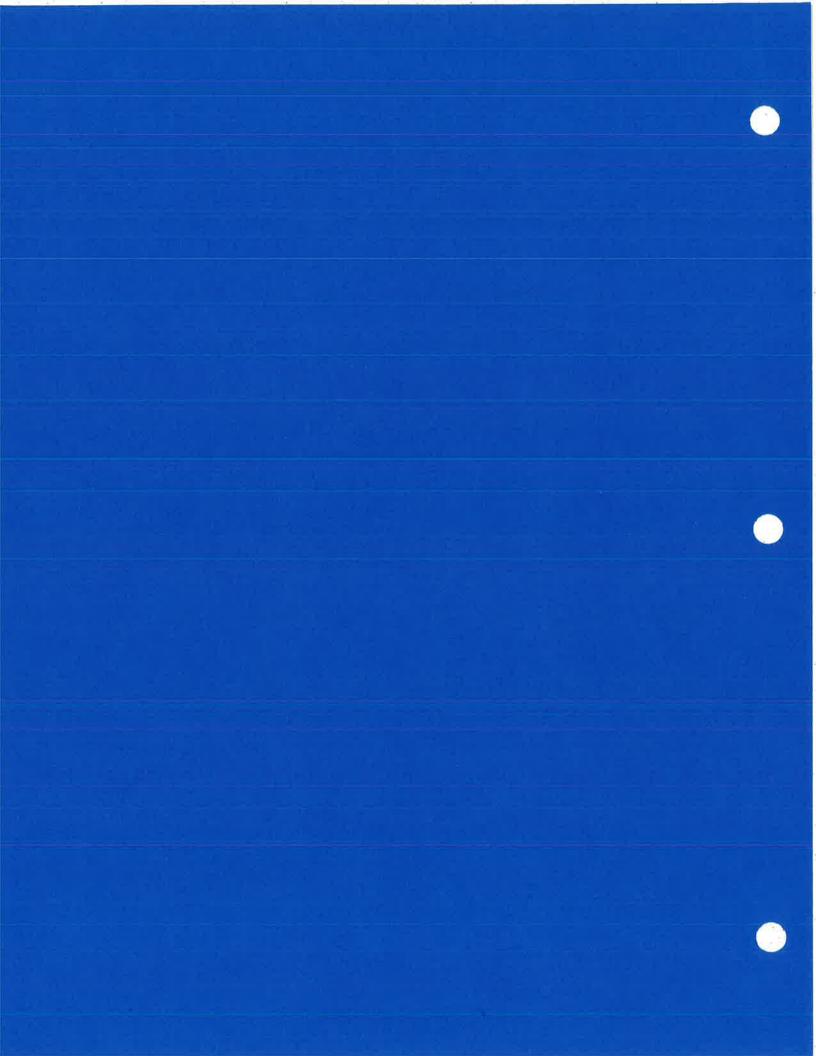
EAP ACCEPTANCE

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

| Dam and understand the agreed-upon responsibilities. |
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| Organization: |
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| Name: |
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| Title: |
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| Signature: |
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| Date: |
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APPENDIX E

EAP REVIEW, REVISIONS, AND TESTING DOCUMENTATION



APPENDIX E

EAP REVIEW, REVISION, AND PERIODIC TEST

EAP Annual Review

An Annual Review should be conducted by the Big Canoe Property Owner's Association. The annual review should be documented in this appendix.

EAP Periodic Test

A periodic test of the EAP procedures is recommended every 5 years. Documentation for the test is provided in this appendix.

Revision

Revisions required from annual reviews or periodic testing should be documented in this appendix. Revisions shall be made in all copies of the EAP as provided in Appendix D.

RECORD OF ANNUAL REVIEW

| Conducted By | Notes and Observations |
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| | Conducted By |

RECORD OF PERIODIC TESTING

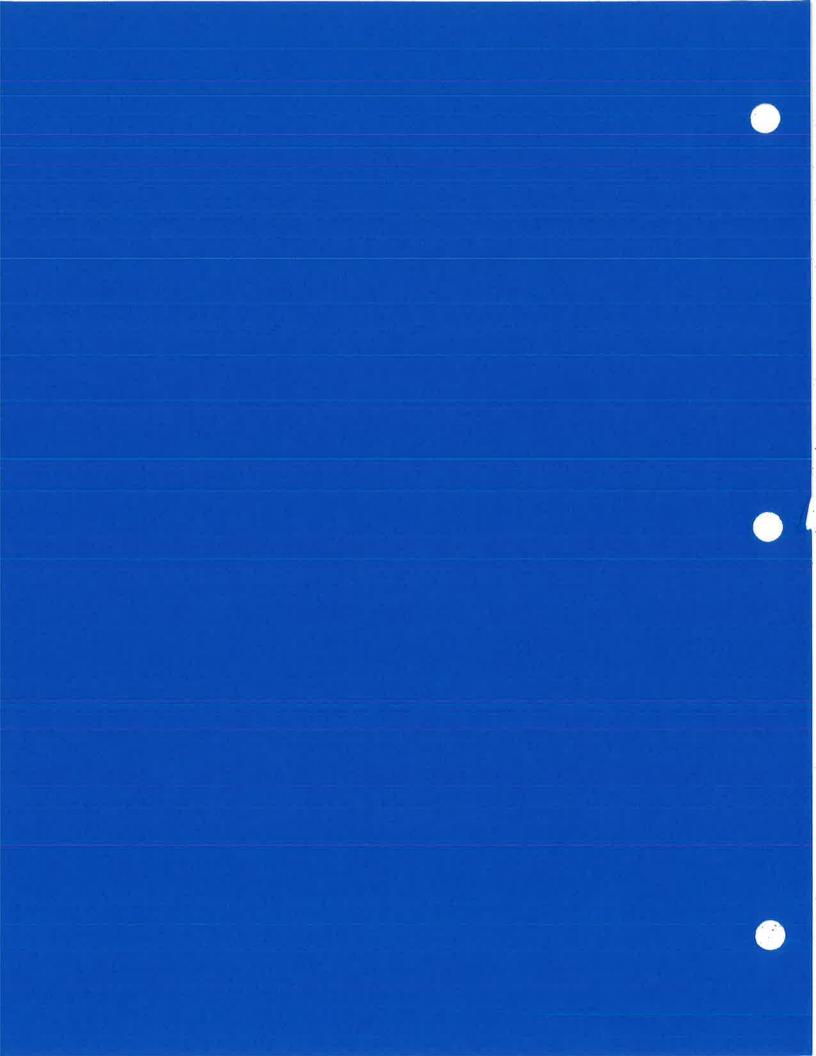
| Date | Conducted By | Observations |
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RECORD OF REVISIONS AND UPDATES

| Revision Number | Date | Revisions Made |
|-----------------|----------------|--|
| 0 | September 1998 | Original Issue |
| 1 | April 2007 | Contact information update |
| 2 | June 2017 | Contact Information, Added Condition C, Updated Appendices |
| 3 | May 2018 | Update Condition Identifications and added Table 3 |
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APPENDIX F LOCALLY AVAILABLE RESOURCES



APPENDIX F

LOCALLY AVAILABLE RESOURCES

The Pickens County Commissioner indicated the following heavy equipment may be available in the case of an emergency (for use, contact John Nicholson at Pickens County EMA at 706-253-8829 or Kim Quinton at the Pickens County Public Works Department at 770-893-9564):

- One (1) Caterpillar D6 Bulldozer;
- One (1) 330 Kobelco Excavator;
- One (1) Kubota 080 Excavator;
- One (1) Caterpillar 303 Excavator;
- One (1) Caterpillar 313 Rubber Tire Trackhoe;
- Two (2) Bobcat skid steers;
- Three (3) tandem dump trucks;
- Two (2) single axle box dump trucks; and
- Three (3) single axle flat bed trucks.

Other locally available resources include:

| Heavy Equipment Service and Rental | Sand and Gravel Supply | Ready-mix Concrete Supply | | |
|---|--|---|--|--|
| Sunbelt Rentals 5290 Lake Pointe Center Drive Cumming, Georgia Main: 770-887-9966 Emergency: 800-667-9328 United Rentals 1151 Northpoint Parkway SE Acworth, Georgia 30102 Main: 770-974-3500 (On call service available 24/7) | Bluegrass Quarry 970 Old Nelson Road Ball Ground, Georgia 30107 678-641-7714 Vulcan Materials Company 4420 Hightower Road Ball Ground, Georgia 30107 Main: 678-947-3310 GA Services: 770-454-3691 | Wayne Davis Concrete 115 River Mill Drive Ball Ground, Georgia 706-692-3464 Ernst Concrete 970 Old Nelson Road Ball Ground, Georgia 30107 770-853-0533 Argos Ready Mix 829 Univeter Road Canton, Georgia 770-704-7778 | | |
| Pumps | Diving Service | Sand Bags | | |
| Xylem Cartersville 402 Old Mill Road Cartersville, Georgia 30120 770-415-8814 (On call service available 24/7) United Rentals 5260 Truman Drive Decatur, Georgia 30035 Main: 404-439-4322 (On call service available 24/7) | Georgia Department of Natural Resources, Law Enforcement Division, Search and Rescue Team 781 Red Top Mountain Road SE, Acworth, Georgia 30102 770-529-2424 (M-F 8am – 4:30pm) 1-800-241-4113 (After hours) Underwater Construction Corporation 8494 Gulf View Drive Soddy Daisy, TN 37379 423-332-6700 | Hanes Geo Components
3105 Sweetwater Road, Shite 200
Lawrenceville, Georgia 30044
Main: 866-961-3565
Emergency: 678-221-7849 | | |

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