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**LAKE PETIT DAM**  
**Pickens County, Georgia**  
**State ID No. 112-009-00462**  
**NID No. GA00685**  
**Emergency Action Plan**

*Prepared for:*

**Big Canoe<sup>®</sup> Property Owners Association, Inc.**

10586 Big Canoe  
Jasper, GA 30143  
Pickens County

*Prepared by:*

**Geosyntec Consultants, Inc.**

835 Georgia Avenue, Suite 500  
Chattanooga, TN 37402

Project No: TN8667

Document No. GA220320

December 2022



**REVISION LOG**

| Revision No. | Effective Date | Affected Page Numbers | Description of Revision/Change  | Performed By                       |
|--------------|----------------|-----------------------|---|------------------------------------|
| 0            | September 1998 | All                   | Original Issue  | Jordan, Jones, and Gouilding, Inc. |
| 1            | April 2007     | -                     | Contact Information Update  | Jordan, Jones, and Gouilding, Inc. |
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## 1. INTRODUCTION

This Emergency Action Plan (EAP or document) was prepared by Geosyntec Consultants, Inc. (Geosyntec) of Chattanooga, Tennessee on behalf of the Big Canoe® Property Owners Association, Inc. (POA or Owner), Jasper, Georgia in accordance with the Rule for Dam Safety of Georgia (GA) Safe Dams Act of 1978.

The purpose of this EAP is to prescribe procedures to be followed in the event of an emergency associated with the Lake Petit Dam (Dam), which may be caused by an unusually large flood, earthquake, a malfunction (hydraulic or structural) of the spillway, malicious human activity such as sabotage, vandalism, or terrorism, or failure of the Dam.

This document establishes procedures for warning, evacuating, and protecting the public, and to protect property, which would be endangered in the event of a failure of the Dam; as well as taking timely action to notify the appropriate emergency management agency (EMA), law enforcement bodies, and/or governing officials of unusual, potential, or imminent events that might lead to failure of the Dam.

### 1.1 Ownership Information

Contact information for the Owner, Primary Operator of the Dam, and Dam Owner's Technical Representative is provided in this Section of the EAP. For emergency purposes, refer to the appropriate Notification Flowchart presented in Section 4 of this document.

#### Owner Information

Owner and Primary Operator: Big Canoe Property Owners Association, Inc.

Address: 10586 Big Canoe, Jasper, Georgia 30143

Daytime Phone #: (706) 268-3346

Emergency Phone #: Refer to the Dam Owner's Representative Emergency Phone.

#### Dam Owner's Representative

Dam Owner's Representative: Scott Auer

Address: 10586 Big Canoe, Jasper, Georgia 30143

Daytime Phone #: (706) 268-2400

Emergency Phone #: (770) 596-9003

#### Additional Contacts

Dam Owner's Technical Representative: Wesley MacDonald, P.E. GA, AL, KY, TN, and WA

Address: 835 Georgia Avenue, Suite 500, Chattanooga, Tennessee 37402

Daytime Phone #: (423) 385-2312

Emergency Phone #: (615) 830-5139

## 1.2 Dam Data Sheet

In this section of the EAP, high-level categorical data with regards to the Dam is provided in the table below (Table 1). For location and descriptions of pertinent dam features refer to the Lake Petit Dam Operations & Maintenance (O&M) Plan (Geosyntec, 2022).

**Table 1 – Dam Data**

|                              |   |                 |
|------------------------------|---|-----------------|
| <b>Dam Name:</b>             | Lake Petit Dam  |                 |
| <b>State ID:</b>             | No. 112-009-00462   |                 |
| <b>NID:</b>                  | GA00685   |                 |
| <b>Dam Owner/Operator:</b>   | Big Canoe POA   |                 |
| <b>Classification:</b>       | Category I  |                 |
| <b>Purpose of Dam:</b>       | Recreation and Water Supply   |                 |
| <b>Drainage Area:</b>        | 1.53 square miles   |                 |
| <b>Height:</b>               | 126 feet  |                 |
| <b>Year Constructed:</b>     | 1972  |                 |
| <b>Year(s) Modified:</b>     | 1974, 1976, 1997, 1998, 2008, 2009, and 2022  |                 |
| <b>Design Engineer/Firm:</b> | Baldwin & Cranston Associates (1971)  |                 |
| <b>GPS Location:</b>         | 34.4625 (North)   | -84.2903 (West) |
| <b>County:</b>               | Pickens   |                 |
| <b>Access to Dam:</b>        | The Dam is located within the private development owned and operated by Big Canoe Property Owner Associates, Inc., which is a gated, private residential community. The Dam can be accessed from the crest via Wilderness Parkway and from the toe via Wolfscratch Drive. |                 |

## 1.3 Project Description

The Dam was constructed in 1972 as a zoned earth embankment consisting of a central clayey silt core and predominantly silty sand embankment shells. The Dam was constructed to supply water and provide recreation for the Big Canoe development and is permitted as a Category I Dam under

Chapter 391-3-8 of the GA State Code Rules for Dam Safety. The Dam is owned, operated, and maintained by the Big Canoe POA. The original design drawings for the Dam were prepared by Baldwin & Cranston Associates (Baldwin & Cranston Associates, 1971).

The Dam is located within the Big Canoe development on Petit Creek approximately 5.8 miles upstream of Marble Hill, in Pickens County, north-central Georgia. The reservoir formed by the Dam has a surface area of 107 acres at a normal pool elevation of 1635.5 and extends up Petit Creek approximately 0.7 miles. The revised maximum water storage for the reservoir approved by the Georgia Safe Dams Program (GSDP) is 5,635 acre-feet (ac-ft). Table 1 presents additional dam data. The topography around the Dam consists of very steep, wooded, mountainous foothills.

The dam has a 15-foot-wide concrete cascading channel spillway on the east side of the earth dam's abutment. The spillway discharge is controlled by a concrete crest underneath a bridge located on the roadway (i.e., Wilderness Parkway) running along the crest of the Dam. The Dam has a 36-inch low-level discharge conduit which is the only permanent means of lowering the reservoir level beneath the spillway crest, short of structural excavations or the use of temporary pumps or siphons. The low-level discharge conduit is operated via a lift system used to operate the heavy-duty sluice gate, which is located under the hatch cover of the low-level discharge conduit gate operator vault.

Lake Sconti, owned by Big Canoe, is located approximately 1.0 mile downstream of Lake Petit. Lake Disharoon, owned by Big Canoe, is located approximately 0.1 mile upstream of Lake Sconti. Lake Petit does not directly recharge the reservoir at Lake Disharoon, however, in the event of an inundation, Lake Disharoon is in the inundation area of Lake Petit. Cox Lake Dam is located approximately 3.5 miles downstream from Lake Petit Dam, south of Cove Road, and is not owned or operated by Big Canoe.

## 1.4 Site and Pertinent Structure Access

Big Canoe has two permanent roadway access points, both of which have a guard shack with gate access: (1) the main entrance off Steve Tate Highway is Wilderness Parkway, which crosses Petit Creek approximately 2.3 miles downstream of Lake Petit Dam, and (2) the secondary (northern) entrance off Steve Tate Highway (also Wilderness Parkway) which crosses over the top of Lake Petit Dam (only staffed during the day). Access to the toe is via Wolfscratch Drive, a connector road that both starts and ends at different portions of Wilderness Parkway. A third access route to Big Canoe is through an unmanned, locked gate at the Big Canoe Golf Maintenance Area. These three (3) connections are the only exit points from Big Canoe to Steve Tate Highway. The use of these evacuation routes will be controlled by the Big Canoe Department of Public Safety to prevent the endangerment of evacuees in case of an imminent failure. To assist with access to the Dam, directions from Downtown Atlanta, Georgia to the Dam were sourced from Google Maps (Google, 2021) and are presented in Appendix A – Directions from Atlanta GA, to the Dam.

For emergency conditions that require the operation of the low-level discharge conduit, the gate operator vault for the low-level discharge conduit is located on the upstream face, near the center of the Dam at Station (STA) 4+70. This is just north of the present-day flagpole. A 30-inch (in.) x 30-in. heavy-duty sluice gate (inlet) was designed (Baldwin & Cranston, 1971) to be mounted on the low-level inlet structure that connects the low-level discharge conduit to the reservoir. The



low-level discharge conduit, and associated structures, are the only permanent means of lowering the reservoir level beneath a few feet below the sill of the spillway, short of structural excavations or the use of temporary pumps or siphons. The lift used to operate the heavy-duty sluice gate is located under the hatch cover of the low-level discharge conduit gate operator vault. The handle to operate the low-level discharge conduit lift is not kept on the structure. For access to the handle, please contact the Dam Owner’s Representative. An alternative handle may be located on a similar lift on the intake structure of Lake Disharoon Dam. Descriptions and locations of all other pertinent Dam features are discussed and presented in the O&M Plan (Geosyntec, 2022).

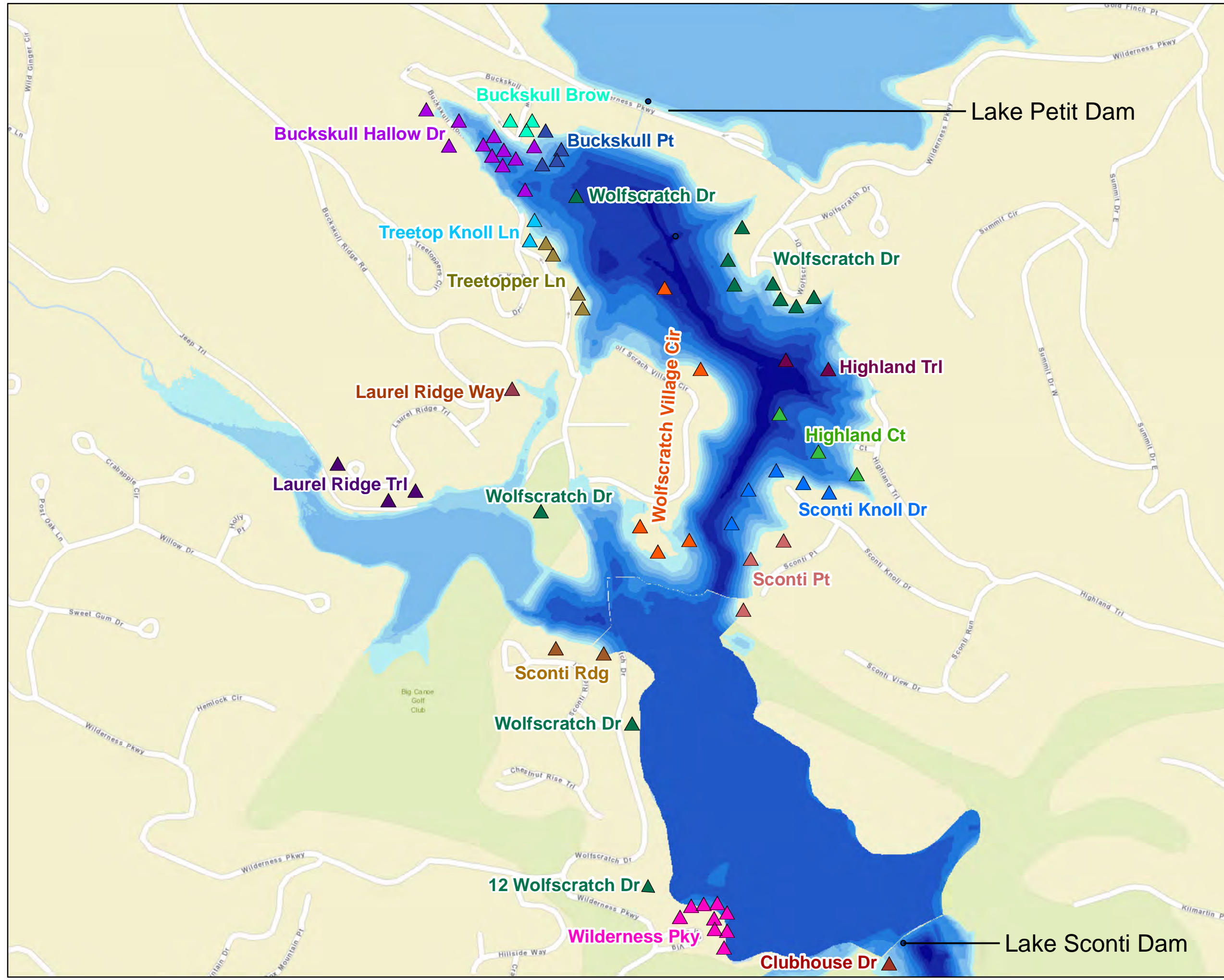
## 1.5 Document History

The EAP for the Dam was originally created and revised by Jordan, Jones, and Goulding, Inc. in 1998 and 2007, respectively. Geosyntec revised the EAP in 2017 and 2018 based on guidance from the GSDP Document Engineer Guidelines (Georgia Environmental Protection Division, 2015), and templates referred to in Engineer Guidelines created by the North Carolina Department of Environmental Quality and the Association of State Dam Safety Officials. A summary of known dates of revision of this EAP is provided in the Revision Log where future revisions and modifications to the EAP should be documented. Concurrences, Records of Holders of Control Copies, documentation from EAP reviews, and periodic tests of this EAP should be documented in the forms provided in Appendix B.

Geosyntec revised this EAP based on guidance from the GSDP template Emergency Action Plan Template (Georgia Environmental Protection Division, 2021); as well as guidelines prepared by the Federal Emergency Management Agency (FEMA) titled Federal Guidelines for Dam Safety (Federal Emergency Management Agency, 2013). The inundation mapping and calculations provided in Appendix C – Inundation Mapping were prepared in 2022 with more recent survey data, updated lake volume, and the updated requirements from the Engineer Guidelines (GA EPD, 2015). The property structures and addresses for properties downstream of the Dam were provided by the Pickens County EMA. Updated evacuation maps were prepared using the inundation calculations from the Inundation Mapping analysis. The evacuation maps are split into a series of five figures to display the inundated structures in the area downstream and clearly identify respective roads for locating structures during an evacuation: (i) Figure 1 – Evacuation Map Upstream of Lake Sconti; (ii) Figure 2 – Evacuation Map Downstream of Lake Sconti; (iii) Figure 3 – Evacuation Map Downstream of Wilderness Parkway; (iv) Figure 4 – Evacuation Map Downstream of Cove Road; and (v) Figure 5 – Evacuation Map Downstream of McArthur Road.

A list of terms frequently used when discussing dam-related topics and features are presented in Appendix D – Definitions.

**Figure 1 - Lake Petit Dam Evacuation Map Upstream of Lake Sconti**



**LOCATION MAP**



**LEGEND**

- △ Inundated Structures
- Inundated Bridge/Dam

**Maximum Depth (feet)**

- < 5
- 5 - 10
- 10 - 15
- 15 - 20
- 20 - 30
- 30 - 40
- 40 - 50
- 50 - 60
- 60 - 70
- > 70

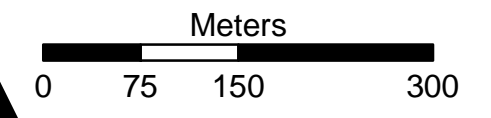
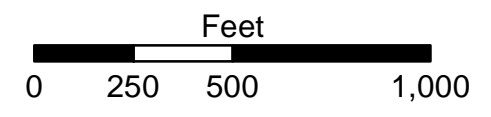
*NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.*

*See Table 4 of EAP for list of complete Street Addresses.*

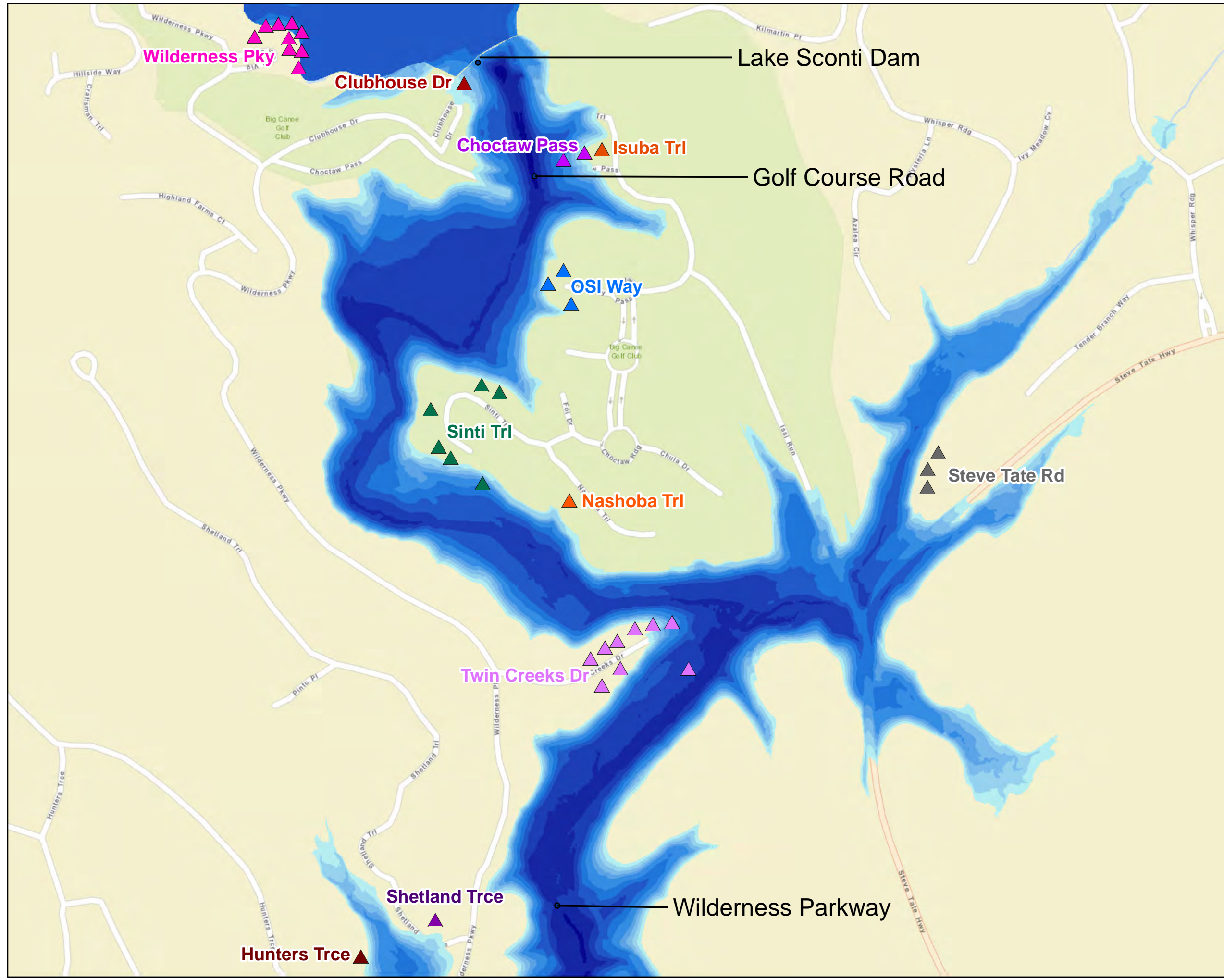
*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

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**Figure 2 - Lake Petit Dam Evacuation Map Downstream of Lake Sconti**



**LOCATION MAP**



**LEGEND**

- △ Inundated Structures
- Inundated Bridge/Dam

**Maximum Depth (feet)**

- < 5
- 5 - 10
- 10 - 15
- 15 - 20
- 20 - 30
- 30 - 40
- 40 - 50
- 50 - 60
- 60 - 70
- > 70

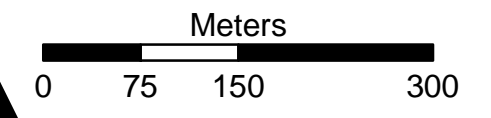
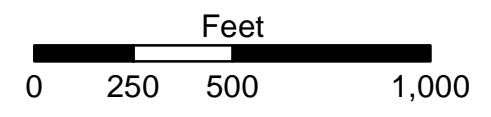
*NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.*

*See Table 4 of EAP for list of complete Street Addresses.*

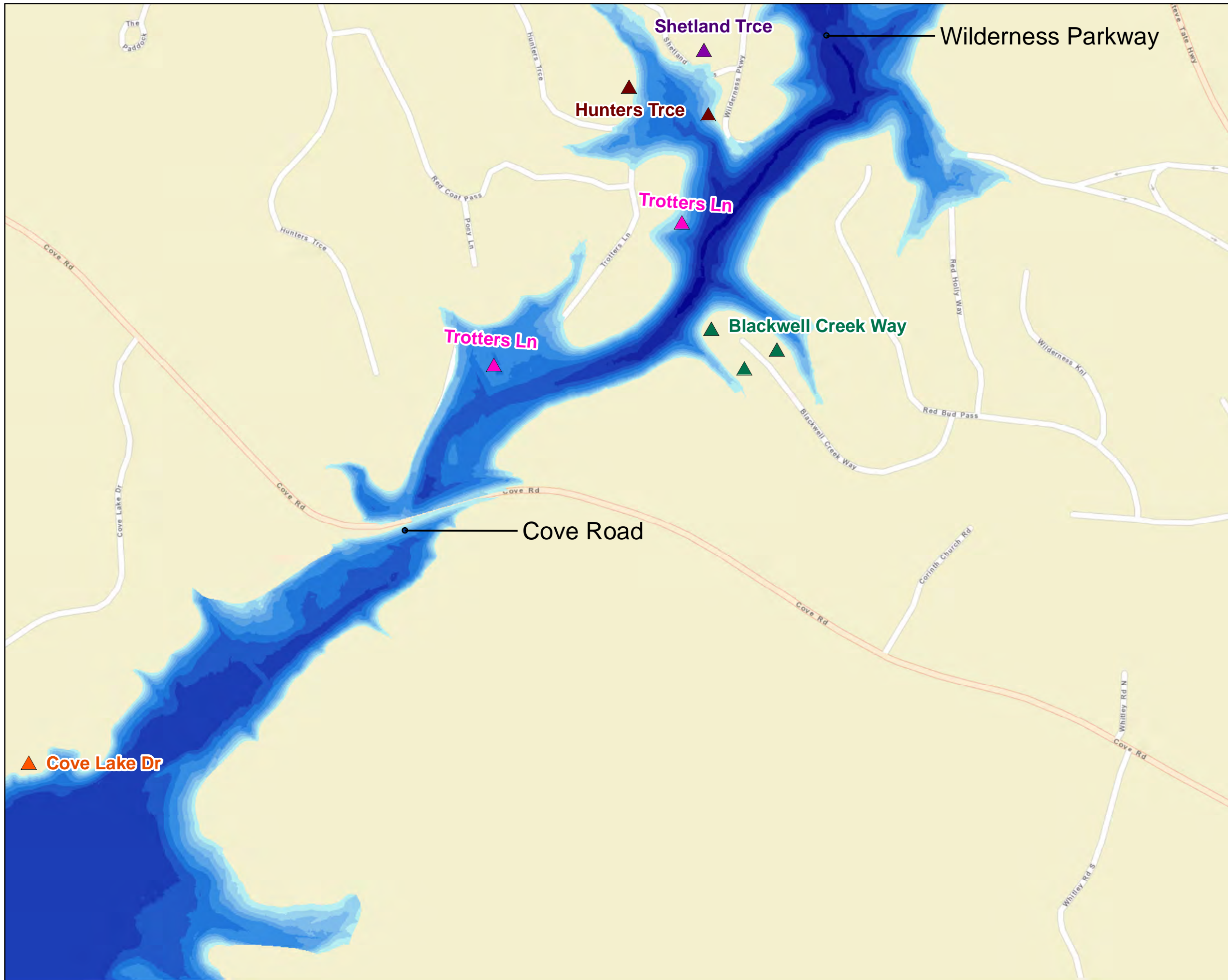
*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

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**Figure 3 - Lake Petit Dam Evacuation Map Downstream of Wilderness Parkway**



**LOCATION MAP**



**LEGEND**

- △ Inundated Structures
- Inundated Bridge/Dam

**Maximum Depth (feet)**

- < 5
- 5 - 10
- 10 - 15
- 15 - 20
- 20 - 30
- 30 - 40
- 40 - 50
- 50 - 60
- 60 - 70
- > 70

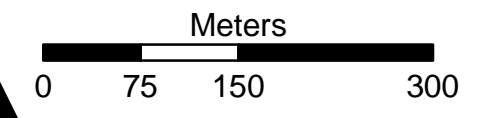
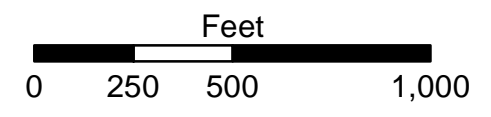
*NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.*

*See Table 4 of EAP for list of complete Street Addresses.*

*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

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**Figure 4 - Lake Petit Dam Evacuation Map Downstream of Cove Road**

**LOCATION MAP**



**LEGEND**

- △ Inundated Structures
- Inundated Bridge/Dam

**Maximum Depth (feet)**

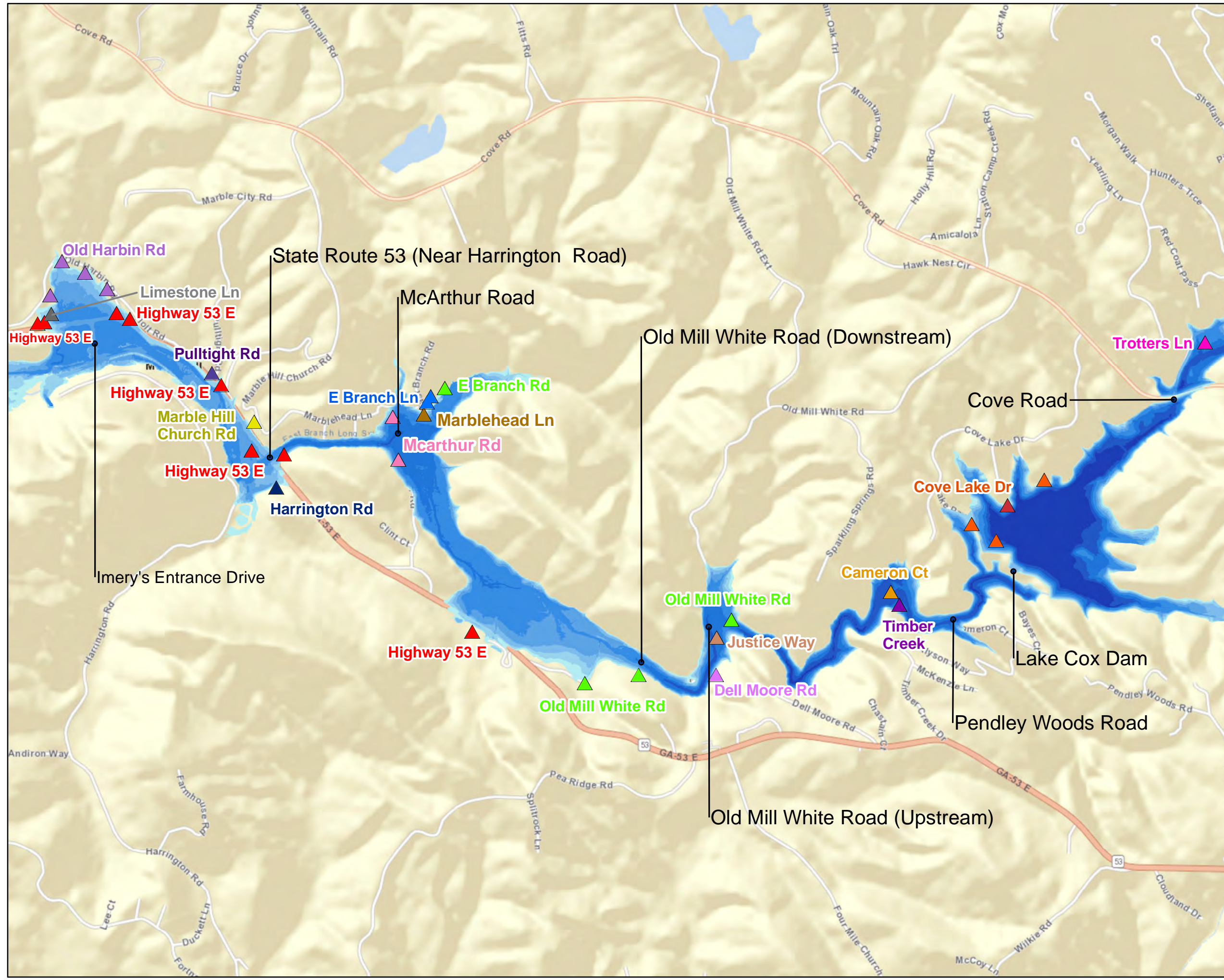
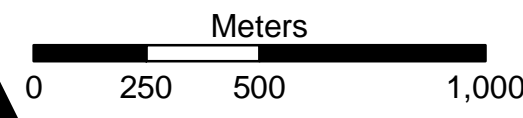
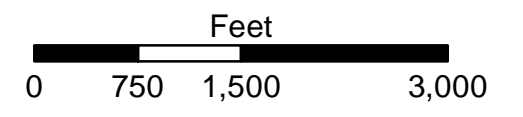
- < 5
- 5 - 10
- 10 - 15
- 15 - 20
- 20 - 30
- 30 - 40
- 40 - 50
- 50 - 60
- 60 - 70
- > 70

*NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.  
See Table 4 of EAP for list of complete Street Addresses.*

*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

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**Figure 5 - Lake Petit Dam  
Evacuation Map Downstream  
of McArthur Road**

**LOCATION MAP**



**LEGEND**

- △ Inundated Structures
- Inundated Bridge/Dam

**Maximum Depth (feet)**

- < 5
- 5 - 10
- 10 - 15
- 15 - 20
- 20 - 30
- 30 - 40
- 40 - 50
- 50 - 60
- 60 - 70
- > 70

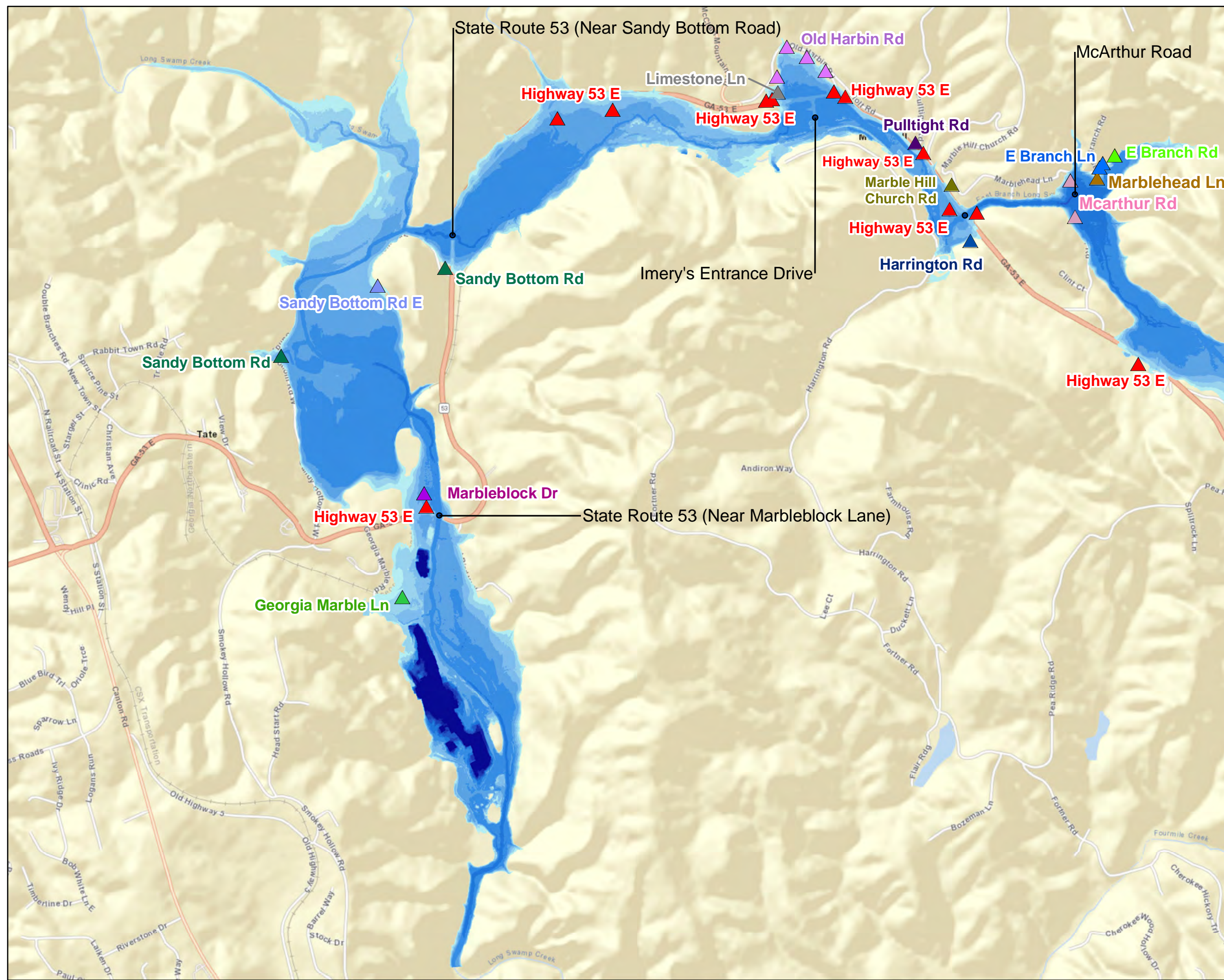
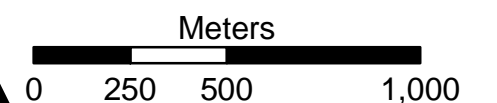
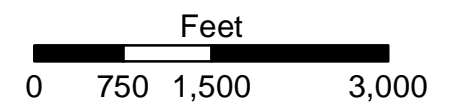
*NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.*

*See Table 4 of EAP for list of complete Street Addresses.*

*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

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## 2. SUMMARY OF EAP PROCESS

In the state of Georgia, dam owners and operators shall develop, and submit to GSDP an EAP for each Category I dam owned. The EAP contains actions designed to prevent a failure to dam structures or to minimize the impact of a dam failure on life and property. It establishes and documents procedures for notifying state and local EMA, law enforcement bodies, and downstream residents affected by a dam failure.

### 2.1 Summary of the EAP Process

There are four steps that must be followed anytime an unusual or emergency event is detected at the Dam. The steps are described in the following sections. An EAP Process Overview Flowchart outlining the following steps is provided in Figure 6 – EAP Process Overview Flowchart. The forms that should be used to document unusual or emergency events are provided in Appendix E (E-1: Contact Checklist; E-2: Condition B (Level 2) or C (Level 3) Event Log; and E-3: Dam Emergency Situation Report forms).

#### Step 1 – Event Detection, and Emergency Level Determination and Index

During the initial step, an unusual event or emergency event is detected at the Dam and classified by the Dam Owner’s Representative or designee. The Emergency Classifications are presented briefly subsequently and discussed in further detail in Section 3.1 of this document.

- **Condition A (Level 1), GREEN:** Unusual Event, slowly developing
- **Condition B (Level 2), YELLOW:** Emergency Event, potential dam failure situation, rapidly developing
- **Condition C (Level 3), RED:** Urgent! Emergency Event, Dam failure imminent or is in progress

#### Step 2 – Notification and Communication

After the event level has been determined, notifications are made in accordance with the appropriate Notification Flowchart provided in Section 4 of this document.

#### Step 3 – Remedial Actions

After the initial notifications are made, the EAP Coordinator should confer with the Dam Owner’s Technical Representative, Dam Owner’s Representative, and the GSDP to develop and execute appropriate preventative actions. During this step of the EAP, there is a continuous process of taking actions, assessing the status of the situation, and keeping others informed through the communication channels established during the initial notifications. The EAP may go through multiple event levels during Steps 2 and 3 as the situation either improves or worsens.

#### Step 4 – Termination and Follow-up

Once the event has ended or been resolved, termination and follow-up procedures should be followed as outlined in Section 6 of this document. EAP operations can only be terminated after completing operations under Condition C (Level 3) or A (Level 1). If Condition B (Level 2) is declared, the operations must be reclassified to Condition A (Level 1) or C (Level 3) before terminating the EAP operations.

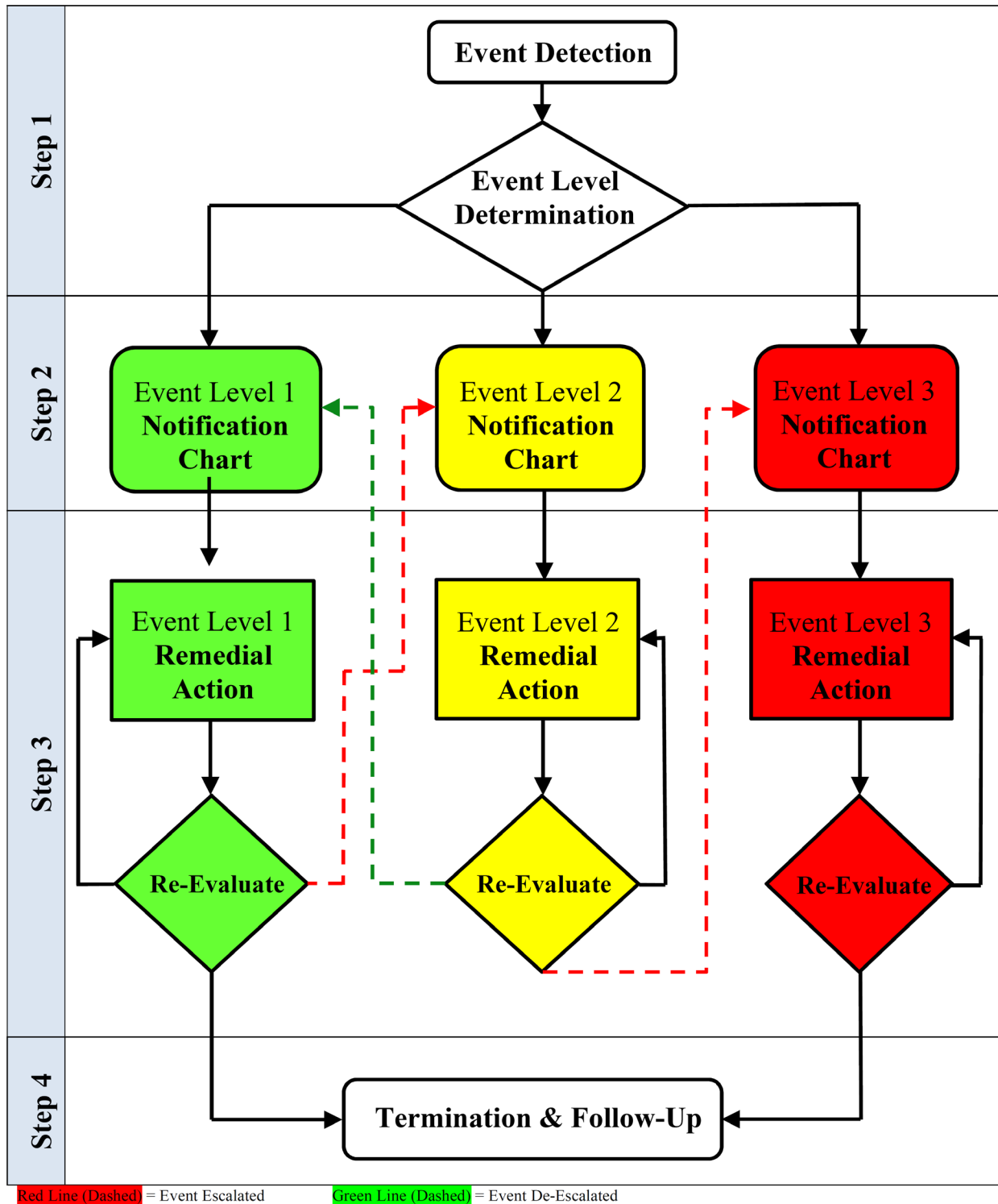


Figure 6 – EAP Process Overview Flowchart



### 3. STEP 1 – EMERGENCY LEVEL DETERMINATION AND INDEX

#### 3.1 Emergency Classification

Dam Failure Emergencies will be classified according to their severity and urgency. For the purposes of this EAP, three emergency classifications are provided. Conditions A, B, and C are consistent with other Big Canoe action plans, while Levels 1, 2, and 3 are consistent with GSDP. To assist the Dam Owner's Representative with the determination of the emergency classification, events and the dam failure mechanism derived from the events are presented in Table 2 – Emergency Level Determination and Emergency Level Index of this document.

To assist the EMA in selecting their appropriate course of action and to provide a proper transition from Condition A (Level 1) to Condition B (Level 2) or Condition B (Level 2) to Condition C (Level 3) the Big Canoe Public Safety Director will clearly communicate the situation to the EMA. For Conditions C (Level 3) and B (Level 2) situations, the Big Canoe Public Safety Director will place the first series of notifications on initial alert, and provide periodic updates on the situation as it develops so that the EMA can assess when they should implement their evacuation procedures. For example, the Dam Owner's Representative will issue an initial warning and periodic updates on the lake level as it rises during flooding conditions and eventually overtops the Dam. As the lake rises, a "potential failure situation is developing" warning should be issued with periodic updates on how much time is available before overtopping occurs. Once the Dam overtops, a "failure is imminent or has occurred" warning should be issued, as suggested in Section 4.3.2 of this document.

##### 3.1.1 Condition A (Level 1) – Unusual Event

This is a condition where a situation is developing but has not yet threatened the operation or the structural integrity of the Dam. The Dam Owner's Technical Representative, and if applicable, GSDP should be contacted to investigate the situation and recommend remedial actions. The condition of the Dam should be closely monitored, especially during storm events, to detect any development of a potential or imminent dam failure situation. The Dam Owner's Representative will assess the situation and determine a path forward approach. Warnings shall not be issued unless the situation develops into a Condition B (Level 2) situation.

##### 3.1.2 Condition B (Level 2) – Potential Failure Event

This is a condition where a failure may eventually occur, but preplanned actions taken during certain events (such as major floods, earthquakes, evidence of piping, etc.) may alleviate dam failure. Generally, for Condition B (Level 2) there is more time available than in a Condition C (Level 3) to issue warnings and/or take preparedness actions. Even if failure is inevitable, for a Condition B (Level 2) Event there should be a reasonable amount of time available for analysis before deciding on the evacuation of downstream residents. Preplanned actions will be initiated once a Condition B (Level 2) Emergency has been declared and the initial notifications have been completed. The preplanned actions that should be undertaken are shown in Section 5 of this document (Step 3 – Remedial Actions).

If time permits, the Dam Owner's Technical Representative, and if applicable GSDP, should be contacted to investigate the situation and recommend additional remedial actions after preplanned

actions were implemented to prevent the progression of the Dam's condition to Condition C (Level 3).

When a dam safety condition is observed that may lead to a failure if left unattended, but there is no immediate danger, the Dam Owner's Representative will issue a warning that a "potential failure situation is developing". The Dam Owner's Representative will assess the situation and determine the urgency of the emergency situation. Based on the Dam Owner's Representative's assessment, the first series of notifications should be made, and it is up to the EMA officials to determine the subsequent course of action to follow.

### **3.1.3 Condition C (Level 3) – Imminent Failure Event**

This is an urgent condition where a failure either has occurred, is occurring, or is about to occur and likely cannot be prevented. Modeling of multiple failure and breach scenarios was not completed as part of his work, and each failure and breach scenario may be different. Therefore, once the Dam Owner's Representative determines that there is no longer time available to implement preplanned actions to prevent dam failure, the "failure is imminent or has occurred" warning, as suggested in Section 4.3, should be issued. EMA officials shall interpret the phrase "failure is imminent" to mean that the dam is failing and order an evacuation of residents in potential inundation areas. For evacuation purposes, "failure is imminent" and "failure has occurred" shall be interpreted as the same condition.

## **3.2 Event Detection and Level Index**

Routine surveillance, observation, and/or instrumentation readings at the site will be the normal methods of detecting potential emergency situations. Unusual or emergency events may be detected by:

- Observations at or near the dam, including reservoir level;
- Evaluation of instrumentation data;
- Earthquakes felt or reported in the vicinity of the Dam; and
- Forewarning of conditions that may cause an unusual event or emergency event at the dam (e.g., a severe weather or flash flood forecast).

**Table 2 – Emergency Level Determination and Emergency Level Index**

| <b>Event</b>  | <b>Dam Failure Mechanism</b>  | <b>Evaluation of Failure</b>   | <b>Condition (Level)<sup>(1)</sup></b> |
|---|---|--|--|
| Unexpected Failure  | <ul style="list-style-type: none"> <li>Unknown</li> </ul>   | Dam unexpectedly and without warning begins to fail  | Condition C (Level 3)                  |
| Major Flood/<br>Embankment<br>Overtopping   | <ul style="list-style-type: none"> <li>Overtopping of dam</li> </ul>  | Erosion and removal of the road and embankment occurring   | Condition C (Level 3)                  |
|   |   | Flood pool rapidly approaching top of dam and embankment still intact  | Condition B (Level 2)                  |
| Global<br>Earthquake<br>or Seismic<br>Activity                                    | <ul style="list-style-type: none"> <li>Settlement of dam crest</li> <li>Slope movement</li> <li>Evidence of seepage or piping</li> <li>Damage to dam appurtenances</li> </ul> | Settlement of more than a few inches   | Condition C (Level 3)                  |
|   |   | Slope movement larger than the size of a car   |  |
|   |   | Flowing water from downstream face of dam  |  |
|   |   | Settlement of less than a few inches   | Condition B (Level 2)                  |
|   |   | Slope movement of less than the size of a car  |  |
|   |   | Wet areas on downstream face of dam that continue to increase in size and intensity of flow  |  |
| Measurable earthquake felt or reported near the dam and dam appears to be stable. | Condition A (Level 1)   |  |  |
| Embankment<br>Movement  | <ul style="list-style-type: none"> <li>Settlement of dam crest</li> <li>Slope movement</li> </ul>   | Settlement of more than a few inches   | Condition C (Level 3)                  |
|   |   | Slope movement larger than the size of a car   |  |
|   |   | Settlement of less than a few inches   | Condition B (Level 2)                  |
|   |   | Slope movement of less than the size of a car  |  |
|   |   | New cracks in the embankment greater than 1/4-inch wide without seepage  | Condition A (Level 1)                  |
| Embankment<br>Seepage   | <ul style="list-style-type: none"> <li>Evidence of seepage or piping</li> </ul>   | Seepage with a notable increase in flow (minimum a 25 % increase) and cloudiness from either weir on Bench No. 1, or the internal toe drains | Condition C (Level 3)                  |
|   |   | Wet areas with cloudy discharge on downstream face of dam that continue to increase in size and intensity of flow                            | Condition B (Level 2)                  |
|   |   | New seepage areas in or near the dam, water flowing clear  | Condition A (Level 1)                  |
| Spillway<br>Flow  | <ul style="list-style-type: none"> <li>Spillway overflow</li> <li>Spillway erosion</li> </ul>   | Spillway overflowing with an advancing head cut that is threatening the control section or that is already flooding people downstream        | Condition C (Level 3)                  |
|   |   | Spillway overflowing with active gully erosion   | Condition B (Level 2)                  |
|   |   | Spillway overflowing with no active erosion  | Condition A (Level 1)                  |
|   |   | Normal flow with erosion under, beneath, or at edges of the spillway   | Condition A (Level 1)                  |

**Table 2 – Emergency Level Determination and Emergency Level Index (Continued)**

| <b>Event</b>  | <b>Dam Failure Mechanism</b>  | <b>Evaluation of Failure</b>  | <b>Condition (Level)<sup>(1)</sup></b> |
|---|---|---|--|
| Sinkholes   | <ul style="list-style-type: none"> <li>Observed sinkhole</li> </ul>   | Rapidly enlarging sinkhole on dam or appurtenances  | Condition C (Level 3)                  |
|   |   | Observation of new sinkhole in reservoir area or on embankment  | Condition B (Level 2)                  |
|   |   | Observation of sinkhole downgradient of the dam   | Condition A (Level 1)                  |
| Routine Instrument. Readings  | <ul style="list-style-type: none"> <li>Significant change in piezometer readings</li> <li>Rapid decrease in lake level</li> </ul> | Increase in piezometer readings of more than 10 feet and flowing water from downstream face of dam  | Condition C (Level 3)                  |
|   |   | Rapid decrease in lake level and flowing water from downstream face of dam  |  |
|   |   | Increase in piezometer readings of more than 10 feet and no flowing water from downstream face of dam                                       | Condition B (Level 2)                  |
|   |   | Rapid decrease in lake level with no apparent reason and no flowing water from downstream face of dam                                       |  |
| Piezometer readings vary beyond predetermined values and no flowing water from downstream face of dam | Condition A (Level 1)   |   |  |
| Security Threat   | <ul style="list-style-type: none"> <li>Bomb threat</li> </ul>   | Detonated bomb that has results in damage to the dam or appurtenances   | Condition C (Level 3)                  |
|   |   | Verified bomb threat that, if carried out, could result in damage to the dam or appurtenances with no impacts to the functioning of the dam | Condition B (Level 2)                  |
|   |   | Reported bomb threat, unverified  | Condition A (Level 1)                  |
| Sabotage/ Vandalism   | <ul style="list-style-type: none"> <li>Damage to dam or appurtenances</li> </ul>  | Damage or modification to the dam or appurtenances with no impacts to the function of the dam   | Condition A (Level 1)                  |
|   |   | Damage to dam or appurtenances that has resulted in seepage flow  | Condition B (Level 2)                  |
|   |   | Damage to dam or appurtenances that has resulted in uncontrolled water release  | Condition C (Level 3)                  |
| Blocked culverts  | <ul style="list-style-type: none"> <li>Blockage</li> </ul>  | Debris is blocking a spillway pipe, causing lake level to rise  | Condition C (Level 3)                  |

Note: (1) Conditions A, B, and C are consistent with other Big Canoe action plans while Levels 1, 2, and 3 are consistent with Georgia Department of Natural Resources (GA DNR) Safe Dams Program. For clarity, both nomenclatures are provided

## 4. STEP 2 – NOTIFICATIONS AND COMMUNICATIONS

After the appropriate Emergency Level has been determined by the Dam Owner’s Representative or designee, the appropriate contacts listed in the Notification Flowcharts in this section of the document should be contacted and notified immediately.

### 4.1 Notification Flowchart

The Notification Flowcharts (Figures 7 and 8) summarize the following information which is applicable during an impending or imminent failure of the Dam:

- Who is responsible for notifying Big Canoe or the Dam Owner Representative and/or EMA officials;
- Who is to be notified; and
- What is the priority order in which individuals are to be notified.

All residents and employees of Big Canoe can and should be observers of unusual events at the Dam. This observer group will be educated through the local newsletter regarding what are symptoms of impending or imminent failure, and who at the POA should be contacted to initiate the Notification Process.

The Dam Owner's Representative or his/her designee is responsible for initiating the Notification Process. The Dam Owner's Representative or his designee will verify the condition of Lake Petit Dam which the observer has identified and initiate the notification. The Dam Failure Notification Flow Charts and Tables (Figures 7 and 8, and Table 3) identify the critical structures, EMA, government agencies, and Big Canoe Property Owners Association employees that should be contacted immediately and in what order. Subsequent contacts by each of these individuals are shown in priority order.

The inundated structures and addresses downstream of the Dam in the inundated areas were provided by the Pickens County EMA and are presented in Table 4 – Dam Failure Notification of Inundated Structures Downstream of Lake Petit Dam. The addresses were identified by Pickens County EMA based on the inundation zone determined in the Inundation Calculation presented in Appendix C plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

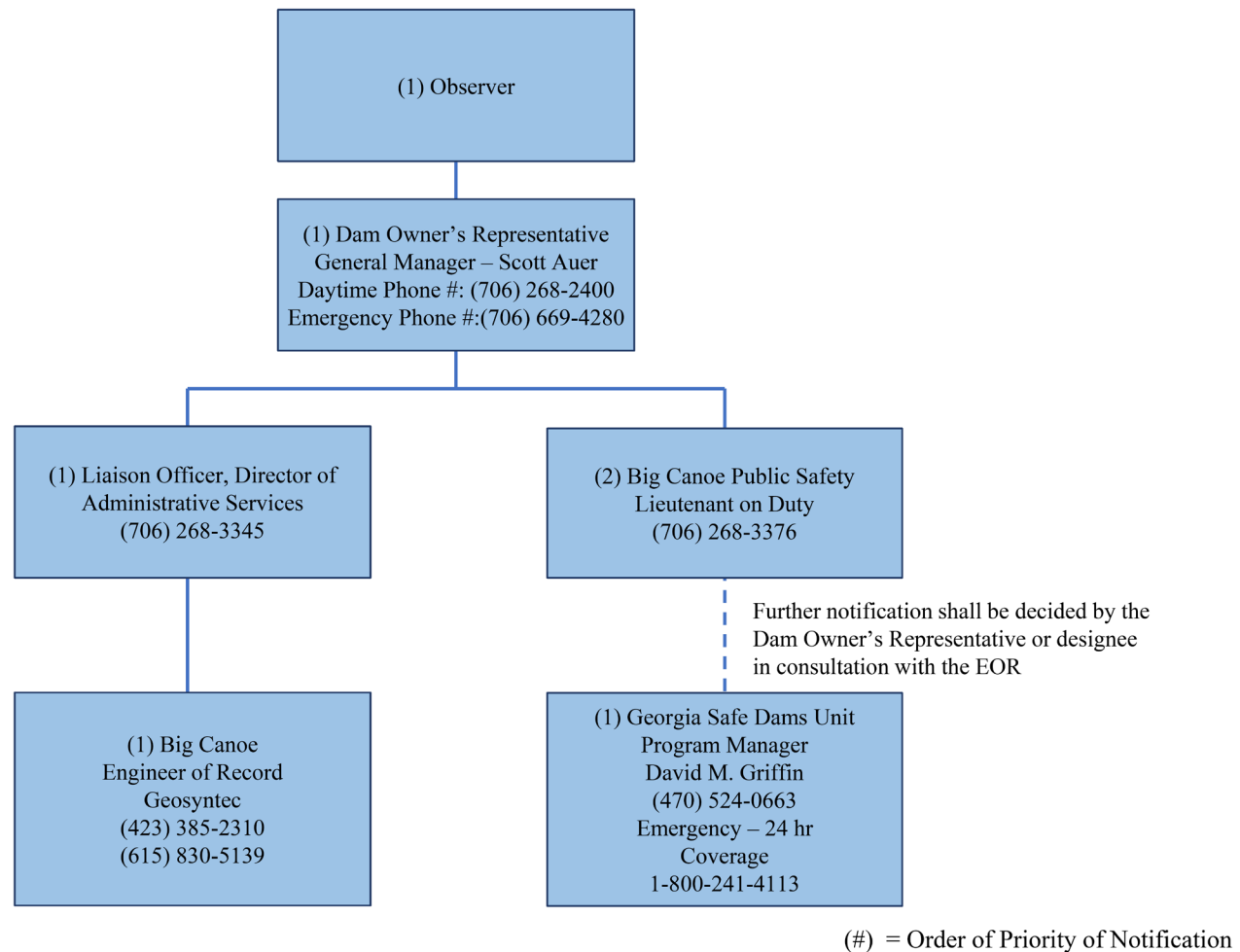
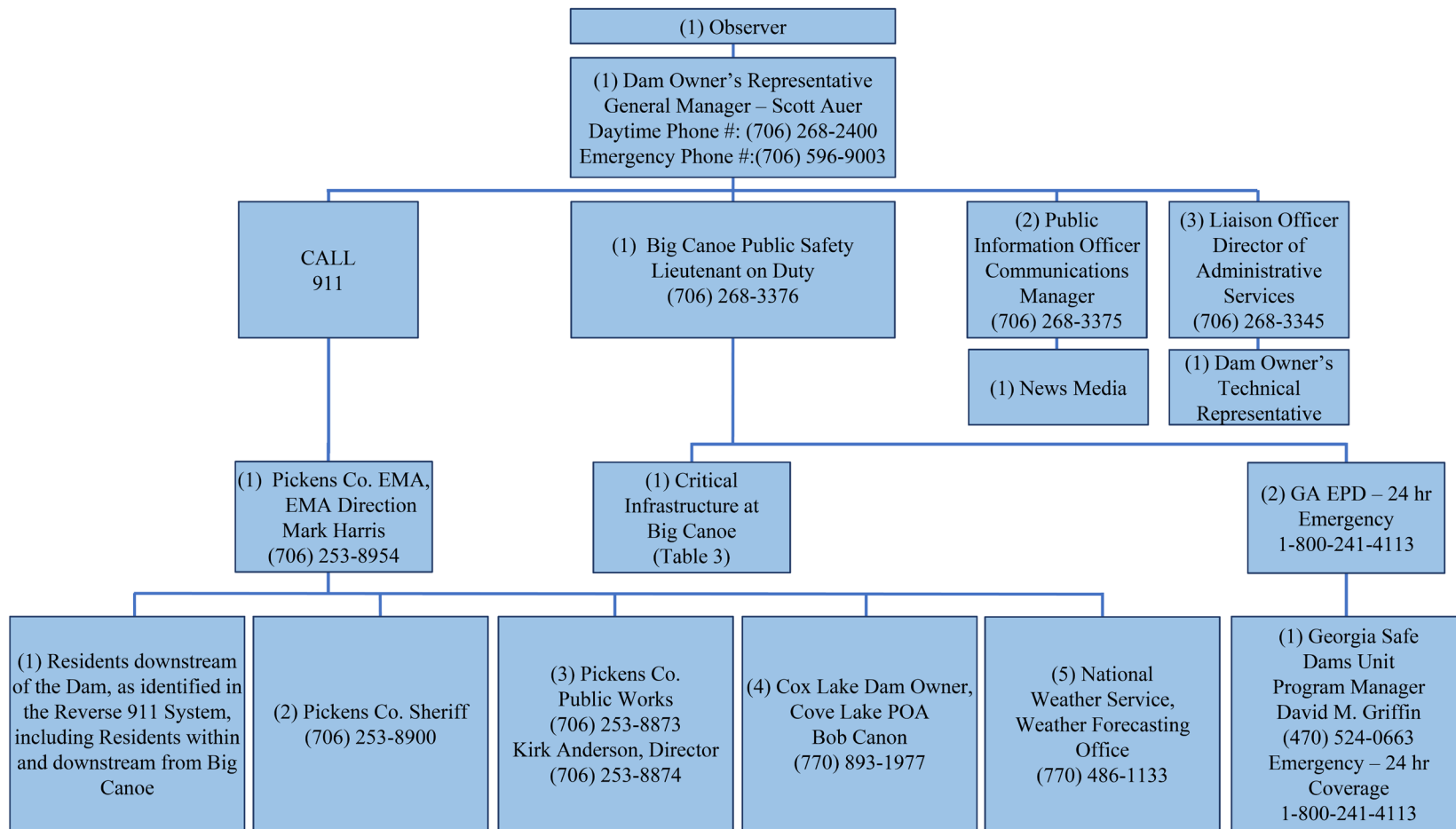


Figure 7 – Dam Failure Notification Flowchart for **Condition A (Level 1)** Emergencies



(#) = Order of Priority of Notification

**Figure 8:** Dam Failure Notification Flowchart for **Conditions B (Level 2)** & **C (Level 3)** Emergencies

**Table 3 – Dam Failure Notification of Big Canoe  
Critical Infrastructure Downstream of Dam**

| <b>Facility</b>   | <b>Big Canoe Street</b>          | <b>Phone Number</b> |
|-------------------|----------------------------------|---------------------|
| Utilities Office  | Highland Trail/Wolfscratch Drive | (706) 268-3400      |
| Beach Club        | Wolfscratch Drive                | (706) 268-3317      |
| Wellness Center   | Wolfscratch Drive                | (706) 268-3441      |
| POA Lodge         | Wolfscratch Drive                | (706) 268-3346      |
| Chapel            | Wolfscratch Circle               | (706) 268-3203      |
| Racquet Club      | Wolfscratch Circle               | (706) 268-3367      |
| Package Porch     | Wolfscratch Circle               | (706) 268-3376      |
| Lakewatch Village | Wilderness Parkway               | (706) 268-3376      |
| Clubhouse         | Clubhouse Drive                  | (706) 268-1253      |
| Fire Station #3   | Wolfscratch Drive                | (706) 268-1792      |
| Duffers           | Clubhouse Drive                  | (706) 268-3273      |
| Golf Shop         | Clubhouse Drive                  | (706) 268-3323      |
| Cart Barn         | Clubhouse Drive                  | (706) 268-3323      |



**Table 4 – Dam Failure Notification of Inundated Structures Downstream of Lake Petit Dam**

| <b>Street Number</b> | <b>Street Name</b>  |
|----------------------|---------------------|
| 255                  | Blackwell Creek Way |
| 270                  | Blackwell Creek Way |
| 295                  | Blackwell Creek Way |
| 10                   | Buckskull Brow      |
| 11                   | Buckskull Brow      |
| 26                   | Buckskull Brow      |
| 11                   | Buckskull Hollow Dr |
| 32                   | Buckskull Hollow Dr |
| 41                   | Buckskull Hollow Dr |
| 57                   | Buckskull Hollow Dr |
| 75                   | Buckskull Hollow Dr |
| 202                  | Buckskull Hollow Dr |
| 229                  | Buckskull Hollow Dr |
| 242                  | Buckskull Hollow Dr |
| 293                  | Buckskull Hollow Dr |
| 301                  | Buckskull Hollow Dr |
| 309                  | Buckskull Hollow Dr |
| 22                   | Buckskull Pt        |
| 26                   | Buckskull Pt        |
| 28                   | Buckskull Pt        |
| 30                   | Buckskull Pt        |
| 400                  | Cameron Ct          |
| 315                  | Choctaw Pass        |
| 333                  | Choctaw Pass        |
| 298                  | Clubhouse Dr        |
| 391                  | Cove Lake Dr        |
| 833                  | Cove Lake Dr        |
| 835                  | Cove Lake Dr        |
| 475                  | Dell Moore Rd       |
| 14                   | E Branch Ln         |
| 30                   | E Branch Ln         |
| 80                   | E Branch Rd         |

Note: Property information was provided by the Pickens County EMA in September 2022 using the mapped inundation area plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

**Table 4 – Dam Failure Notification of Inundated Structures Downstream of Lake Petit Dam (Continued)**

| <b>Street Number</b> | <b>Street Name</b>    |
|----------------------|-----------------------|
| 200                  | Georgia Marble Ln     |
| 9                    | Harrington Rd         |
| 15                   | Highland Ct           |
| 57                   | Highland Ct           |
| 110                  | Highland Ct           |
| 38                   | Highland Trl          |
| 84                   | Highland Trl          |
| 6361                 | Highway 53 E          |
| 8100                 | Highway 53 E          |
| 8200                 | Highway 53 E          |
| 8817                 | Highway 53 E          |
| 8839                 | Highway 53 E          |
| 9037                 | Highway 53 E          |
| 9077                 | Highway 53 E          |
| 9399                 | Highway 53 E          |
| 9502                 | Highway 53 E          |
| 9679                 | Highway 53 E          |
| 10322                | Highway 53 E          |
| 61                   | Hunters Trce          |
| 100                  | Hunters Trce          |
| 11                   | Isuba Trl             |
| 30                   | Justice Way           |
| 261                  | Laurel Ridge Trl      |
| 279                  | Laurel Ridge Trl      |
| 327                  | Laurel Ridge Trl      |
| 17                   | Laurel Ridge Way      |
| 34                   | Limestone Ln          |
| 414                  | Marble Hill Church Rd |
| 66                   | Marbleblock Dr        |
| 482                  | Marblehead Ln         |
| 393                  | Mcarthur Rd           |
| 427                  | Mcarthur Rd           |

Note: Property information was provided by the Pickens County EMA in September 2022 using the mapped inundation area plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

**Table 4 – Dam Failure Notification of Inundated Structures Downstream of Lake Petit Dam (Continued)**

| Street Number | Street Name       |
|---------------|-------------------|
| 52            | Nashoba Trl       |
| 154           | Old Harbin Rd     |
| 280           | Old Harbin Rd     |
| 356           | Old Harbin Rd     |
| 454           | Old Harbin Rd     |
| 1543          | Old Mill White Rd |
| 1985          | Old Mill White Rd |
| 2192          | Old Mill White Rd |
| 42            | Osi Way           |
| 43            | Osi Way           |
| 48            | Osi Way           |
| 112           | Overlook Ct       |
| 25            | Pulltight Rd      |
| 1138          | Sandy Bottom Rd   |
| 1511          | Sandy Bottom Rd   |
| 1150          | Sandy Bottom Rd E |
| 186           | Sconti Knoll Dr   |
| 191           | Sconti Knoll Dr   |
| 193           | Sconti Knoll Dr   |
| 196           | Sconti Knoll Dr   |
| 200           | Sconti Knoll Dr   |
| 60            | Sconti Pt         |
| 86            | Sconti Pt         |
| 111           | Sconti Pt         |
| 48            | Sconti Rdg        |
| 86            | Sconti Rdg        |
| 10            | Shetland Trce     |
| 126           | Sinti Trl         |
| 140           | Sinti Trl         |
| 180           | Sinti Trl         |
| 204           | Sinti Trl         |
| 220           | Sinti Trl         |

Note: Property information was provided by the Pickens County EMA in September 2022 using the mapped inundation area plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

**Table 4 – Dam Failure Notification of Inundated Structures Downstream of Lake Petit Dam (Continued)**

| Street Number | Street Name      |
|---------------|------------------|
| 250           | Sinti Trl        |
| 2191          | Steve Tate Rd    |
| 2193          | Steve Tate Rd    |
| 2195          | Steve Tate Rd    |
| 189           | Timber Creek Dr  |
| 35            | Treetop Knoll Dr |
| 95            | Treetop Knoll Dr |
| 40            | Treetop Ln       |
| 52            | Treetopper Ln    |
| 92            | Treetopper Ln    |
| 104           | Treetopper Ln    |
| 87            | Trotters Ln      |
| 125           | Trotters Ln      |
| 101           | Twin Creeks Dr   |
| 104           | Twin Creeks Dr   |
| 112           | Twin Creeks Dr   |
| 115           | Twin Creeks Dr   |
| 131           | Twin Creeks Dr   |
| 151           | Twin Creeks Dr   |
| 165           | Twin Creeks Dr   |
| 185           | Twin Creeks Dr   |
| 194           | Twin Creeks Dr   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 1944          | Wilderness Pky   |
| 244           | Wolfscratch Dr   |

Note: Property information was provided by the Pickens County EMA in September 2022 using the mapped inundation area plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

**Table 4 – Dam Failure Notification of Inundated Structures Downstream of Lake Petit Dam (Continued)**

| Street Number | Street Name             |
|---------------|-------------------------|
| 461           | Wolfscratch Dr          |
| 800           | Wolfscratch Dr          |
| 1125          | Wolfscratch Dr          |
| 1127          | Wolfscratch Dr          |
| 1136          | Wolfscratch Dr          |
| 1136          | Wolfscratch Dr          |
| 1136          | Wolfscratch Dr          |
| 1136          | Wolfscratch Dr          |
| 1175          | Wolfscratch Dr          |
| 12            | Wolfscratch Dr*         |
| 50            | Wolfscratch Village Cir |
| 84            | Wolfscratch Village Cir |
| 100           | Wolfscratch Village Cir |
| 226           | Wolfscratch Village Cir |
| 350           | Wolfscratch Village Cir |

Note: Property information was provided by the Pickens County EMA in September 2022 using the mapped inundation area plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

\*12 Wolfscratch Drive was not provided by Pickens County EMA, however was added due to its importance for the Dam Owner’s response during an emergency.

#### 4.2 Communication – **Emergency Condition A (Level 1)**

For a Condition A (Level 1) emergency, the Dam Owner’s Representative or designee, is responsible for contacting the Public Safety Director at Big Canoe and the Liaison Officer, Director of Administrative Services.

The Liaison Officer, Director of Administrative Services is responsible for contacting the Dam Owner’s Technical Representative. The Public Safety Director at Big Canoe is responsible for contacting the GSDP, if required by the Dam Owner’s Representative.

Note that a warning message shall not be issued for a Condition A (Level 1) emergency.

#### 4.3 Communication – **Emergency Condition B (Level 2) and C (Level 3)**

For a Condition B (Level 2) or Condition C (Level 3) emergency, the Dam Owner’s Representative or designee, is responsible for contacting the Public Safety Director at Big Canoe, the Public Information Officer Communications Manager, and the Liaison Officer, Director of Administrative Services.

The Public Safety Director at Big Canoe is responsible for notifying Pickens County EMA, Critical Infrastructure at Big Canoe, Dawson County 911 Services, and the GA EPD 24-Hour Emergency Contact in the event of an emergency. If time allows, they should seek advice and assistance.

The Public Information Officer, Communications Manager is responsible for notifying the News Media (including radio and television media). The Liaison Officer, Director of Administrative Services is responsible for notifying Dam Owner's Technical Representative.

The Pickens County EMA is responsible for notification of the residents below the Dam as identified in the Reverse 911 System, including: (i) Big Canoe residents and residents downstream from Big Canoe; (ii) Pickens County Sheriff's Department; (iii) Pickens County Public Works; (iv) the Cox Lake Dam Owner (Cove Lake Property Owner's Association, Inc.); and the (v) National Weather Service Weather Forecasting Office.

The GA EPD 24 Hour Emergency Contact is responsible for notification of the GSDP.

#### 4.3.1 **Emergency Condition B (Level 2) Warning Message**

All warning messages for an Emergency Condition B (Level 2) should be brief and to the point. The following message may be used to help describe the emergency situation to the emergency management personnel:

*"This is (**Big Canoe Public Safety Director or Lieutenant on Duty**). We have an emergency condition at Lake Petit Dam, located about 5.7 miles upstream of Marble Hill in Pickens County, Georgia. We have activated the Emergency Action Plan for this dam and are currently under Emergency Condition B (Level 2). We are implementing predetermined actions to respond to a rapidly developing situation that could result in dam failure. Please prepare to evacuate the areas downstream of the Dam based on the inundation mapping. Reference the evacuation map in your copy of the Emergency Action Plan. We will advise you when the situation is resolved or if the situation gets worse.*

*I can be contacted at the following number (**Big Canoe Public Safety Director's or Lieutenant's on Duty phone number to be contacted**). If you cannot reach me, please call the following alternative telephone number (**Big Canoe Public Safety Director's or Lieutenant's on Duty alternative phone number to be contacted at**)."*

#### 4.3.2 **Emergency Condition C (Level 3) Warning Message**

All warning messages for an Emergency Condition C (Level 3) should be brief and to the point. The EMA should be contacted immediately and the area evacuated. The following actions should be taken:

- Call 911. Be sure to say, "This is an emergency." They will call other authorities and begin the evacuation. The following message may be used to help describe the emergency situation to the Pickens County Sheriff's Department, or Pickens County EMA:

*"This is an emergency. This is (**Big Canoe Public Safety Director or Lieutenant on Duty**). Lake Petit Dam, located about 5.7 miles upstream of Marble Hill in Pickens County, Georgia is failing. The downstream area must be evacuated immediately based on the inundation mapping. Repeat, Lake Petit Dam, is failing; evacuate the downstream*

area. We have activated the Emergency Action Plan for this dam and are currently under Emergency Condition C (Level 3). Reference the evacuation map in your copy of the Emergency Action Plan.

I can be contacted at the following number (Big Canoe Public Safety Director's or Lieutenant's on Duty phone number to be contacted). If you cannot reach me, please call the following alternative telephone number (Big Canoe Public Safety Director's or Lieutenant's on Duty alternative phone number to be contacted at)."

- Do whatever is necessary to bring anyone in immediate danger (anyone on the Dam, downstream from the Dam, boating on the reservoir, or evacuees) to safety if directed by the EMA.
- Keep in frequent contact with the EMA and emergency services to keep them up to date on the condition of the Dam.
- If all means of communication are lost: (1) try to find out why, (2) try to get to another radio or telephone that works, or (3) get someone else to try to re-establish communications. If these means fail, handle the immediate problems that can be resolved, and periodically try to re-establish contact with the local police department and emergency services.

The following pre-scripted message may be used as a guide for the local law enforcement or the emergency services personnel to communicate the status of the emergency with the public:

"Attention: This is an emergency message from the Pickens County Emergency Management Agency. Listen carefully. Your life may depend on immediate action. Lake Petit Dam, located 5.7 miles upstream of Marble Hill, is failing. The downstream area must be evacuated immediately. Repeat, Lake Petit Dam, is failing. If you are in or near this area, proceed immediately to high ground away from the flood wave. Do not travel on Wilderness Parkway or Wolfscratch Drive in Big Canoe, on State Route 53 or along other streams or roads crossing East Branch Long Swamp Creek. Additionally, Cove Road one mile west of Steve Tate Road may be impassible. Do not return to your home to recover your possessions. You cannot outrun or drive away from the flood wave. Proceed immediately to high ground."

Repeat message.

## 5. STEP 3 – REMEDIAL ACTIONS

After the initial notifications are made, the Dam Owner’s Representative or designee should confer with the Dam Owner’s Technical Representative, and the GSDP, to develop and execute appropriate preventative actions. Some suggested preplanned actions that should be undertaken are shown in Table 5 – Preplanned Actions for Emergency Condition B (Level 2). Refer to the list of locally available resources that could be used in the event of an emergency in Appendix F – Locally Available Resources. These businesses can supply pumps, power generators, divers for inspections, and materials for a temporary repair, depending on the emergency. Also, the Pickens County EMA has provided a list of equipment that may be available during a dam failure. During this step of the EAP, there is a continuous process of taking actions, assessing the status of the situation, and keeping others informed through communication channels established during the initial notifications. The EAP may go through multiple event levels during Steps 2 and 3 as the situation either improves or worsens.



**Table 5 – Preplanned Actions for Emergency Condition B (Level 2)**

| Event                              | Impending Dam Failure Mechanism | Preplanned Actions in Priority Order  |
|------------------------------------|---------------------------------|---|
| Major Flood/Embankment Overtopping | Overtopping of the Dam          | 1) Ensure concrete chute spillway is unblocked, remove any debris.*   |
|                                    |                                 | 2) Make a reasonable attempt to open the sluice gate on the low-level drain or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.*  |
|                                    |                                 | 3) Open the bypass valve on the water supply line to the water treatment plant.*  |
|                                    |                                 | 4) If lake levels continue to rise dangerously close to top of the dam crest, excavate emergency channel in abutment area adjacent to concrete chute spillway. Also bring in emergency pumps and discharge outflow to spillway or directly into Petit Creek.* |
|                                    |                                 | <b>Do not excavate channel on top of dam, or discharge pump outflow on face of dam</b>  |
| Earthquake or Seismic Activity     | Slope Failure                   | 1) Make a reasonable attempt to open the sluice gate on the low-level drain or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.*  |
|                                    |                                 | 2) Open the bypass valve on the water supply line to the water treatment plant.*  |
|                                    |                                 | 3) Monitor piezometer readings daily and plot readings to identify significant changes in readings. Also record lake levels.  |
|                                    |                                 | 4) Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings.   |
|                                    |                                 | 5) Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).   |
| Embankment Movement                | Slope Failure                   | 1) Open the bypass valve on the water supply line to the water treatment plant.*  |
|                                    |                                 | 2) Monitor piezometer readings daily and plot readings to identify significant changes in readings. Also record lake levels.  |
|                                    |                                 | 3) Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings.   |
|                                    |                                 | 4) Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).   |

\*Action should be undertaken only if it is safe to do so.

**Table 5 – Preplanned Actions for Emergency Condition B (Level 2) (Continued)**

| Event  | Impending Dam Failure Mechanism | Preplanned Operations in Priority Order  |
|--|---------------------------------|--|
| Embankment Seepage, Spillway Flow, or Sinkhole | Internal Erosion Failure        | 1) Make a reasonable attempt to open the sluice gate on the low-level drain or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.* |
|  |                                 | 2) Open the bypass valve on the water supply line to the water treatment plant.*   |
|  |                                 | 3) Monitor piezometer readings daily and plot readings to identify significant changes in readings. Also record lake levels.   |
|  |                                 | 4) Monitor weirs and internal drain pipes daily for changes in flow quantity and quality, looking for muddy discharge, and plot readings to identify changes in readings.              |
|  |                                 | 5) Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).  |
| Routine Instrumentation                        | Slope Failure                   | 1) Make a reasonable attempt to open the sluice gate on the low-level drain or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.* |
|  |                                 | 2) Open the bypass valve on the water supply line to the water treatment plant.*   |
|  |                                 | 3) Monitor piezometer readings daily and plot reading to identify significant changes in readings. Also record lake levels.  |
|  |                                 | 4) Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings.  |
|  |                                 | 5) Obtain input from Georgia Safe Dams program or Owner's Engineer as to emergency repairs to be constructed (if any).   |
| Security Threat or Sabotage                    | Slope Failure                   | 1) Make a reasonable attempt to open the sluice gate on the low-level drain or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.* |
|  |                                 | 2) Open the bypass valve on the water supply line to the water treatment plant.*   |
|  |                                 | 3) Notify appropriate authorities and secure access to the dam.  |

\*Action should be undertaken only if it is safe to do so.

## 6. STEP 4 – TERMINATION AND FOLLOW-UP

Pickens County EMA in coordination with the GSDP and the Dam Owner’s Representative or designee, is responsible for terminating the EAP operations and relaying this decision to all parties active in EAP operations. It is then the responsibility of each person to notify the same group of contacts that were notified during the original event notification process and inform them that the event has been terminated.

Prior to termination of an Emergency Condition C (Level 3) event that has not caused actual dam failure, the Dam Owner’s Technical Representative will inspect the dam and assess whether any damage has occurred to the Dam that could potentially result in loss of life, injury, or property damage. If it is determined, in coordination with the GSDP, conditions do not pose a threat to human life or property, Pickens County EMA will be advised to terminate EAP operations as described above.

The Dam Owner’s Representative or designee shall ensure that the *Dam Safety Emergency Situation Report* in Appendix E-3 is completed to document the emergency event and all actions taken. The Dam Owner’s Representative or designee shall distribute copies of the completed report to the GSDP.

## 7. ROLES AND RESPONSIBILITIES

Dam owners, in coordination with EMA authorities, are responsible for implementing the EAP. EMA authorities with statutory obligations are responsible for warning and evacuation within affected areas. All entities involved with EAP implementation should document incident-related events. All parties responsible for implementing the EAP shall verify their responsibilities with their signature in Appendix B-1 – Concurrences of this document.

### 7.1 Dam Owner's Responsibilities

The person responsible for performing the tasks required under the EAP is the Dam Owner's Representative or designee, and the Public Safety Director. If the Public Safety Director is absent, the responsible person will be the Public Safety Director Lieutenant on duty at that time.

As soon as an emergency event is observed or reported, the Dam Owner's Representative and Public Safety Director under the EAP shall:

Initiate the initial assessment of the event and designate the appropriate emergency condition (i.e., also referred to by GA EPD as an emergency level):

- Condition A (Level 1);
- Condition B (Level 2); or
- Condition C (Level 3).

1) Based on the type of condition:

- a) If a Condition A (Level 1) is determined, initiate Figure 7 – Dam Failure Notification Flowchart for Condition A (Level 1) Emergencies;
- b) If a Condition B (Level 2) is determined, initiate the preplanned actions in Table 5 – Preplanned Actions for Emergency Condition B (Level 2), and Figure 8 – Dam Failure Notification Flowchart for Condition B (Level 2) & C (Level 3) Emergencies; and
- c) If a Condition C (Level 3) is determined, initiate Figure 8 – Dam Failure Notification Flowchart for Condition B (Level 2) and C (Level 3) Emergencies.

2) If a Condition B (Level 2) or Condition C (Level 3) emergency is determined, provide updates of the situation to the EMA to assist them in making timely and accurate decisions regarding warnings and evacuations.

3) Provide leadership to assure the EAP is reviewed and updated annually and copies of the revised EAP are distributed to all who received copies of the original EAP.

### 7.2 EAP Coordinator Responsibility

The Dam Owner's Representative or designee will be the designated EAP Coordinator who will be responsible for EAP-related activities, including preparing revisions to the EAP, establishing training activities, coordinating EAP exercises, etc. They will also be the EAP contact if any of

the involved parties have questions about the plan. The EAP will be reviewed annually with contacts, phone numbers, verified for accuracy. Revisions to the EAP should be documented in the Revision Log of this document. As infrastructure and homes are built in the inundation map hazard areas, information (i.e., addresses) will be added to Tables 3 and 4, as appropriate and available.

### **7.3 Local Emergency Management (Pickens County EMA)**

Pickens County EMA will serve as the primary contact responsible for coordination of all emergency actions. During EAP preparation they will coordinate with local responders and dispatchers to ensure each has an opportunity for input into the EAP and each has a copy and is aware of their responsibilities and participate in review and updates of the EAP.

#### **7.3.1 Responsibility of Evacuation**

Warning and evacuation planning are the responsibilities of the Pickens County EMA who have the statutory obligation. Under the EAP, the Big Canoe Public Safety Director is responsible for notifying the Pickens County EMA when a failure is imminent or has occurred (Condition C or Level 3), or a potential failure situation is developing (Condition B or Level 2). Big Canoe will not assume the responsibility of government entities for the evacuation of people. This procedure should be coordinated with the appropriate public officials prior to an emergency situation developing.

When a Condition B (Level 2) situation occurs, The Pickens County EMA will:

- Prepare response personnel for possible evacuations that may be needed if a Level 3 situation develops.
- Alert the public as appropriate.

When a Condition C (Level 3) situation develops:

- Alert the public.
- Immediately close roads and evacuate people within and possibly adjacent to the inundation area.

#### **7.3.2 Responsibility for Duration, Security Termination, and Follow-Up**

The Pickens County EMA is responsible for monitoring the situation at the Dam and keeping local authorities informed of developing conditions at the Dam from the time that an emergency starts until the emergency has been terminated. Security measures at the Dam should be implemented by the Pickens County Sheriff's Department.

The Pickens County EMA is responsible for declaring that the emergency at the Dam is terminated in coordination with the Dam Owner and the GSDP.

A follow-up evaluation after an emergency by all participants will be conducted, as outlined in Section 8.

#### **7.4 Dam Owner's Technical Representative(s)**

The Dam Owner's Technical Representative is an individual with intimate knowledge of the Dam. During an emergency condition if time permits, the Technical Representative will be contacted accordingly and will,

- Advise the Dam Owner's Representative of the emergency level determination; and
- Advise the Dam Owner's Representative of remedial actions to take if an event occurs.

#### **7.5 Georgia Safe Dams Program**

The GSDP, or the GSDP technical representative, is responsible for providing technical assistance to the EAP Coordinator as needed and as time allows.

## 8. MAINTENANCE, PREVENTION, AND PREPAREDNESS ACTIONS

### 8.1 EAP Annual Review

The Dam Owner's Representative or designee will review and, if needed, update the EAP at least once each year. The EAP annual review will include the following:

- Call all contacts in the Notification Flowcharts in this document to verify that the phone numbers and the contact personnel are current. The EAP will be revised if any of the contacts have changed.
- Contact the local law enforcement agency to verify the phone numbers and/or personnel in the specified positions. In addition, the Dam Owner's Representative or designee will ask if the person contacted knows where the EAP is kept and if responsibilities described in the EAP are understood.
- Call the locally available resources (Appendix F) to verify that the phone numbers, addresses, and services are current.
- Confirm all-hazard contact information listed in the document is correct.

### 8.2 EAP Revisions

The Dam Owner's Representative or designee is responsible for updating the EAP document. The EAP document held by the Big Canoe POA is the master document. When revisions occur, the Dam Owner's Representative or designee will provide the revised pages and a revised revision summary page to all the EAP document holders. The document holders are responsible for revising outdated copies of the respective document(s) whenever revisions are received. Outdated pages shall be immediately discarded to avoid any confusion with the revisions. Future revisions and modifications to the EAP should be documented in the Revision Log of this document.

### 8.3 EAP Periodic Tests

The Dam Owner's Representative or designee will host and facilitate a periodic test of the EAP at least once every 5 years.

The periodic test will consist of a meeting, including a tabletop exercise. Attendance should include the Dam Owner's Representative or designee, GSDP staff, and Pickens County EMA, at least one representative of the local law enforcement agency, and others with key responsibilities listed in the EAP document. At the discretion of the Dam Owner's Representative or designee, other organizations that may be involved with an unusual or emergency event at the Dam are encouraged to participate. Before the tabletop exercise begins, meeting participants will visit the Dam during the periodic test to familiarize themselves with the Dam site.

The tabletop exercise will begin with the facilitator presenting a scenario of an unusual or emergency event at the Dam. The scenario will be developed prior to the exercise. Once the scenario has been presented, the participants will discuss the responses and actions that they would take to address and resolve the scenario. The narrator will control the discussion, ensuring realistic

responses and developing the scenario throughout the exercise. The Dam Owner's Representative or designee should complete an event log as they would during an actual event.

After the tabletop exercise, the EAP will be reviewed and discussed. Mutual aid agreements and other emergency procedures can be discussed. The Dam Owner's Representative or designee will prepare a written summary of the periodic test and revise the EAP, as necessary.

## 8.4 Prevention and Preparedness Actions

The following prevention and preparedness actions should be taken in preparation of an emergency:

- Initiation of a dam inspection and surveillance program. The Dam should be formally inspected quarterly, at minimum, and readings from the Dam instrumentation should be taken and interpreted quarterly, at minimum.
- Preparation of a systematic warning and evacuation plan. A formal notification system should be coordinated with residents and businesses in the inundation area, and evacuation routes should be discussed/provided to those residents.
- Preparations should be made for evacuation on weekends, weekdays, and any time of day or night, including holidays. The emergency responders should have backup ways of communicating and a way to respond to the emergency in case of power outages.
- Community awareness programs for emergency response procedures. The community should be made aware of the possible emergencies and procedures associated with the possible failure of the Dam.
- Establishment of emergency flood operating procedures.
- Revisions of this EAP should be documented in the Revision Log of this document.
- Conduct emergency exercises. At minimum, an orientation meeting should be held with key people so that those playing key roles and those having responsibilities outlined in this EAP can become familiar with it. A drill or tabletop exercise can be held and coordinated with the Pickens County EMA. Documentation of reviews, and tests of this EAP should be documented in the forms provided in Appendix B.
- Organization of equipment, labor, and materials for use in emergency situations. A list of locally available resources that could be used in the event of an emergency are provided in Appendix F. These businesses can supply pumps, power generators, divers for inspections, and materials for temporary repair, depending on the emergency. Also, the Pickens County EMA has provided a list of equipment that may be available during a dam failure.



## 9. REFERENCES

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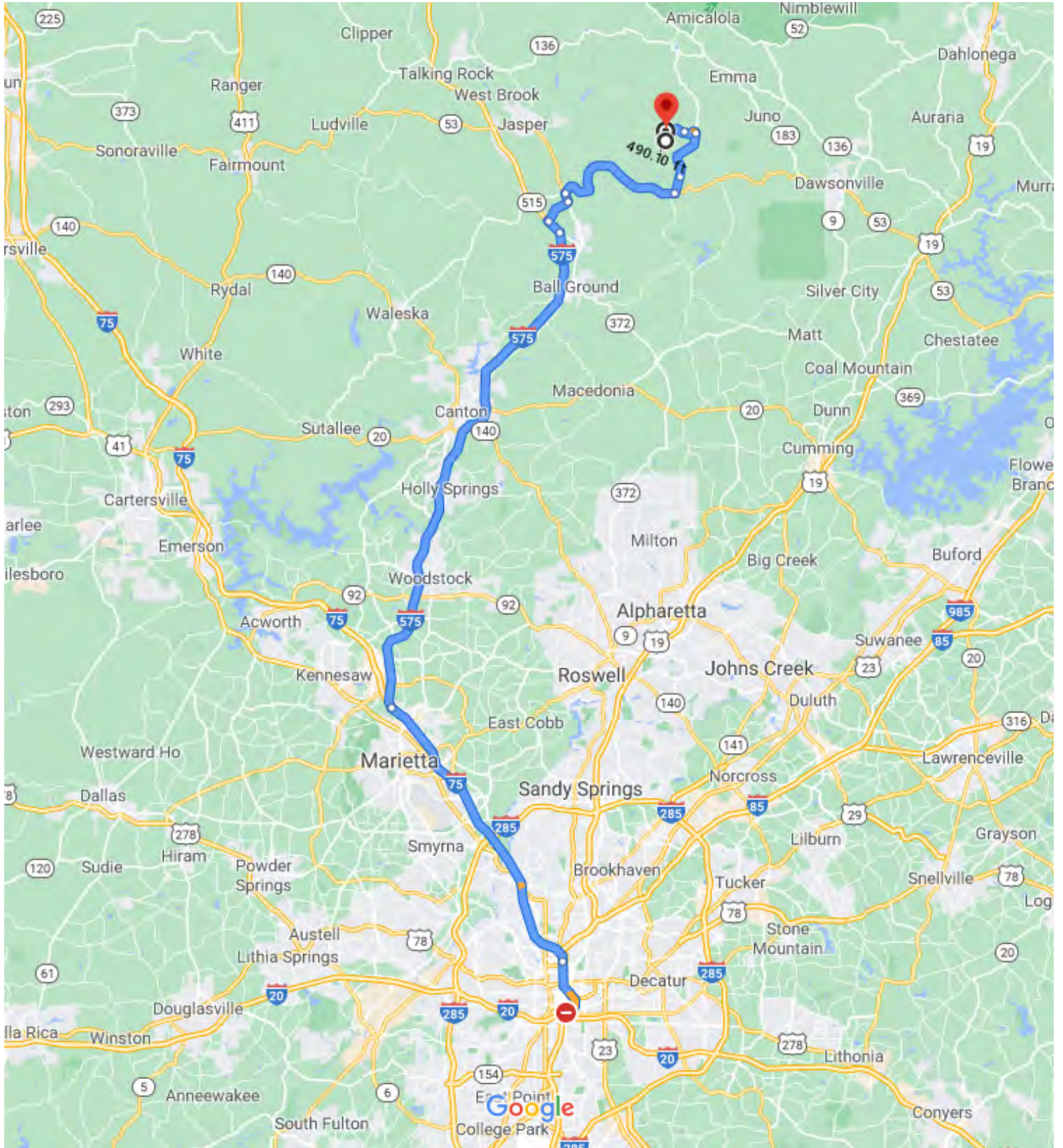
# APPENDIX A

## Directions from Atlanta, GA to the Dam



Atlanta, GA to Lake Petit Dam, Jasper, GA

Drive 70.0 miles, 1 hr 18 min



Map data ©2021 Google 5 mi

Measure distance  
Total distance: 490.10 ft (149.38 m)



Atlanta, GA to Lake Petit Dam, Jasper, GA

Drive 70.0 miles, 1 hr 17 min

## Atlanta

Georgia

### Get on I-75 N/I-85 N

- 1 min (0.4 mi)
1. Head north on Capitol Ave SW  
79 ft
  2. Turn right onto M.L.K. Jr Dr SE  
0.2 mi
  3. Turn left to merge onto I-75 N/I-85 N  
0.2 mi

### Follow I-75 N and I-575 N to GA-5 N/GA-515 E in Pickens County

- 46 min (51.8 mi)
4. Merge onto I-75 N/I-85 N  
3.1 mi
  5. Keep right to continue on I-75 N  
18.0 mi
  6. Keep right at the fork to continue on GA-5 N/I-575 N, follow signs for Canton  
30.8 mi

### Take Hwy 53 E and Steve Tate Hwy to Wilderness Pkwy

- 28 min (17.8 mi)
7. Continue onto GA-5 N/GA-515 E  
0.9 mi
  8. Sharp right onto Worley Crossroads  
1.7 mi
  9. Turn left onto Canton Rd  
0.5 mi
  10. Turn right onto Hwy 53 E  
8.5 mi
  11. Turn left onto Steve Tate Hwy  
1.0 mi
  12. At the traffic circle, take the 1st exit and stay on Steve Tate Hwy  
3.0 mi
  13. Turn left onto Wilderness Pkwy  
0.8 mi

↩ 14. Turn left to stay on Wilderness Pkwy

 Destination will be on the right

1.3 mi

## Lake Petit Dam

Jasper, GA 30143

These directions are for planning purposes only. You may find that construction projects, traffic, weather, or other events may cause conditions to differ from the map results, and you should plan your route accordingly. You must obey all signs or notices regarding your route.

## **APPENDIX B**

**B-1: Concurrences**

**B-2: Record of Holders of Control Copies**

**B-3: EAP Review**

**B-4: Periodic Tests**

## APPENDIX B-1 – CONCURRENCES

By my signature, I acknowledge that I, or my representative, have reviewed this plan and concur with the tasks and responsibilities assigned herein for me and my organization.

### Dam Owner's Representative, Big Canoe POA

1. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_

### Big Canoes POA's Public Safety

2. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_

### Big Canoe POA's Lake Petit Dam Technical Representative

3. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: Wesley MacDonald, PE, Senior Engineer

### Georgia Safe Dams Program Representative

4. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_

### Pickens County Sheriff's Department

5. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_

### Pickens County Emergency Management Agency

6. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_

**Pickens County Public Works**

7. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_

**Cox Lake Dam Owner, Cove Lake Property Association**

8. \_\_\_\_\_  
*Signature* *Organization* *Date*  
Printed name and title: \_\_\_\_\_



## **EAP CONCURRENCE**

By my signature, I acknowledge that I, or my representative, have reviewed this plan and concur with the tasks and responsibilities assigned herein for me and my organization.

Organization:

---

Name:

---

Title:

---

Signature:

---

Date:

---

## APPENDIX B-2 – RECORD OF HOLDERS OF CONTROL COPIES

| Copy Number | Organization  | Person Receiving Copy                         |
|-------------|---|---|
| 1           | Big Canoe Property Owners Association, Inc.<br>10586 Big Canoe<br>Jasper, Georgia 30143                 | Scott Auer                                    |
| 2           | Big Canoe POA’s Public Safety Director<br>41 Wolfscratch Circle<br>Marble Hill, GA 30148                | Ricky Jordan                                  |
| 3           | Engineer – Geosyntec Consultants, Inc.<br>835 Georgia Avenue, Suite 500<br>Chattanooga, Tennessee 37402 | Wesley MacDonald,<br>P.E.(TN, AL, GA, and WA) |
| 4           | Georgia Safe Dams Program<br>2 Martin Luther King Jr. Drive SE, Suite 1362<br>Atlanta, GA 30334         | David M. Griffin, P.E.                        |
| 5           | Pickens County Sheriff’s Department<br>2985 Camp Rd., Jasper GA 30143                                   | Donnie Craig                                  |
| 6           | Pickens County EMA<br>1266 East Church Street<br>Jasper, Georgia 30143                                  | Mark Harris                                   |
| 7           | Pickens County Public Works<br>3043 Camp Road<br>Jasper, Georgia 30143                                  | Kirk Anderson                                 |
| 8           | Cox Lake, Cove Lake Property Association  | Bob Canon                                     |

### **APPENDIX B-3 – EAP ANNUAL REVIEW**

An Annual Review of this EAP should be conducted by the Big Canoe Property Owner’s Association. The annual review should be documented in this appendix.

| <b>Date</b> | <b>Conducted By</b> | <b>Notes and Observations</b> |
|-------------|---------------------|-------------------------------|
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |
|             |                     |                               |

## APPENDIX B-4 – EAP PERIODIC TEST

A periodic test of the EAP procedures is recommended every 5 years. Documentation for the test is provided in this appendix.

| Date | Conducted By | Observations |
|------|--------------|--------------|
|      |              |              |
|      |              |              |
|      |              |              |
|      |              |              |
|      |              |              |
|      |              |              |
|      |              |              |
|      |              |              |
|      |              |              |

# **APPENDIX C**

## **Inundation Mapping**

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**LAKE PETIT DAM**  
**Pickens County, Georgia**  
**State ID No. 112-009-00462**  
**NID No. GA00685**

**Lake Petit Dam Breach Analysis**

*Prepared for:*

**Big Canoe® Property Owners Association, Inc.**  
10586 Big Canoe  
Jasper, GA 30143

*Prepared by:*

**Geosyntec Consultants, Inc.**  
835 Georgia Avenue, Suite 500  
Chattanooga, TN 37402

Project No: TN8667

December 2022





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| Attachment B | Electronic Copy with Digital Files (On CD)              |
| Attachment C | Stage Storage Tables                                    |
| Attachment D | Breach Parameter Calculation                            |
| Attachment E | Settings and Tolerances                                 |
| Attachment F | Inundation Map  |
| Attachment G | Velocity Map  |
| Attachment H | Summary of Warnings and Notes                           |
| Attachment I | Potential Hazard Addresses                              |
| Attachment J | Hydrographs   |
| Attachment K | Photographs of the Dam and Points of Interest           |
| Attachment L | Aerial Photos of Point of Interests                     |

## **EXECUTIVE SUMMARY**

This dam breach analysis report has been prepared in support of revised inundation mapping of Lake Petit Dam located in Pickens County, Georgia. The most recent inundation mapping was performed in 2000 by Jordan, Jones, and Goulding utilizing DAMBRK software. The current recommendations from the State of Georgia's Safe Dam Program (SDP) require inundation mapping utilizing Hydrologic Engineering Center's River Analysis System (HEC-RAS) software. These regulations were published in the Engineer Guidelines (Version 4.0, 2015) by the SDP on July 2015.

Lake Petit Dam, constructed in 1972, is located within the Big Canoe Development on Petit Creek about 5.8 miles upstream of Marble Hill, in Pickens County, north-central Georgia.

Three other dams of interest are located downstream of the Lake Petit Dam. Two are located directly downstream. Lake Sconti is located approximately 1.0 miles downstream of Lake Petit.. Cox Lake Dam is located approximately 3.5 miles downstream from Lake Petit Dam south of Cove Road. Lake Disharoon Dam is located approximately 0.5 miles downstream but is not directly downstream of Lake Petit on Petit Creek. Rather, both Lake Petit and Lake Disharoon drain to Sconti Lake.

There are twelve (12) bridges downstream of Lake Petit that are of interest. In the hydraulic model, all downstream dams, bridges and culverts are set to breach when overtopped by two feet. All bridges and culverts were modeled assuming 50 percent obstruction. Lake Petit Dam and Lake Sconti Dam breached during the simulation. Lake Cox Dam did not breach. Lake Disharoon Dam was inundated. Modeled bridges at Wolfscratch Drive, Golf Course Road, Wilderness Parkway, Cove Road, Pendley Woods, Old Mill Road (Upstream) (also known as Justice Way), Old Mill Road (Downstream), McArthur Road, State Route 53 (Near Harrington Road), and Imerys Entrance Drive, and State Route 53 (Near Sandy Bottom Road) breached during the simulation. The modeled bridge at State Route 53 (Near Marbleblock Lane) did not breach.

The dam breach analysis described in this report evaluated the impacts of a potential embankment failure for Lake Petit Dam, and found that the Lake Petit Dam has a high hazard potential with potential for loss of life. Consequently, Lake Petit Dam is classified as Category I, Very Large Dam and also provides an updated inundation map for evacuation considerations during a breach failure.

# **DAM BREACH ANALYSIS**

## **LAKE PETIT DAM**

### **1. BACKGROUND AND PURPOSE**

Geosyntec prepared this calculation package in support of revised inundation mapping of Lake Petit Dam located in Pickens County, Georgia. The most recent inundation mapping was performed in 2000 by Jordan, Jones, and Goulding utilizing DAMBRK software. The current recommendations from the State of Georgia's Safe Dam Program (SDP) require inundation mapping utilizing HEC-RAS software (Georgia DNR, 2015). These regulations were published in the Engineer Guidelines (Version 4.0, 2015) by the SDP on July 2015.

The purpose of this calculation package is to evaluate the impacts of a potential embankment failure for Lake Petit Dam, and, in particular, to evaluate the depth and velocity of potential flood waters and to identify impacted structures and roadways within the inundation extent. The dam breach analysis and inundation mapping will ensure the emergency planning and response meets the most recent State requirements.

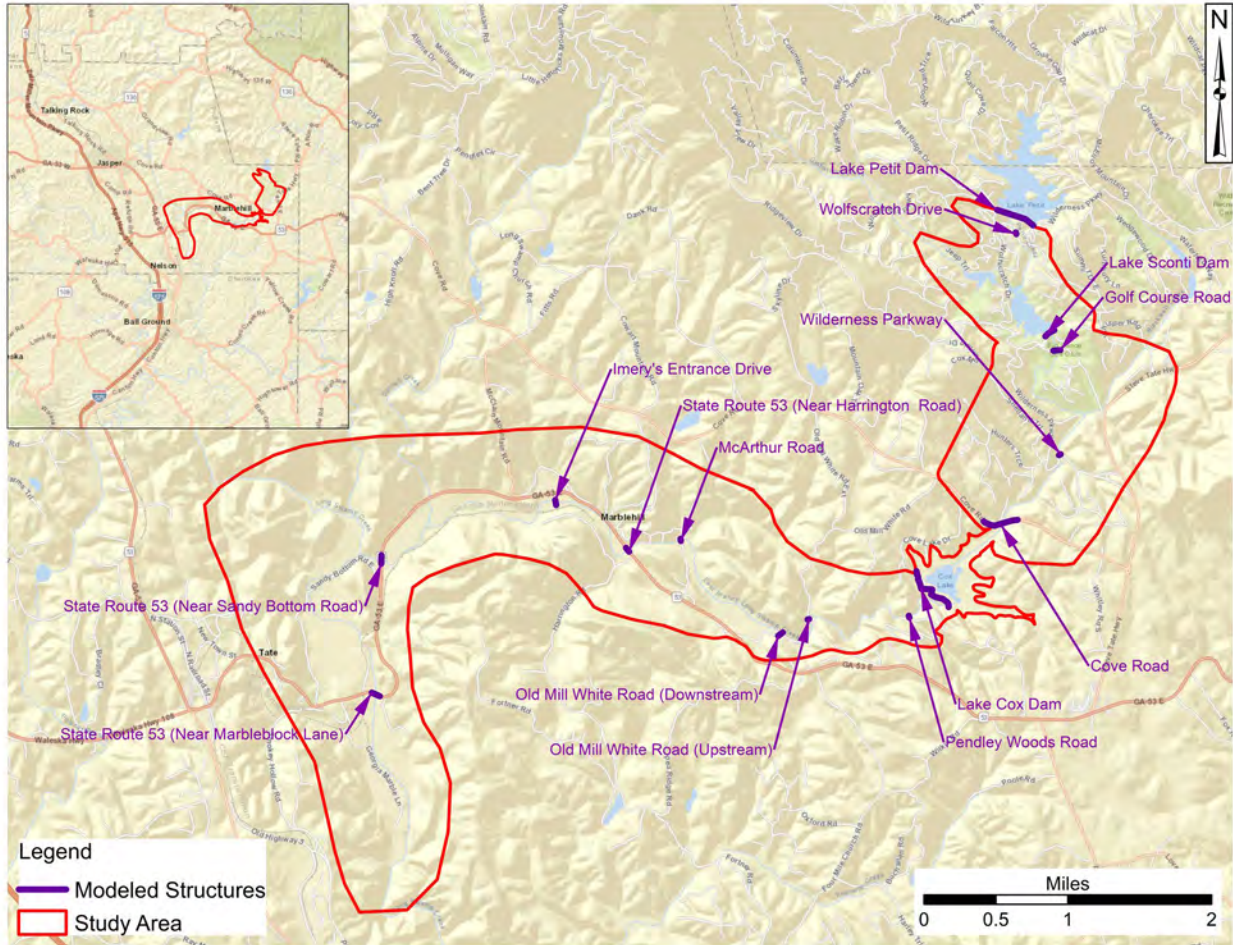
### **2. SITE DESCRIPTION**

Lake Petit Dam is located within the Big Canoe Development on Petit Creek about 5.8 miles upstream of Marble Hill, in Pickens County, north central Georgia. The reservoir formed by the dam has a surface area of 107 acres at a normal pool elevation of 1635.5 and extends up Petit Creek approximately 0.7 miles. The maximum storage volume for the reservoir is approximately 5,635 acre-feet (ac-ft) at maximum water storage elevation 1647.0 feet. The drainage area upstream of Big Canoe Dam is 1.53 square miles. The topography around the dam consists of very steep, wooded, mountainous foothills.

Lake Petit Dam is listed in the Georgia State Safe Dams Program and the National Inventory of Dams (NID) under the following identification numbers, respectively: 112-009-00462 and GA00685. Per the NID, the earth dam has a maximum height of 126 feet and a length of 908 feet. The dam has a 15-foot wide concrete cascading channel spillway on the east side of the earth dam's abutment. The spillway discharge is controlled by a concrete crest underneath a bridge located on the roadway (i.e. Wilderness Parkway) running along the crest of the dam.

Lake Sconti is located approximately 1.0 miles downstream of Lake Petit. Per the NID, Lake Sconti Dam has a maximum height of 45 feet and a length of 209 feet. The dam has a 10-foot wide concrete spillway on the eastern abutment. Cox Lake Dam is located approximately 3.5 miles downstream from Lake Petit Dam south of Cove Road. Per the NID, the dam has a maximum height of 85 feet. The dam has a length of 2,110 feet and a top width of 20 feet as measured on the digital elevation model (DEM).

There are twelve (12) crossings downstream of Lake Petit that are of interest. See Figure 1 for location of dams and bridges. Approximate distance of bridges from Lake Petit are presented in Attachment A.



**Figure 1: Site Location**

### **3. EXISTING CONDITIONS**

The HEC-RAS model used Pickens County Light Detection and Ranging (LiDAR) topography from NOAA (National Oceanic and Atmospheric Administration, 2012) and field data provided by Jordan Engineering (2021). The dates of field work were January 13 and 14, 2021. Field data included bridge and culvert crossing survey. Elevations for the lowest adjacent grades for points of interests are estimates using LiDAR elevations. The bridge survey files along with photos of points of interest are provided in Attachment A. Shapefiles and the digital elevation model are included in Attachment B.

### **4. EMBANKMENT BREACH DEVELOPMENT**

#### **4.1 Modes of Breach Failure**

Typical dam failure modes based on FEMA (2013) area summarized in Table 1.

**Table 1: Typical Dam Failure Modes, FEMA (2013)**

| <b>Failure Mode</b> | <b>Example</b>                      | <b>Percentage of Failures<sup>1</sup></b> |
|---------------------|-------------------------------------|---|
| Hydrologic          | Overtopping                         | 70.9%                                     |
| Geologic            | Piping/Seepage                      | 14.3%                                     |
| Structural          | Failure of upstream/downstream face | 1.8%                                      |
| Seismic             | Earthquake                          | unlisted                                  |
| Human Related       | Misoperation/Terrorism              | 0.6%                                      |

Overtopping is the most frequent failure scenario. In accordance with SDP guidance, it was assumed that a non-hydrologic failure due to overtopping occurs.

#### **4.2 Selected Breach Scenario**

In accordance with SDP guidance, a sunny day failure due to overtopping was chosen for the dam breach analysis. Geosyntec created a HEC-RAS 2D model to simulate the inundation of the downstream area due to embankment failure to evaluate the potential impact due to the directional flow of the breach.

The model was run under the following assumptions:

1. A sunny day mode of failure was assumed;
2. No infiltration losses are assumed;
3. The water surface elevation (WSEL) is 1647.0 ft NAVD 1988 Datum when the breach occurs;
4. The discharge structure is completely blocked; and
5. No losses due to evapotranspiration were considered.

A sunny day failure is appropriate for Lake Petit dam because of the large storage capacity and small inflow drainage area to the Lake. A small drainage area to the pond means that additional flows to the pond would be relatively small during a wet weather event.

All additional downstream structures: Lake Sconti Dam, Lake Cox Dam and twelve (12) crossings were modeled using a sunny day failure. The stage storage tables of Lake Petit, Lake Sconti and Lake Cox are provided in Attachment C.

Table 2 provides dam characteristics used as input in the model. Storage volume at dam failure is the storage capacity at 1,647 feet, 1,470 feet and 1,336 feet elevation for Lake Petit Dam, Lake Sconti Dam and Lake Cox Dam respectively. Lake Petit Dam was surveyed, but publicly available data was used to estimate the characteristics of Lake Sconti Dam and Lake Cox Dam. Embankment top elevation and bottom elevation are elevation as seen in the LiDAR, except for Lake Petit as noted below. Maximum depth is the difference between embankment top elevation and bottom elevation. Embankment lengths represent the length of the structures as drawn in the geometry file

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<sup>1</sup> Based on Table 14-2 of the FEMA Federal Guidelines for Inundation Mapping of Flood Risks Associated with Dam Incidents and Failures (July 2013)

of the model. Embankments are drawn to capture length of the dams and extended further to capture any high ridges at the ends of the dam. Crest widths represent widths of the dams as seen in the LiDAR.

The height of Lake Petit Dam shown in Table 2 is the height from the upstream toe of the dam, not the regulatory height (which is measured from downstream toe). The height from the upstream toe of the dam is the “breachable height” of the dam. Because the model does not compute scour beneath the ground surface, the height is based on the upstream toe of the dam rather than the downstream toe. Elevations are in NAVD 88.

**Table 2: Dam Characteristics**

| <b>Estimated Permitted Area Characteristics<br/>(units)</b> | <b>Lake Petit</b> | <b>Lake Scont</b> | <b>Lake Cox</b> |
|---|-------------------|-------------------|-----------------|
| Storage Volume at Failure (acre-feet)                       | 5,635             | 281               | 4,490           |
| Embankment Top Elevation (feet)                             | 1,647             | 1,470             | 1,336           |
| Bottom Elevation (feet)                                     | 1,538.3*          | 1,430             | 1,239           |
| Maximum Depth of Water Behind Dam (feet)                    | 108.7*            | 40                | 97              |
| Embankment Length (feet)                                    | 1,443             | 398               | 2,110           |
| Embankment Crest Width (feet)                               | 35                | 20                | 20              |

\*Measured from upstream toe of the dam

### **4.3 Breach Characteristics**

Breach characteristics used to develop the breach hydrograph include shape, final depth, width, side slopes, breaching time, and the rate at which the breach develops. Table 3 is based on the guidelines provided by SDP (2015). This was used as an aid to determine breach characteristics.

**Table 3: SDP Suggested Breach Parameters (Embankment Dams)**

| <b>Failure Mode</b>          |                  |
|------------------------------|------------------|
| Minimum Average Breach Width | 3x Height of dam |
| Side Slope (H:1V)            | 1                |
| Maximum Breach Time (hour)   | 0.5              |

The Froehlich (2008) equations were used for computing the average breach widths and times of embankment dams, but with a minimum average breach width of three (3) times the height of the dam, and a maximum breach time of a half (½) hour. These equations have been found to correlate well with breach widths for actual failures for every dam size, and they are widely used and respected in the engineering community. Side slopes for the breach were modeled as a 1 horizontal to 1 vertical (1H:1V) slope based on the guidelines provided by SDP (2015). Because Cove Road is a large embankment crossing, it was modeled the same way as the embankment dams with the Froehlich (2008) equations. The other road crossings are generally much smaller and constructed of rigid materials (wood, concrete). In order to model the other (non-Cove Road) crossings, the following assumptions were made:

1. The model assumes obstruction on the bottom half of all downstream bridges and culverts.

2. One (1) acre-foot of pool volume at failure has been assumed for all bridges for calculation of breach parameters. The one (1) acre-foot of pool volume was considered a conservative assumption because it resulted in a faster breach time of approximately three (3) minutes.
3. All downstream structures breach when overtopped with two feet of flow, or more.
4. The entire bridge structure collapses due to the breach and washes away. The bottom breach widths were calculated using Froelich (2008) and widened to match the breach opening to the riverbed.

The average breach width and breach development time based on Froelich (2008) used for the analysis are summarized in Table 4. Breach parameter calculation for the three dams and Cove Road are presented in Attachment D.

**Table 4: Summary of Breach Parameters used for Dam Breach Analysis**

| <b>Modeled Dams and Bridges</b>                | <b>Connection name used in the model</b> | <b>Average Breach Width (feet)</b> | <b>Breach Bottom Width (feet)</b> | <b>Breach Time (hour)</b> |
|--|--|------------------------------------|-----------------------------------|---------------------------|
| <b>Lake Petit Dam</b>                          | <b>Petit Dam</b>                         | 326.1                              | 217.4                             | 0.45                      |
| <b>Wolfscratch Drive</b>                       | <b>1. Wolfscratch D</b>                  | 44*                                | 25                                | 0.03                      |
| <b>Lake Sconti Dam</b>                         | <b>Sconti Dam</b>                        | 120                                | 80                                | 0.27                      |
| <b>Golf Course Road</b>                        | <b>2. Golfcourse Ro</b>                  | 99                                 | 80                                | 0.03                      |
| <b>Wilderness Parkway</b>                      | <b>3. Wilderness Pa</b>                  | 45*                                | 12                                | 0.03                      |
| <b>Cove Road</b>                               | <b>Cove Road</b>                         | 132                                | 88                                | 0.1                       |
| <b>Lake Cox Dam</b>                            | <b>Cox Dam</b>                           | 291                                | 194                               | 0.45                      |
| <b>Pendley Woods Road</b>                      | <b>4. Pendley Woods</b>                  | 11*                                | 3                                 | 0.08                      |
| <b>Old Mill White Road (Upstream)</b>          | <b>5. Old Mill Upst</b>                  | 47*                                | 25                                | 0.08                      |
| <b>Old Mill White Road (Downstream)</b>        | <b>6. Old Mill Down</b>                  | 39*                                | 25                                | 0.04                      |
| <b>McArthur Road</b>                           | <b>7. McArthur Road</b>                  | 32                                 | 25                                | 0.09                      |
| <b>State Route 53 (Near Harrington Road)</b>   | <b>8. Harrington Ro</b>                  | 106                                | 90                                | 0.04                      |
| <b>Imerys Entrance Drive</b>                   | <b>09. To Imery Pla</b>                  | 37                                 | 25                                | 0.05                      |
| <b>State Route 53 (Near Sandy Bottom Road)</b> | <b>10. State Route</b>                   | 142                                | 120                               | 0.03                      |
| <b>State Route 53 (Near Marbleblock Lane)</b>  | <b>11. Route 53 - M</b>                  | 113                                | 85                                | 0.02                      |

\*Average breach width does not meet the SDP guidelines that minimum average breach width should be three (3) times the height of the dam. The bridge openings were widened to fit a natural ground profile. Widening breach bottom width further in order to increase average breach width would have made the bridge opening wider than the natural ground profile.

## 5. EMBANKMENT BREACH ANALYSIS

### 5.1 Breach Model Development

HEC-RAS 2D (HEC-RAS 5.0.7, 2019) modeling software was used to develop breach inundation maps for the study area. The following sections describe select parameters and assumptions used in developing the HEC-RAS 2D model.

#### *Model Mesh*

The downstream area was modeled with a computational mesh of 100-foot by 100-foot cells. Instead of computing an average elevation for each cell like competitive 2D modeling software, HEC-RAS 2D utilizes meshes for each cell such that the resolution of the underlying topography is not lost. The extent of 2D modeling domain is shown in Figure 1 as Study Area. 2D flow area break lines were added along modeled structures, roads and terrain ridges. Break lines force cell faces to follow terrain ridges and ensure the flow is going over the ridge lines.

#### *Topography Data*

2012 LiDAR data provided by NOAA (National Oceanic and Atmospheric Administration) was used to create a digital elevation model of the land surrounding the project area.

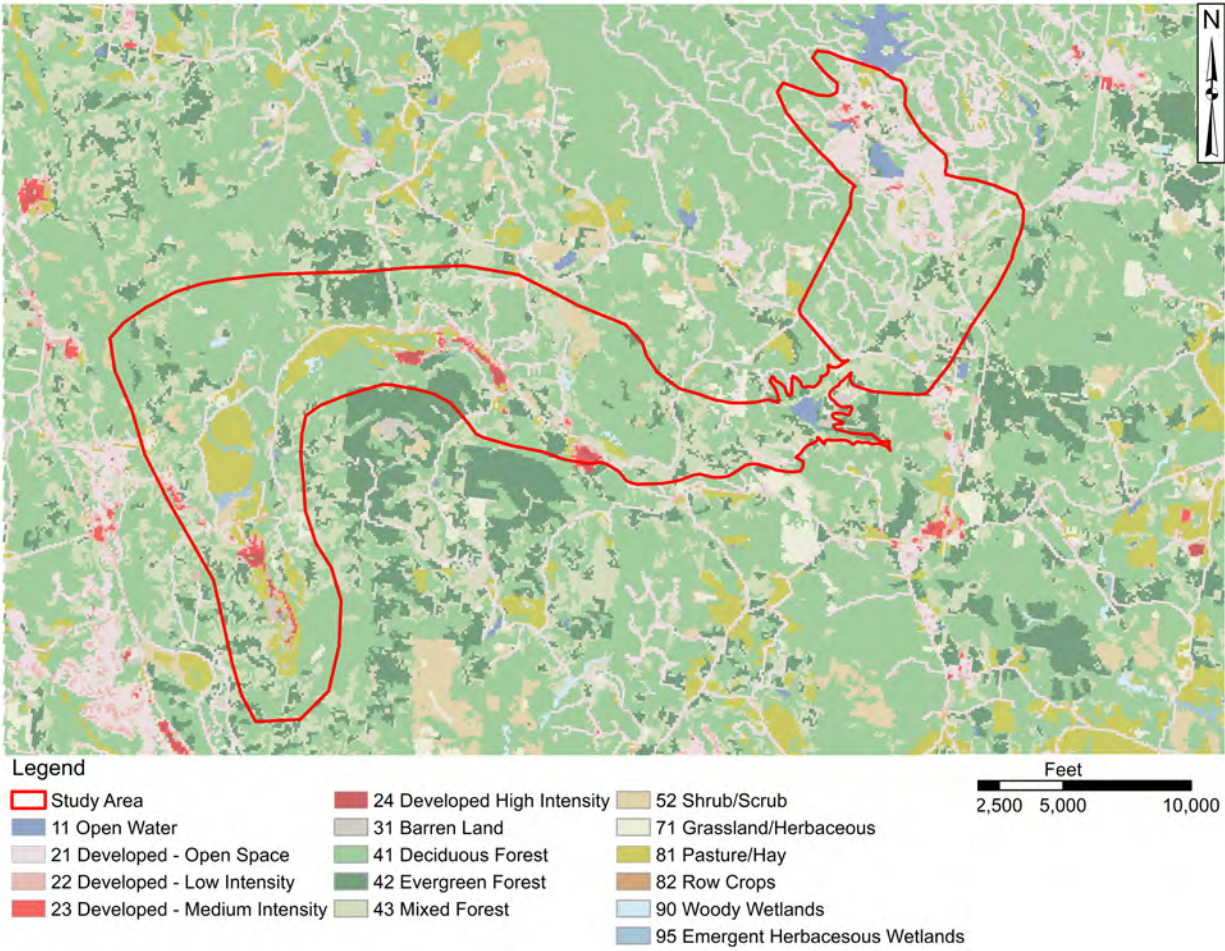
#### *Floodplain Manning's Roughness Values*

Landuse data was downloaded from National Land Cover Database (NLCD, 2016) and Manning's roughness values were assigned based on recommendations from Natural Resource Conservation Service provided in "Manning's n Values for Various Land Covers" (USDA, 2016). Table 5 provides a summary of the Manning's roughness values assigned for each land use. Figure 2 shows the extent of different land uses in the model.

**Table 5: Floodplain Manning's Roughness**

| <b>Land Use</b>              | <b>Manning's Roughness</b> |
|------------------------------|----------------------------|
| Barren land rock/sand/clay   | 0.025                      |
| Deciduous forest             | 0.16                       |
| Developed, high intensity    | 0.15                       |
| Developed, low intensity     | 0.10                       |
| Developed, medium intensity  | 0.08                       |
| Developed, open space        | 0.04                       |
| Emergent herbaceous wetlands | 0.07                       |
| Evergreen forest             | 0.16                       |
| Grassland/herbaceous         | 0.035                      |
| Mixed forest                 | 0.16                       |
| Open water                   | 0.04                       |
| Pasture/hay                  | 0.03                       |
| Shrub/scrub                  | 0.10                       |
| Woody wetlands               | 0.12                       |





**Figure 2: Extent of different land uses in the modeling domain**

*Unsteady Flow Data*

The initial boundary condition was set to top of dam elevation 1,647 feet for Lake Petit and to normal water surface elevation 1,464.3 feet and 1,276.2 feet for Lake Sonti and Lake Cox, respectively. Water was assumed to flow off the grid with a normal depth boundary condition at the south-most location of the study area.

*Computational Settings*

An adaptive timestep based on courant number, mapping output of 5 minutes, hydrograph output of 1 minute and detailed output of 1 minute were selected for the run. The model was developed and refined with a diffusion wave equation and the final run utilized the full momentum equation as recommended by the HEC-RAS manual for dam breach analyses. The settings and tolerances used in HEC-RAS are shown in Attachment E.

*Breach of Structures*

A total of three (3) dams and twelve (12) crossings were modeled to be able to breach when overtopped with two (2) feet. The bridges consisted of circular culverts, rectangular box culverts or piers. The opening in Cove Road is small and was not included in modeling as a conservative measure. Due to limitations of HEC-RAS 5.0.7 version to model bridges; all bridge openings were

created using culverts. Box culverts with spacing were used to represent piers. The breach characteristics and assumptions are discussed in Section 4.3. The bridge profiles are provided in Attachment A.

## **5.2 Results**

The 2D HEC-RAS model was run with the inputs described above. The breach analysis found that Lake Petit Dam and Lake Sconti Dam breached while Lake Cox overtopped but did not breach. Lake Disharoon Dam was inundated. All modeled downstream bridges (see Table 4), except State Route 53 (Near Marbleblock Lane), breached during the simulation. The maximum inundation depths and velocity for the 2D flow region are shown in Attachment F and Attachment G respectively. See Attachment H for summary of warnings and notes.

Please note that when the model was run with the full momentum equation, the full receding limb of the hydrograph was not observed in the downstream model domain; an apparent bug in the model. The results of the diffusion wave equation and the full momentum equation were compared, and it appears that the full momentum equation best represents the timing and intensity of the breach wave, as anticipated from HEC-RAS guidance. The diffusion wave equation is able to model the receding limb of the hydrograph and confirms that flow rates and water levels recede over time. The diffusion wave hydrographs were also smooth compared to some observed minor instability in the full momentum hydrographs.

### **5.2.1 Hazard Potential**

The hazard classification is based on simulated flow depth and velocity downstream of Lake Petit. The areas that are shown to be inundated have habitable structures and public roads as shown in inundation map (Attachment F).

### **5.2.2 Probable Loss of Life**

Loss of life is considered probable by the SDP when any of the following conditions exist:

1. A structure is flooded by 18 inches or more of water above finished floor elevation.
2. A structure is flooded by 30 inches or more of water against the building at lowest adjacent grade.
3. A structure is flooded such that the destruction factor (maximum velocity in feet per second x maximum depth in feet) is equal to or greater than 15.
4. An unanchored mobile home is flooded such that the destruction factor is equal to or greater than 9.
5. A structure which is flooded such that the destruction factor is 7 or greater shall be evaluated using engineering judgment to determine if other factors warrant a probable loss of life designation.

The SDP recommends classification of dams according to whether probable loss of life is present downstream. The maximum destruction factor raster was used to extract factors at the points of interests using GIS. Based on destruction factors higher than 15 for majority of points of interests and inundation areas encompassing existing structures, the breach of the dam would likely result in loss of life. Destruction factor at points of interests are provided in Table 6. The destruction factors are calculated as maximum velocity in feet per second times the maximum depth in feet.

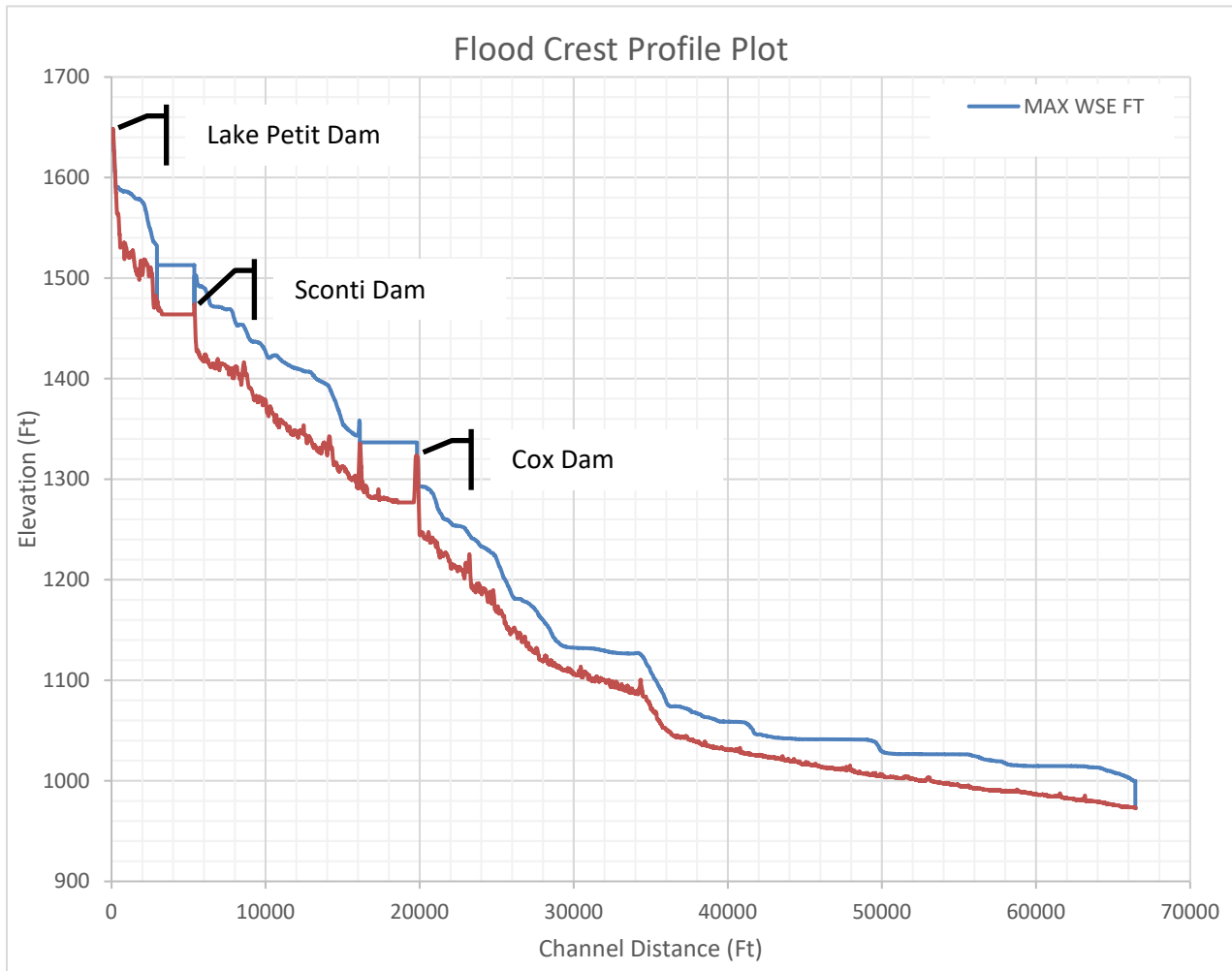
Maximum overtopping depths (simulated depth over the point of interest) from a breach of Lake Petit Dam are calculated as shown in Table 7. Maximum velocity is taken at the location of the modeled structures as seen in velocity map in Attachment F. Based on the lateral extent of the inundation zone, the list of addresses potentially affected by the breach of Lake Petit Dam were provided by the Pickens County 911 on September 30<sup>th</sup>, 2022 and are available in Attachment I. Pickens County included structures within a 100-foot buffer outside of the extent of the mapped inundation zone.

**Table 6: Destruction Factor at Points of Interest**

| <b>Points of Interest</b>                      | <b>Maximum Overtopping Depth (ft)</b> | <b>Maximum Velocity (ft/sec)</b> | <b>Destruction Factor (sq.ft/sec)</b> |
|--|---------------------------------------|----------------------------------|---------------------------------------|
| <b>Wolfscratch Drive</b>                       | 48.0                                  | 11.7                             | 559.8                                 |
| <b>Lake Sconti Dam</b>                         | 42.8                                  | 17.4                             | 743.1                                 |
| <b>Golf Course Road</b>                        | 41.6                                  | 15.8                             | 657.4                                 |
| <b>Wilderness Parkway</b>                      | 36.3                                  | 16.4                             | 597.3                                 |
| <b>Cove Road</b>                               | 6.7                                   | 12.3                             | 82.0                                  |
| <b>Lake Cox Dam</b>                            | 1.0                                   | 6.2                              | 5.9                                   |
| <b>Pendley Woods Road</b>                      | 34.1                                  | 13.5                             | 458.9                                 |
| <b>Old Mill White Road (Upstream)</b>          | 20.7                                  | 9.7                              | 199.9                                 |
| <b>Old Mill White Road (Downstream)</b>        | 19.5                                  | 14.1                             | 275.0                                 |
| <b>McArthur Road</b>                           | 32.9                                  | 7.0                              | 229.1                                 |
| <b>State Route 53 (Near Harrington Road)</b>   | 9.4                                   | 9.4                              | 88.3                                  |
| <b>Imerys Entrance Drive</b>                   | 16.0                                  | 9.9                              | 159.1                                 |
| <b>State Route 53 (Near Sandy Bottom Road)</b> | 12.2                                  | 10.5                             | 128.9                                 |
| <b>State Route 53 (Near Marbleblock Lane)</b>  | 1.5                                   | 9.4                              | 13.6                                  |

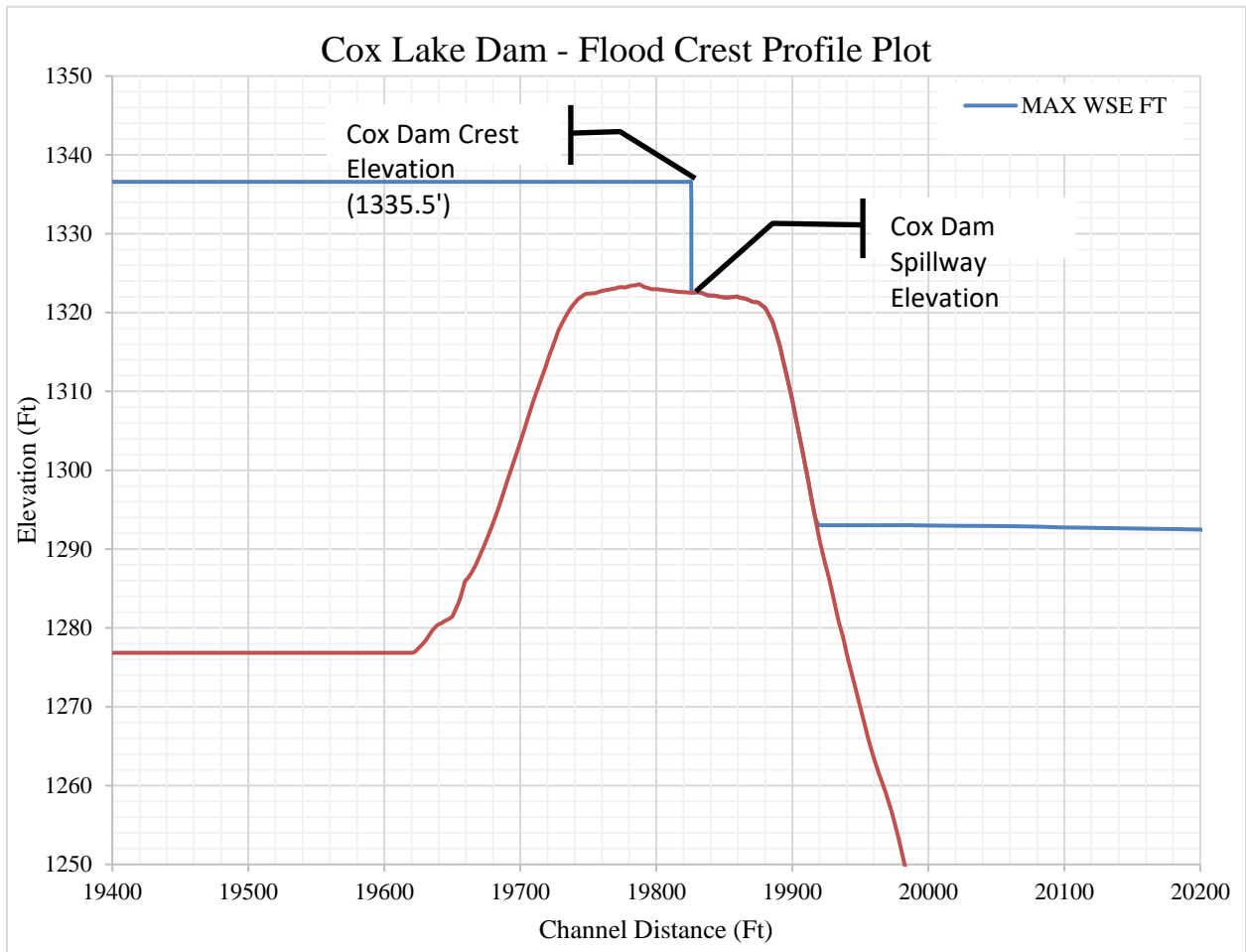
### 5.2.1 Flood Crest Profile Plot

Figure 3 presents the flood crest profile plot showing the three dams mentioned. Lake Petit Dam and Sconti Dam breached as they were overtopped by two (2) feet. Cox Dam did not breach. The depth of overtopping for all structures are presented in Table 7. See Attachment J for hydrographs at each of the points of interest listed in Table 7.

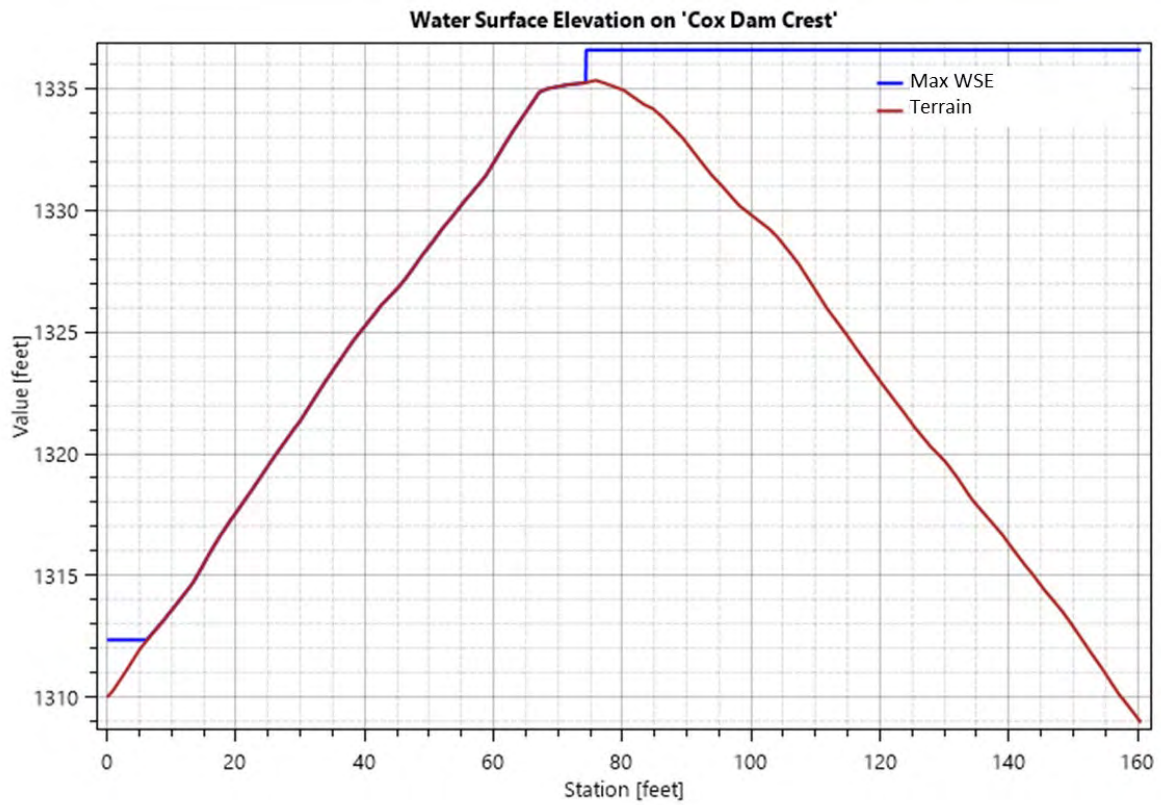


**Figure 3: Flood Crest Profile Plot**

Based on the profile plot (Figure 3), Lake Cox Dam appears to overtop with depth greater than two feet indicating breach of Lake Cox Dam however, computation message from HEC-RAS and hydrograph directly taken from HEC-RAS confirm that Lake Cox Dam overtopped by one foot and did not breach. The maximum water surface elevation at Lake Cox Dam is 1,336.5 feet and the dam crest is at 1,335.5 feet as seen in terrain profile. The ground elevation as seen in the profile plots are based on a profile line that goes through the spillway and thus shows a lower ground elevation of approximately 1,324 feet. See Figure 4 for terrain profile of Lake Cox Dam and Figure 5 for zoomed in flood crest profile plot at Lake Cox Dam.



**Figure 4: Zoomed-in Flood Crest Profile at Lake Cox Dam**



**Figure 5: Terrain Profile - Lake Cox Dam**

**Table 7: Overtopping Depths at Points of Interest (Elevations in NAVD 88)**

| <b>Points of Interest</b>                          | <b>High Chord<br/>(feet)</b> | <b>Max Stage Elevation<br/>(feet)</b> | <b>Maximum<br/>Overtopping Depth<br/>(feet)</b> |
|--|------------------------------|---------------------------------------|---|
| <b>Wolfscratch Drive</b>                           | 1538.0                       | 1586.0                                | 48.0  |
| <b>Lake Sconti Dam</b>                             | 1470.0                       | 1512.8                                | 42.8  |
| <b>Golf Course Road</b>                            | 1447.8                       | 1489.4                                | 41.6  |
| <b>Wilderness Parkway</b>                          | 1371.5                       | 1407.9                                | 36.3  |
| <b>Cove Road</b>                                   | 1335.0                       | 1341.7                                | 6.7   |
| <b>Lake Cox Dam</b>                                | 1335.5                       | 1336.5                                | 1.0   |
| <b>Pendley Woods Road</b>                          | 1230.1                       | 1264.2                                | 34.1  |
| <b>Old Mill White Road<br/>(Upstream)</b>          | 1160.1                       | 1180.8                                | 20.7  |
| <b>Old Mill White Road<br/>(Downstream)</b>        | 1129.8                       | 1149.3                                | 19.5  |
| <b>McArthur Road</b>                               | 1093.6                       | 1126.5                                | 32.9  |
| <b>State Route 53 (Near<br/>Harrington Road)</b>   | 1066.3                       | 1075.7                                | 9.4   |
| <b>Imerys Entrance<br/>Drive</b>                   | 1042.8                       | 1058.8                                | 16.0  |
| <b>State Route 53 (Near<br/>Sandy Bottom Road)</b> | 1028.7                       | 1040.9                                | 12.2  |
| <b>State Route 53 (Near<br/>Marbleblock Lane)</b>  | 1017.8                       | 1019.3                                | 1.5   |

## **6. CONCLUSIONS**

This report summarizes the results of the dam breach analysis for Lake Petit Dam. The findings of the inundation model are consistent with Lake Petit Dam’s current classification as a Category I, Very Large Dam per the Georgia SDP Engineering Guidelines. This analysis also provides an updated inundation map for evacuation considerations during a breach failure.

## 7. REFERENCES

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**ATTACHMENT A**

**BRIDGE SURVEY, PROFILE AND PHOTOS OF POINTS OF INTEREST**

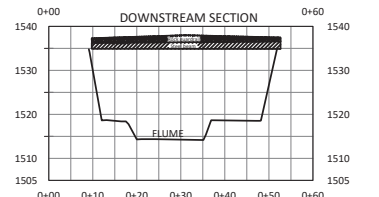
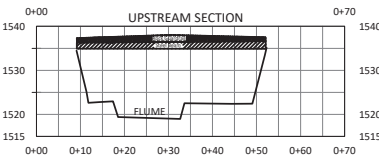
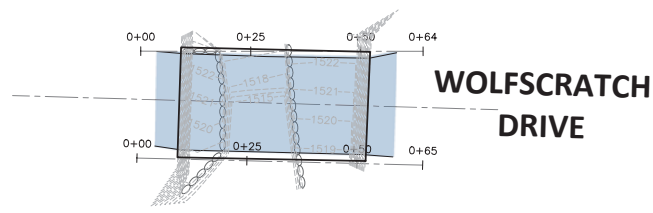
**Approximate Distance of Structures from Lake Petit Dam**

| <b>Modeled Dams and Bridges</b>                | <b>Distance (Miles) from Lake Petit Dam</b> |
|--|---|
| <b>Lake Petit Dam</b>                          | 0   |
| <b>Wolfscratch Drive</b>                       | 0.2   |
| <b>Lake Sconti Dam</b>                         | 1.0   |
| <b>Golf Course Road</b>                        | 1.2   |
| <b>Wilderness Parkway</b>                      | 2.4   |
| <b>Cove Road</b>                               | 3.1   |
| <b>Lake Cox Dam</b>                            | 3.8   |
| <b>Pendley Woods Road</b>                      | 4.0   |
| <b>Old Mill White Road (Upstream)</b>          | 5.0   |
| <b>Old Mill White Road (Downstream)</b>        | 5.4   |
| <b>McArthur Road</b>                           | 6.5   |
| <b>State Route 53 (Near Harrington Road)</b>   | 6.8   |
| <b>Imerys Entrance Drive</b>                   | 7.6   |
| <b>State Route 53 (Near Sandy Bottom Road)</b> | 9.3   |
| <b>State Route 53 (Near Marbleblock Lane)</b>  | 10.8  |

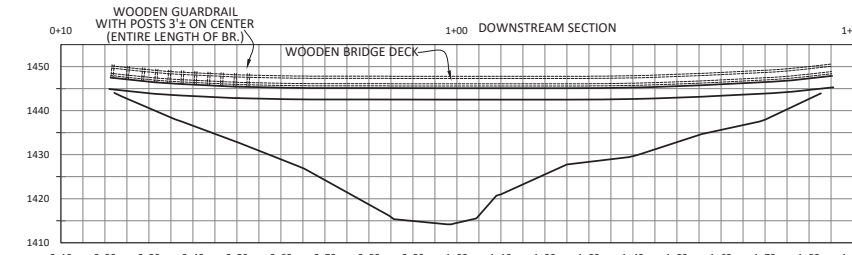
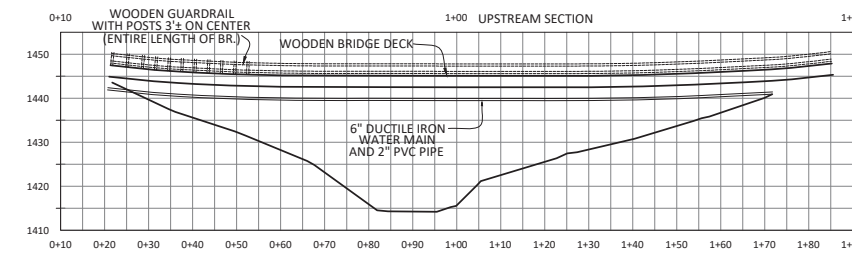
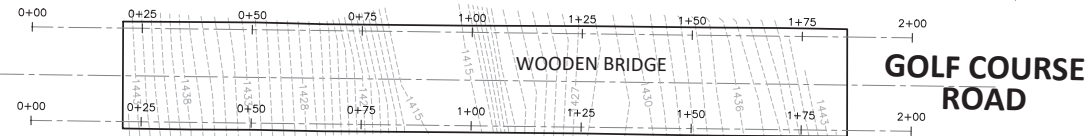


### SITE 1 WOLFSCRATCH DRIVE

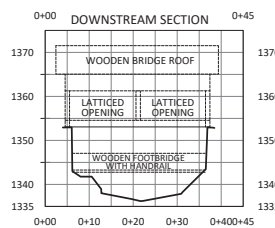
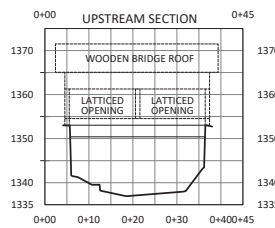
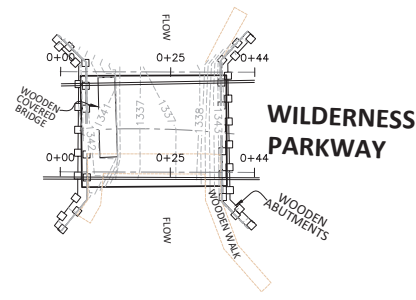
PETTIT CREEK



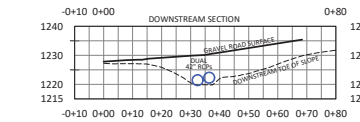
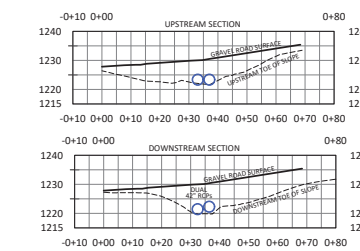
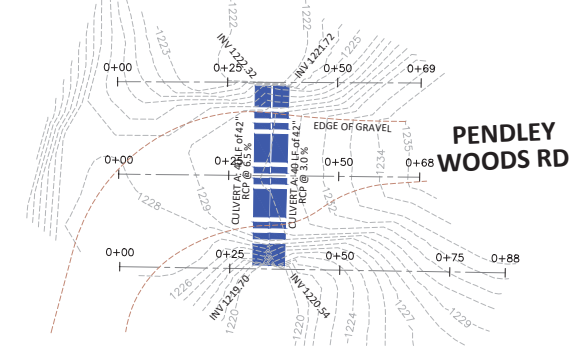
### SITE 5 GOLF COURSE ROAD



### SITE 6 WILDERNESS PARKWAY



### SITE 10 PENDLEY WOODS ROAD



- SITES:**
- 1 - WILDERNESS PARKWAY
  - 5 - GOLF COURSE ROAD
  - 6 - WILDERNESS PARKWAY
  - 10 - PENDLEY WOODS ROAD

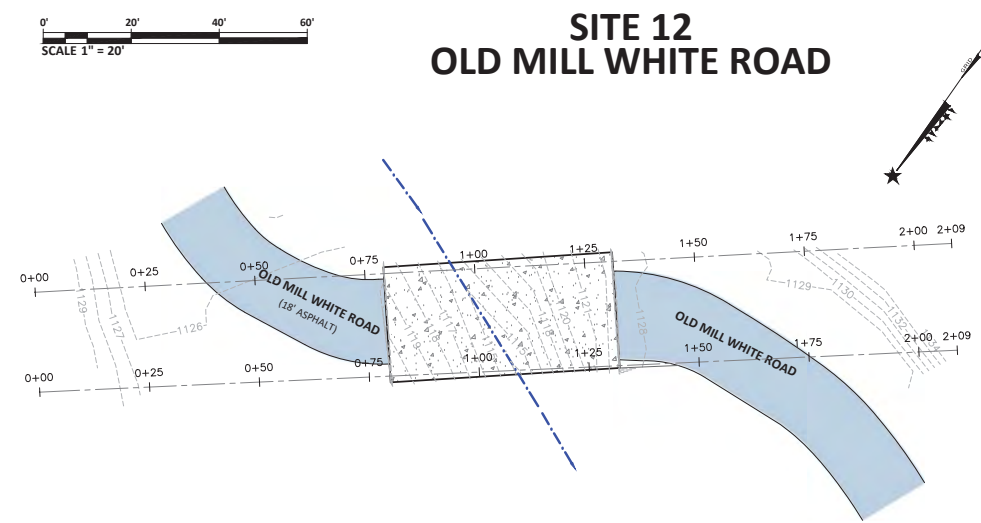
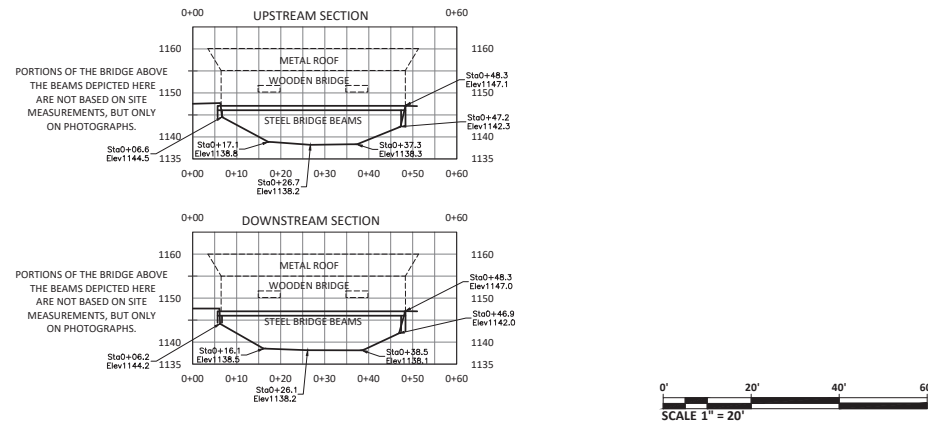
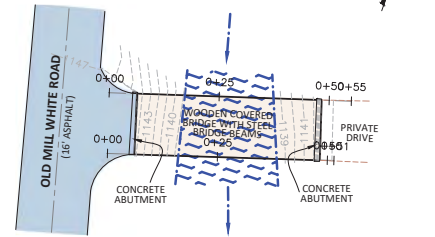
Big Canoe Stream Sections  
Pickens County, Georgia

| Rev | Revision Description | Date     |
|-----|----------------------|----------|
| 0   | Initial Issue        | XX/XX/XX |

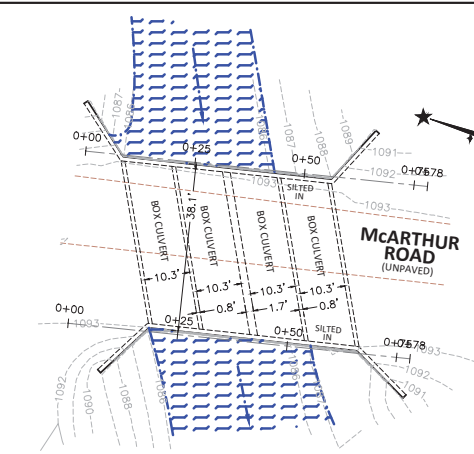
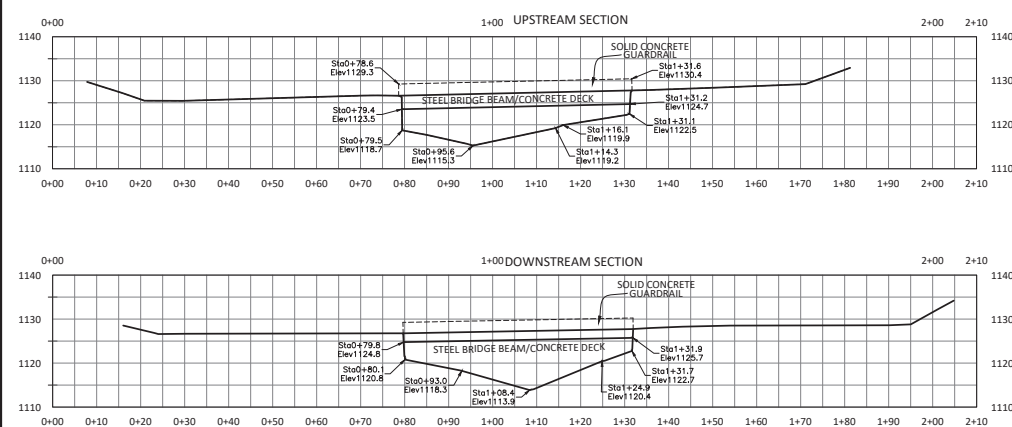
Sheet No.



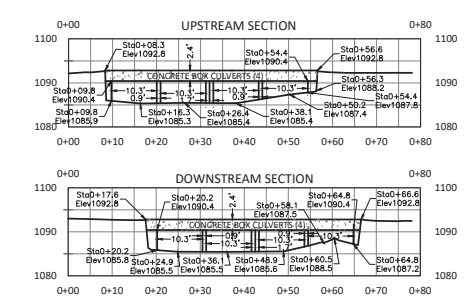
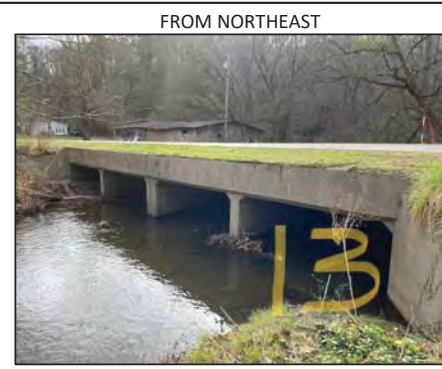
**SITE 11  
OLD MILL WHITE ROAD  
(JUSTICE WAY)**



**SITE 12  
OLD MILL WHITE ROAD**



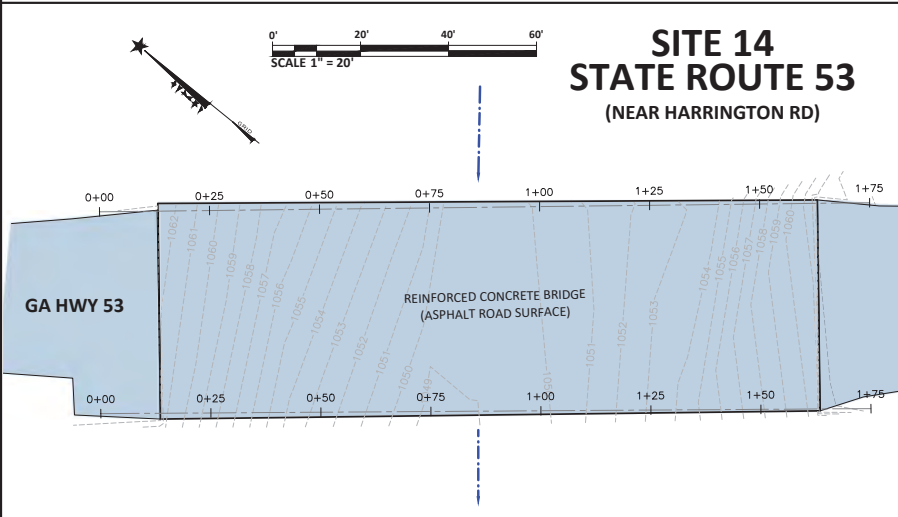
**SITE 13  
McARTHUR ROAD**



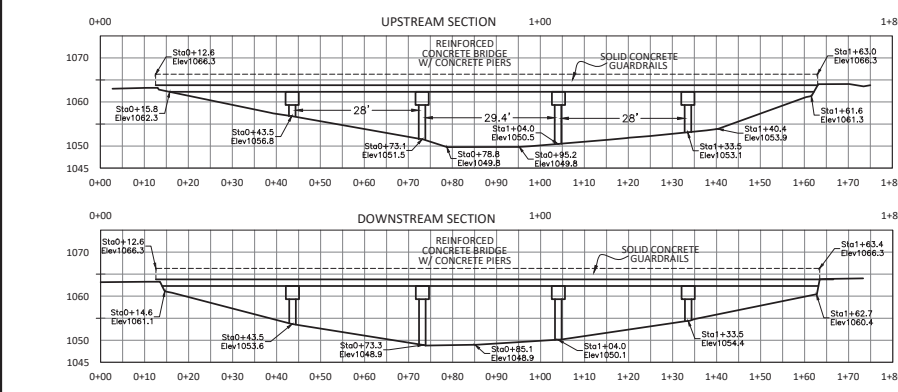
**FROM NORTHWEST**



**FROM NORTH**



**SITE 14  
STATE ROUTE 53  
(NEAR HARRINGTON RD)**



**FROM WEST**

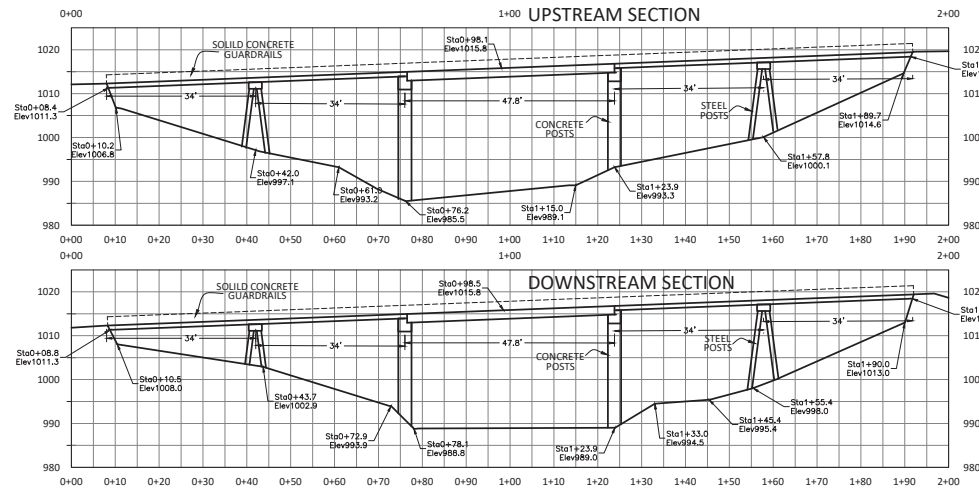
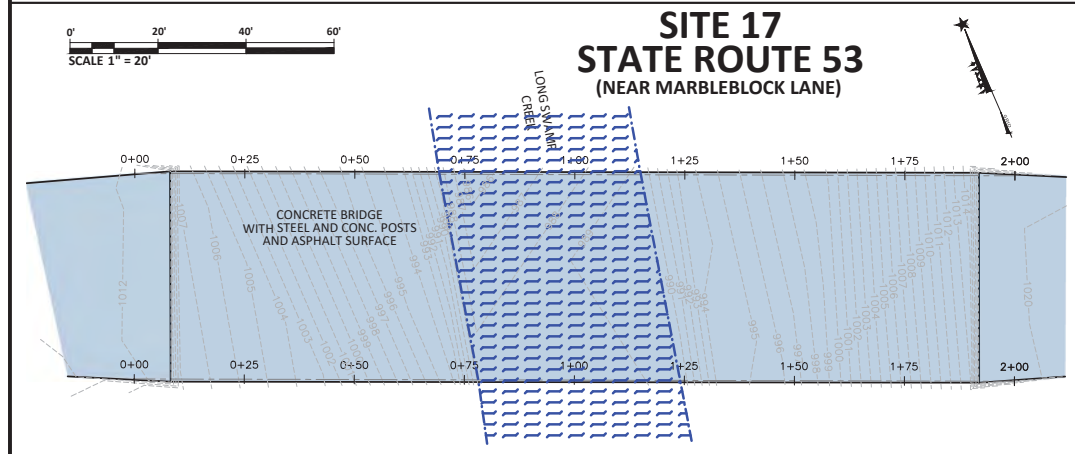
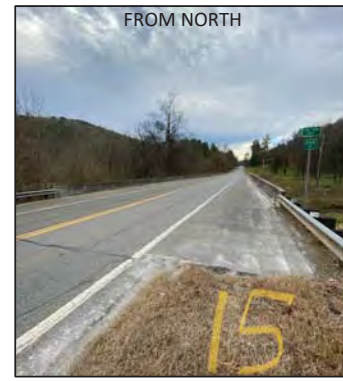
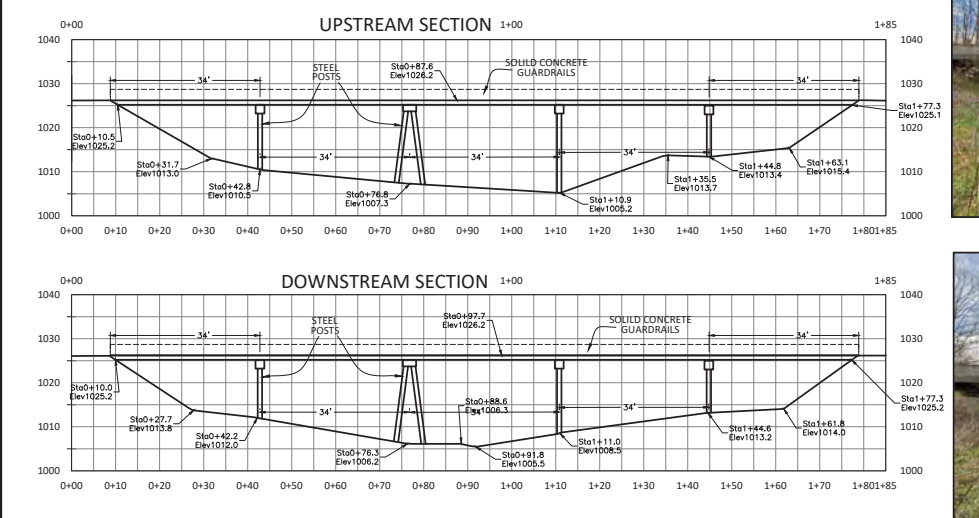
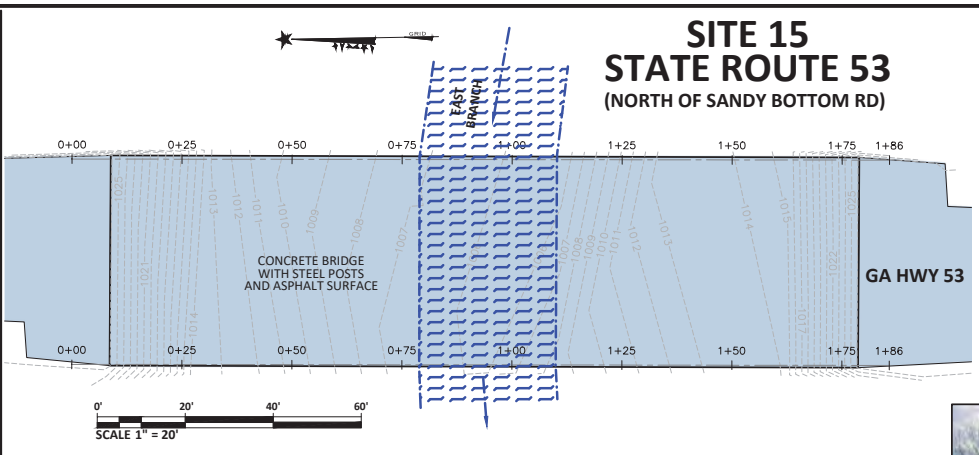
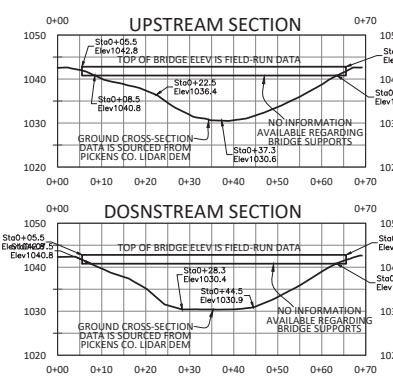
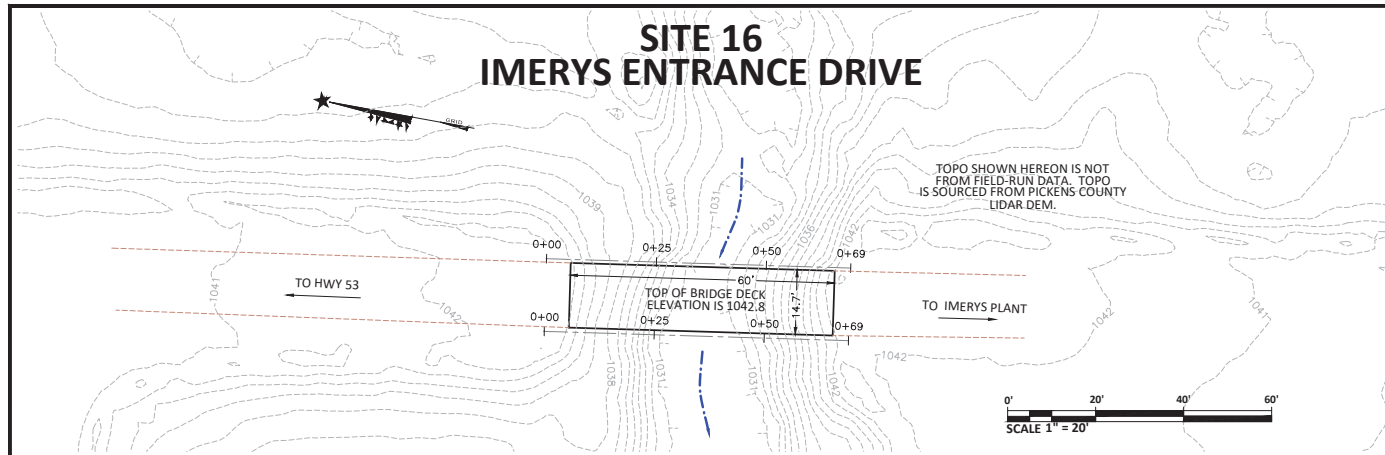
**FROM EAST**



**SITES:**  
11 - OLD MILL WHITE ROAD  
12 - OLD MILL WHITE ROAD  
13 - McARTHUR ROAD  
14 - STATE ROUTE 53 (HARRINGTON RD)

**Big Canoe Stream Sections**  
Pickens County, Georgia

| Rev | Revision Description    | Date     |
|-----|-------------------------|----------|
| 0   | Initial Issue - Sheet 1 | 01/22/21 |



**JORDAN**  
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Land Planning/Surveying/Soils Classification

GEORGIA REGISTERED  
LAND SURVEYOR  
No. 2902  
ROBERT Q. JORDAN

SITES:  
16 - IMERYS ENTRANCE DRIVE  
15 - STATE ROUTE 53  
17 - STATE ROUTE 53

Big Canoe Stream Sections  
Pickens County, Georgia

| Rev | Revision Description           | Date     |
|-----|--------------------------------|----------|
| 0   | Initial Issue - Sheet 1        | 01/22/21 |
| 1   | Initial Issue - Sheets 2 and 3 | 01/28/21 |

Sheet No.  
**3 of 3**

## **ATTACHMENT B**

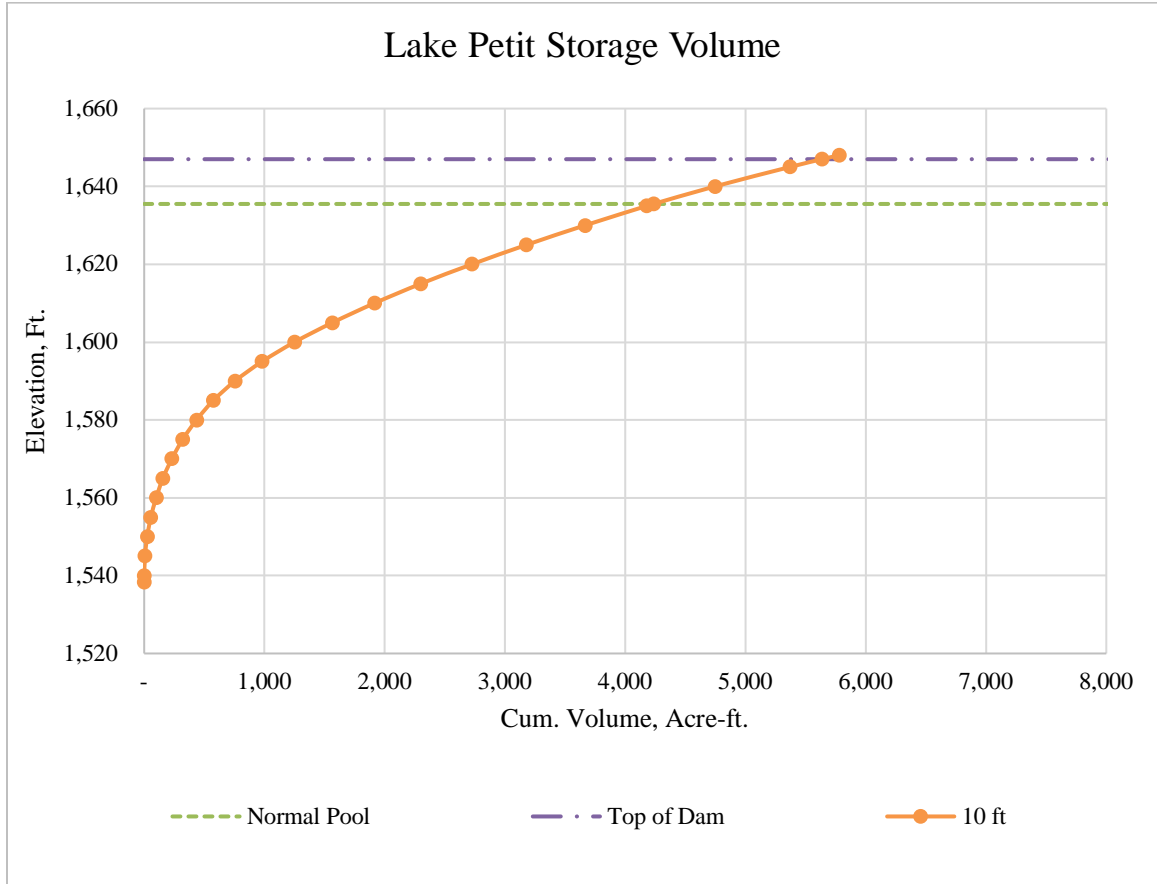
### **ELECTRONIC COPY WITH DIGITAL FILES (ON CD)**

#### List of files Provided on CD

1. Shapefiles
  - a. Dam Breach Location
  - b. Inundation Boundary (Limits of Inundation)
  - c. Inundation Raster
  - d. Velocity Raster
  - e. Potential Hazards Labeled with Address
  - f. Distance Downstream
  - g. Points of Interest
    - i. Distance downstream
    - ii. Name of Road or Dam
    - iii. Depth of Overtopping
    - iv. Maximum Flow Velocity
    - v. Destruction Factor
    - vi. Lowest Adjacent Grade
  - h. Digital Elevation Model
2. HEC-RAS Model

**ATTACHMENT C**  
**STAGE STORAGE TABLES**

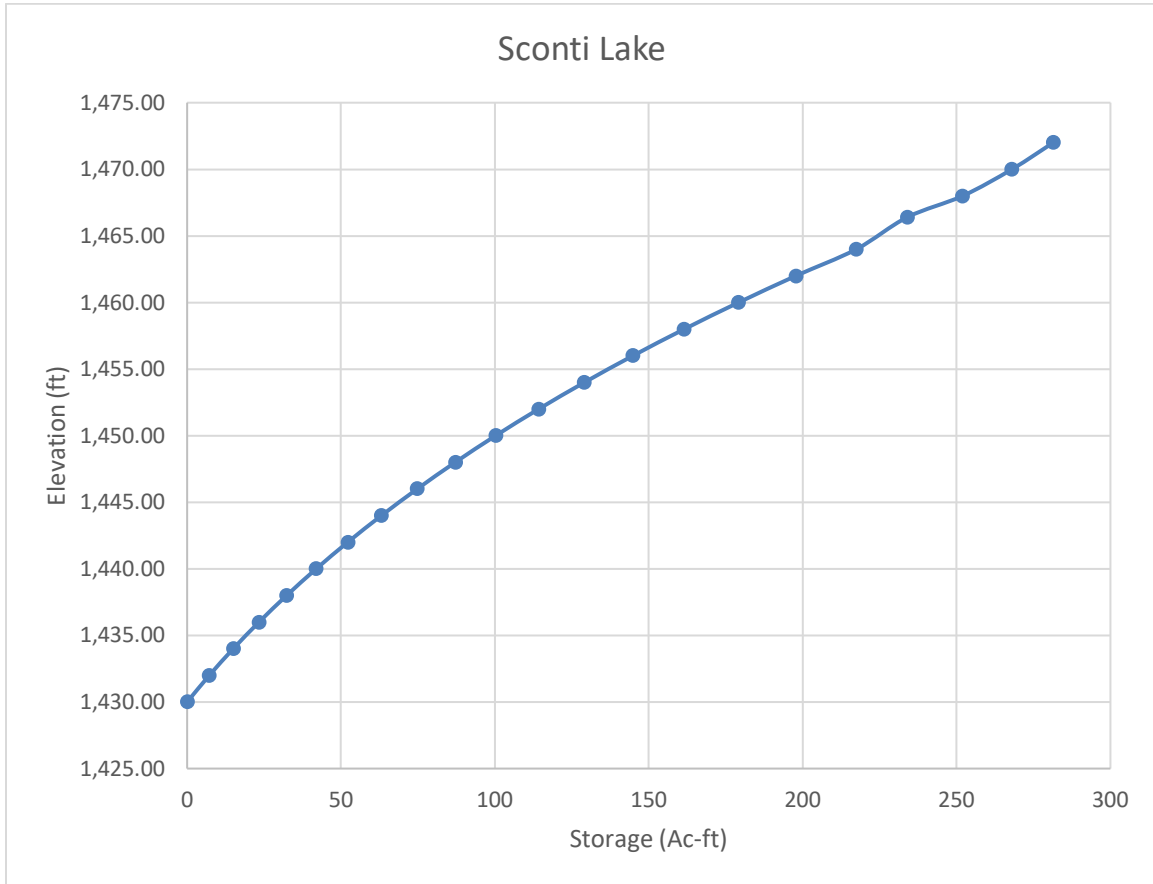
1. Lake Petit Stage-storage Table



| Elevation (ft) | Volume (Acre-ft) |
|----------------|------------------|
| 1,538.38       | 0                |
| 1,540          | 0.4              |
| 1,560          | 100              |
| 1,580          | 435              |
| 1,600          | 1250             |
| 1,620          | 2725             |
| 1,635          | 4180             |
| 1,647          | 5635             |
| 1,648          | 5780             |

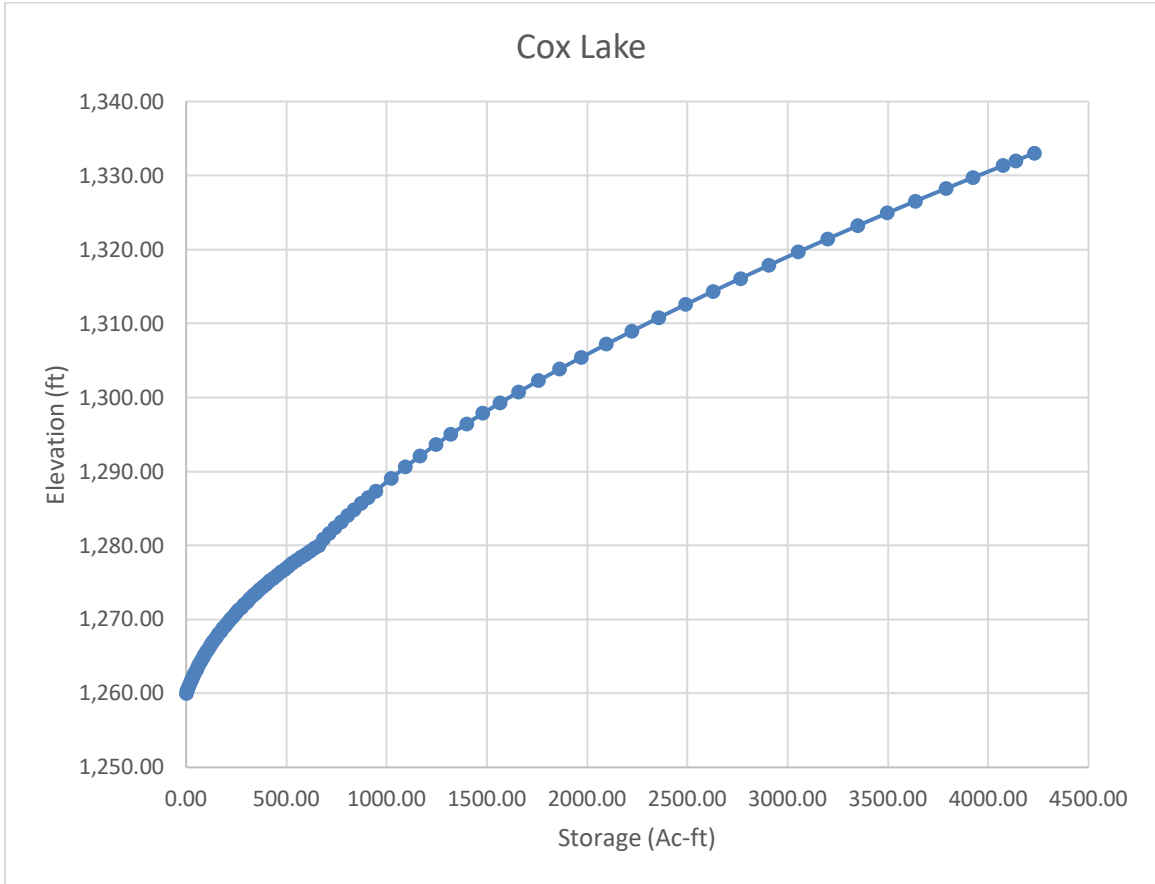


## 2. Lake Sconti Stage-storage Table



| Elevation (ft) | Volume (Acre-ft) |
|----------------|------------------|
| 1,430.00       | 0                |
| 1,432.00       | 7.21             |
| 1,434.00       | 14.99            |
| 1,436.00       | 23.36            |
| 1,438.00       | 32.34            |
| 1,440.00       | 41.95            |
| 1,442.00       | 52.21            |
| 1,444.00       | 63.15            |
| 1,446.00       | 74.79            |
| 1,448.00       | 87.14            |
| 1,450.00       | 100.27           |
| 1,452.00       | 114.26           |
| 1,454.00       | 129.12           |
| 1,456.00       | 144.88           |
| 1,458.00       | 161.56           |
| 1,460.00       | 179.21           |
| 1,462.00       | 197.83           |
| 1,464.00       | 217.46           |
| 1,466.40       | 234              |
| 1,468.00       | 252              |
| 1,470.00       | 268              |
| 1,472.00       | 281.40           |

### 3. Lake Cox Stage-storage Table



| Elevation (ft) | Volume (Acre-ft) |
|----------------|------------------|
| 1,260.00       | 0.00             |
| 1,260.40       | 5.57             |
| 1,260.80       | 11.38            |
| 1,261.20       | 17.42            |
| 1,261.60       | 23.71            |
| 1,262.00       | 30.25            |
| 1,262.40       | 37.05            |
| 1,262.80       | 44.10            |
| 1,263.20       | 51.42            |
| 1,263.60       | 59.00            |
| 1,264.00       | 66.86            |
| 1,264.40       | 75.00            |
| 1,264.80       | 83.42            |
| 1,265.20       | 92.13            |
| 1,265.60       | 101.13           |
| 1,266.00       | 110.43           |
| 1,266.40       | 120.03           |
| 1,266.80       | 129.94           |
| 1,267.20       | 140.16           |
| 1,267.60       | 150.70           |
| 1,268.00       | 161.57           |
| 1,268.40       | 172.75           |
| 1,268.80       | 184.27           |
| 1,269.20       | 196.13           |
| 1,269.60       | 208.33           |
| 1,270.00       | 220.87           |
| 1,270.40       | 233.77           |
| 1,270.80       | 247.02           |
| 1,271.20       | 260.63           |
| 1,271.60       | 274.61           |
| 1,272.00       | 288.95           |
| 1,272.40       | 303.68           |
| 1,272.80       | 318.78           |
| 1,273.20       | 334.27           |
| 1,273.60       | 350.15           |
| 1,274.00       | 366.42           |

| Elevation (ft) | Volume (Acre-ft) |
|----------------|------------------|
| 1,274.40       | 383.09           |
| 1,274.80       | 400.17           |
| 1,275.20       | 417.66           |
| 1,275.60       | 435.56           |
| 1,276.00       | 453.88           |
| 1,276.40       | 472.62           |
| 1,276.80       | 491.79           |
| 1,277.20       | 511.40           |
| 1,277.60       | 531.44           |
| 1,278.00       | 551.93           |
| 1,278.40       | 572.86           |
| 1,278.80       | 594.25           |
| 1,279.20       | 616.09           |
| 1,279.60       | 638.40           |
| 1,280.00       | 661.18           |
| 1,280.83       | 685.85           |
| 1,281.60       | 713.28           |
| 1,282.39       | 742.58           |
| 1,283.19       | 772.93           |
| 1,284.00       | 804.74           |
| 1,284.82       | 837.71           |
| 1,285.65       | 872.23           |
| 1,286.49       | 907.89           |
| 1,287.34       | 944.58           |
| 1,289.04       | 1,021.15         |
| 1,290.60       | 1,094.26         |
| 1,292.10       | 1,167.52         |
| 1,293.61       | 1,244.65         |
| 1,295.01       | 1319.24          |
| 1,296.44       | 1,398.50         |
| 1,297.84       | 1,479.73         |
| 1,299.28       | 1,566.68         |
| 1,300.75       | 1,658.89         |
| 1,302.25       | 1,756.54         |
| 1,303.81       | 1,860.78         |
| 1,305.43       | 1,972.16         |
| Elevation (ft) | Volume (Acre-ft) |

|          |          |
|----------|----------|
| 1,307.19 | 2,096.00 |
| 1,308.94 | 2,222.23 |
| 1,310.76 | 2,355.77 |
| 1,312.55 | 2,490.39 |
| 1,314.34 | 2,627.39 |
| 1,316.08 | 2,764.12 |
| 1,317.87 | 2,906.40 |
| 1,319.66 | 3,052.27 |
| 1,321.44 | 3,199.95 |
| 1,323.21 | 3,349.16 |
| 1,324.92 | 3,496.27 |
| 1,326.51 | 3,635.25 |
| 1,328.23 | 3,788.83 |
| 1,329.69 | 3,922.35 |
| 1,331.34 | 4,075.13 |
| 1,332.00 | 4,136.09 |
| 1,333.00 | 4,228.77 |
| 1,335.00 | 4,344.62 |
| 1,337.50 | 4,490.22 |

**ATTACHMENT D**

**BREACH PARAMETER CALCULATION**

|                  |                    |              |           |
|------------------|--------------------|--------------|-----------|
| Dam Name:        | Petit Lake         | Prepared by: | SS        |
| Location:        | Pickens County, GA | Date:        | 8/17/2022 |
| Breach Scenario: | Sunny Day          |              |           |

|  |             |
|--|-------------|
| Height of Dam (ft):  | 108.7       |
| Breach Bottom Elevation:                                       | 1538.3      |
| Height of water above breach bottom (ft):                      | 108.7       |
| Reservoir Storage Volume at Failure (acre-feet):               | 5635.0      |
| Reservoir Surface Area at Failure (acres):                     | 148.80      |
| Failure Scenario:  | Overtopping |
| Discharge through spillways at failure (Q <sub>o</sub> , cfs): | 0           |

### Breach Parameters

#### Froelich (2008)

|                           |              |                        |                 |
|---------------------------|--------------|------------------------|-----------------|
| Avg. Breach Width (ft):   | <u>326.1</u> | Breach Side Slopes:    | <u>1.0</u> H:1V |
| Breach Bottom Width (ft): | <u>217.4</u> | K <sub>o</sub> Factor: | <u>1.3</u>      |
| Time of failure (hrs):    | <u>0.45</u>  |                        |                 |

#### Froelich (1995)

|                           |              |                        |                 |
|---------------------------|--------------|------------------------|-----------------|
| Avg. Breach Width (ft):   | <u>326.1</u> | Breach Side Slopes:    | <u>1.4</u> H:1V |
| Breach Bottom Width (ft): | <u>173.9</u> | K <sub>o</sub> Factor: | <u>1.0</u>      |
| Time of failure (hrs):    | <u>0.48</u>  |                        |                 |

#### MacDonald & Langridge-Monopolis (1984) [For Piping Scenario Only when Storage Volume is less than 100 acre-feet]

|                           |                           |                     |                 |
|---------------------------|---------------------------|---------------------|-----------------|
| Avg. Breach Width (ft):   | <u>66.5</u>               | Breach Side Slopes: | <u>0.5</u> H:1V |
| Breach Bottom Width (ft): | <u>12.2</u>               | Upstream Slopes:    | <u>2.5</u> H:1V |
| Time of failure (hrs):    | <u>1.03</u>               | Downstream Slopes:  | <u>3.5</u> H:1V |
|                           | Storage exceeds 100 ac-ft | Crest Width (ft):   | <u>35</u>       |

#### VALUES USED FOR ANALYSIS (To be Entered by Engineer)

|                           |              |                               |               |
|---------------------------|--------------|-------------------------------|---------------|
| Avg. Breach Width (ft):   | <u>326.1</u> | Breach Side Slopes:           | <u>1</u> H:1V |
| Breach Bottom Width (ft): | <u>217.4</u> | (based on on selected values) |               |
| Time of failure (hrs):    | <u>0.45</u>  |                               |               |



Check for: Time of Failure too long



Check for: Time of Failure less than recommended minimum value

#### Notes:

- The average breach width cannot be wider than The width of The stream valley at The particular elevation.
- The check for time of failures are based on minimum reasonable value (based on MDE experience) and the maximum reasonable values based on expected erosion rate (Von Thun & Gillette (1990)).

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

|                  |                    |              |           |
|------------------|--------------------|--------------|-----------|
| Dam Name:        | Petit Lake         | Prepared by: | SS        |
| Location:        | Pickens County, GA | Date:        | 8/17/2022 |
| Breach Scenario: | Sunny Day          |              |           |



## Peak Breach Discharge

### National Weather Service Simple Dam Break Equation

|  |       |
|--|-------|
| Avg. Breach Width (ft) from previous sheet:            | 326.1 |
| Time of failure (hrs) from previous sheet:             | 0.45  |
| Height of water above breach bottom (ft):              | 108.7 |
| Reservoir Surface Area at Failure (acres):             | 148.8 |
| Discharge through spillways at failure ( $Q_o$ , cfs): | 0     |

$$Q_b = Q_o + 3.1B_r(C/(T_f + C/\sqrt{H}))^3$$

$Q_b$  = Peak breach discharge plus discharge through spillways (cfs)

$Q_o$  = Discharge through principal and emergency spillways with water surface at failure level

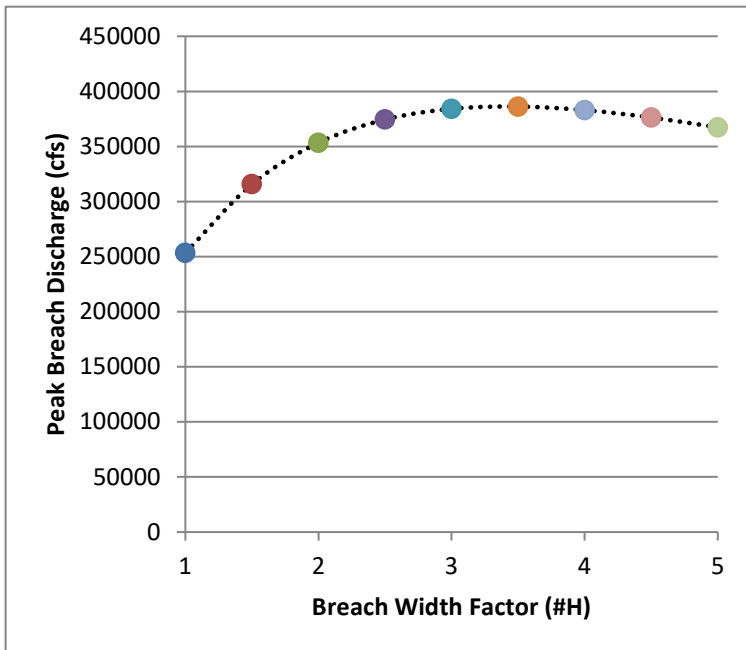
$B_r$  = Avg. Breach Width (ft), typically 1 to 5 times height of dam

$A_s$  = Reservoir Surface Area at with water surface at failure level (acres)

H = Height of water above breach bottom (ft)

$T_f$  = Time to failure (hrs)

$$C = 23.4 * A_s / B_r$$



| Factor | Breach Width (feet) | C     | $Q_b$ (cfs) |
|--------|---------------------|-------|-------------|
| [H]    | 108.7               | 32.03 | 253426      |
| [1.5H] | 163.05              | 21.35 | 315695      |
| [2H]   | 217.4               | 16.02 | 353377      |
| [2.5H] | 271.75              | 12.81 | 374425      |
| [3H]   | 326.1               | 10.68 | 384161      |
| [3.5H] | 380.45              | 9.15  | 386192      |
| [4.0H] | 434.8               | 8.01  | 382998      |
| [4.5H] | 489.15              | 7.12  | 376297      |
| [5.0H] | 543.5               | 6.41  | 367293      |

**Peak Breach Discharge: 386192.1 cfs**

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

Dam Name: Sconti Lake  
 Location: Pickens County, GA  
 Breach Scenario: Sunny Day

Prepared by: RT  
 Date: 4/19/2021

|  |             |
|--|-------------|
| Height of Dam (ft):  | 40          |
| Breach Bottom Elevation:                                       | 1430        |
| Height of water above breach bottom (ft):                      | 42          |
| Reservoir Storage Volume at Failure (acre-feet):               | 281.4       |
| Reservoir Surface Area at Failure (acres):                     | 140.70      |
| Failure Scenario:  | Overtopping |
| Discharge through spillways at failure (Q <sub>o</sub> , cfs): | 0           |

**Breach Parameters**

**Froelich (2008)**

|                           |              |                        |                 |
|---------------------------|--------------|------------------------|-----------------|
| Avg. Breach Width (ft):   | <u>120.0</u> | Breach Side Slopes:    | <u>1.0</u> H:1V |
| Breach Bottom Width (ft): | <u>80.0</u>  | K <sub>o</sub> Factor: | <u>1.3</u>      |
| Time of failure (hrs):    | <u>0.26</u>  |                        |                 |

**Froelich (1995)**

|                           |             |                        |                 |
|---------------------------|-------------|------------------------|-----------------|
| Avg. Breach Width (ft):   | <u>99.1</u> | Breach Side Slopes:    | <u>1.4</u> H:1V |
| Breach Bottom Width (ft): | <u>43.1</u> | K <sub>o</sub> Factor: | <u>1.0</u>      |
| Time of failure (hrs):    | <u>0.28</u> |                        |                 |

**MacDonald & Langridge-Monopolis (1984)** [For Piping Scenario Only when Storage Volume is less than 100 acre-feet]

|                           |                           |                     |                 |
|---------------------------|---------------------------|---------------------|-----------------|
| Avg. Breach Width (ft):   | <u>20.2</u>               | Breach Side Slopes: | <u>0.5</u> H:1V |
| Breach Bottom Width (ft): | <u>0.2</u>                | Upstream Slopes:    | <u>3.0</u> H:1V |
| Time of failure (hrs):    | <u>0.34</u>               | Downstream Slopes:  | <u>3.0</u> H:1V |
|                           | Storage exceeds 100 ac-ft | Crest Width (ft):   | <u>35</u>       |

**VALUES USED FOR ANALYSIS (To be Entered by Engineer)**

|                           |              |                               |               |
|---------------------------|--------------|-------------------------------|---------------|
| Avg. Breach Width (ft):   | <u>120.0</u> | Breach Side Slopes:           | <u>1</u> H:1V |
| Breach Bottom Width (ft): | <u>80.0</u>  | (based on on selected values) |               |
| Time of failure (hrs):    | <u>0.27</u>  |                               |               |

- Check for: Time of Failure too long
- Check for: Time of Failure less than recommended minimum value

Notes:

- The average breach width cannot be wider than The width of The stream valley at The particular elevation.
- The check for time of failures are based on minimum reasonable value (based on MDE experience) and the maximum reasonable valuesbased on expected erosion rate (Von Thun & Gillette (1990)).

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

Dam Name: Sconti Lake  
 Location: Pickens County, GA  
 Breach Scenario: Sunny Day

Prepared by: RT  
 Date: 4/19/2021

## Peak Breach Discharge

### National Weather Service Simple Dam Break Equation

|  |       |
|--|-------|
| Avg. Breach Width (ft) from previous sheet:            | 120   |
| Time of failure (hrs) from previous sheet:             | 0.27  |
| Height of water above breach bottom (ft):              | 42    |
| Reservoir Surface Area at Failure (acres):             | 140.7 |
| Discharge through spillways at failure ( $Q_o$ , cfs): | 0     |

$$Q_b = Q_o + 3.1B_r(C/(T_f + C/\sqrt{H}))^3$$

$Q_b$  = Peak breach discharge plus discharge through spillways (cfs)

$Q_o$  = Discharge through principal and emergency spillways with water surface at failure level

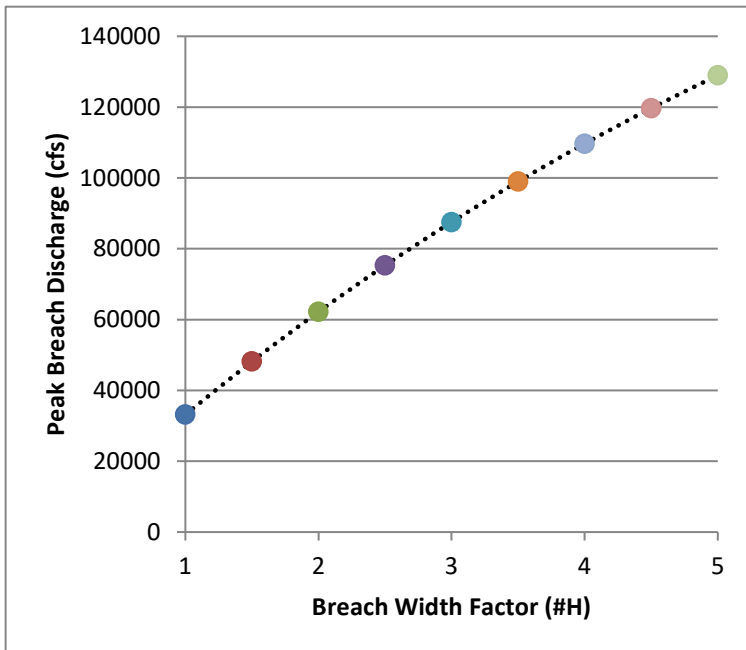
$B_r$  = Avg. Breach Width (ft), typically 1 to 5 times height of dam

$A_s$  = Reservoir Surface Area at with water surface at failure level (acres)

H = Height of water above breach bottom (ft)

$T_f$  = Time to failure (hrs)

$$C = 23.4 * A_s / B_r$$



| Factor | Breach Width (feet) | C     | $Q_b$ (cfs) |
|--------|---------------------|-------|-------------|
| [H]    | 42                  | 78.39 | 33168       |
| [1.5H] | 63                  | 52.26 | 48158       |
| [2H]   | 84                  | 39.20 | 62174       |
| [2.5H] | 105                 | 31.36 | 75279       |
| [3H]   | 126                 | 26.13 | 87530       |
| [3.5H] | 147                 | 22.40 | 98979       |
| [4.0H] | 168                 | 19.60 | 109678      |
| [4.5H] | 189                 | 17.42 | 119671      |
| [5.0H] | 210                 | 15.68 | 129003      |

**Peak Breach Discharge: 129002.8 cfs**

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

Dam Name: Cox Lake  
 Location: Pickens County, GA  
 Breach Scenario: Sunny Day

Prepared by: RT  
 Date: 4/19/2021

|  |             |
|--|-------------|
| Height of Dam (ft):                                    | 97          |
| Breach Bottom Elevation:                               | 1235.5      |
| Height of water above breach bottom (ft):              | 99          |
| Reservoir Storage Volume at Failure (acre-feet):       | 4490.2      |
| Reservoir Surface Area at Failure (acres):             | 222.91      |
| Failure Scenario:                                      | Overtopping |
| Discharge through spillways at failure ( $Q_o$ , cfs): | 0           |

**Breach Parameters**


**Froelich (2008)**

|                           |              |                     |            |      |
|---------------------------|--------------|---------------------|------------|------|
| Avg. Breach Width (ft):   | <u>291.0</u> | Breach Side Slopes: | <u>1.0</u> | H:1V |
| Breach Bottom Width (ft): | <u>194.0</u> | $K_o$ Factor:       | <u>1.3</u> |      |
| Time of failure (hrs):    | <u>0.44</u>  |                     |            |      |

**Froelich (1995)**



|                           |              |                     |            |      |
|---------------------------|--------------|---------------------|------------|------|
| Avg. Breach Width (ft):   | <u>245.3</u> | Breach Side Slopes: | <u>1.4</u> | H:1V |
| Breach Bottom Width (ft): | <u>109.5</u> | $K_o$ Factor:       | <u>1.0</u> |      |
| Time of failure (hrs):    | <u>0.47</u>  |                     |            |      |

**MacDonald & Langridge-Monopolis (1984)** [For Piping Scenario Only when Storage Volume is less than 100 acre-feet]

|                           |   |                     |            |      |
|---------------------------|---|---------------------|------------|------|
| Avg. Breach Width (ft):   | <u>64.5</u>   | Breach Side Slopes: | <u>0.5</u> | H:1V |
| Breach Bottom Width (ft): | <u>16.0</u>   | Upstream Slopes:    | <u>3.0</u> | H:1V |
| Time of failure (hrs):    | <u>0.94</u>   | Downstream Slopes:  | <u>3.0</u> | H:1V |
|                           |  Storage exceeds 100 ac-ft | Crest Width (ft):   | <u>35</u>  |      |

**VALUES USED FOR ANALYSIS (To be Entered by Engineer)**

|                           |              |                               |          |      |
|---------------------------|--------------|-------------------------------|----------|------|
| Avg. Breach Width (ft):   | <u>291.0</u> | Breach Side Slopes:           | <u>1</u> | H:1V |
| Breach Bottom Width (ft): | <u>194.0</u> | (based on on selected values) |          |      |
| Time of failure (hrs):    | <u>0.45</u>  |                               |          |      |

-  Check for: Time of Failure too long
-  Check for: Time of Failure less than recommended minimum value

Notes:

- The average breach width cannot be wider than The width of The stream valley at The particular elevation.
- The check for time of failures are based on minimum reasonable value (based on MDE experience) and the maximum reasonable values based on expected erosion rate (Von Thun & Gillette (1990)).

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

Dam Name: Cox Lake  
 Location: Pickens County, GA  
 Breach Scenario: Sunny Day

Prepared by: RT  
 Date: 4/19/2021

## Peak Breach Discharge

### National Weather Service Simple Dam Break Equation

|  |        |
|--|--------|
| Avg. Breach Width (ft) from previous sheet:            | 291    |
| Time of failure (hrs) from previous sheet:             | 0.45   |
| Height of water above breach bottom (ft):              | 99     |
| Reservoir Surface Area at Failure (acres):             | 222.91 |
| Discharge through spillways at failure ( $Q_o$ , cfs): | 0      |

$$Q_b = Q_o + 3.1B_r(C/(T_f + C/\sqrt{H}))^3$$

$Q_b$  = Peak breach discharge plus discharge through spillways (cfs)

$Q_o$  = Discharge through principal and emergency spillways with water surface at failure level

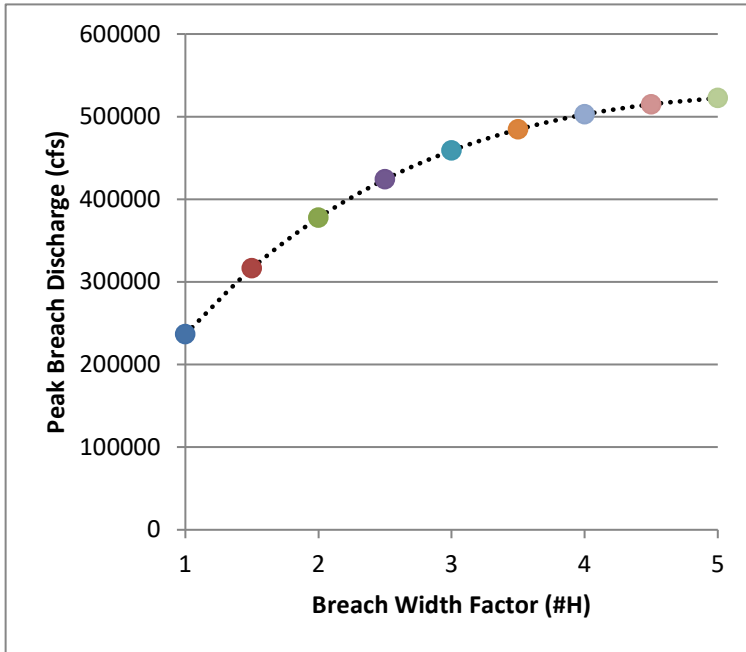
$B_r$  = Avg. Breach Width (ft), typically 1 to 5 times height of dam

$A_s$  = Reservoir Surface Area at with water surface at failure level (acres)

H = Height of water above breach bottom (ft)

$T_f$  = Time to failure (hrs)

$C = 23.4 * A_s / B_r$



| Factor | Breach Width (feet) | C     | $Q_b$ (cfs) |
|--------|---------------------|-------|-------------|
| [H]    | 99                  | 52.69 | 236692      |
| [1.5H] | 148.5               | 35.13 | 316392      |
| [2H]   | 198                 | 26.34 | 377542      |
| [2.5H] | 247.5               | 21.08 | 424030      |
| [3H]   | 297                 | 17.56 | 458881      |
| [3.5H] | 346.5               | 15.05 | 484466      |
| [4.0H] | 396                 | 13.17 | 502656      |
| [4.5H] | 445.5               | 11.71 | 514931      |
| [5.0H] | 495                 | 10.54 | 522473      |

**Peak Breach Discharge 522472.8 cfs**

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

Dam Name: Cove Road  
 Location: Pickens County, GA  
 Breach Scenario: Sunny Day

Prepared by: EC  
 Date: 10/24/2022

|  |             |
|--|-------------|
| Height of Dam (ft):  | 44          |
| Breach Bottom Elevation:                                       | 1291        |
| Height of water above breach bottom (ft):                      | 46          |
| Reservoir Storage Volume at Failure (acre-feet):               | 206.8       |
| Reservoir Surface Area at Failure (acres):                     | 10.37       |
| Failure Scenario:  | Overtopping |
| Discharge through spillways at failure (Q <sub>o</sub> , cfs): | 0           |

### Breach Parameters

#### Froelich (2008)


|                           |              |                        |                 |
|---------------------------|--------------|------------------------|-----------------|
| Avg. Breach Width (ft):   | <u>132.0</u> | Breach Side Slopes:    | <u>1.0</u> H:1V |
| Breach Bottom Width (ft): | <u>88.0</u>  | K <sub>o</sub> Factor: | <u>1.3</u>      |
| Time of failure (hrs):    | <u>0.20</u>  |                        |                 |

#### Froelich (1995)

|                           |              |                        |                 |
|---------------------------|--------------|------------------------|-----------------|
| Avg. Breach Width (ft):   | <u>132.0</u> | Breach Side Slopes:    | <u>1.4</u> H:1V |
| Breach Bottom Width (ft): | <u>70.4</u>  | K <sub>o</sub> Factor: | <u>1.0</u>      |
| Time of failure (hrs):    | <u>0.22</u>  |                        |                 |

#### MacDonald & Langridge-Monopolis (1984)

[For Piping Scenario Only when Storage Volume is less than 100 acre-feet]

|                           |   |                     |                 |
|---------------------------|---|---------------------|-----------------|
| Avg. Breach Width (ft):   | <u>14.4</u>   | Breach Side Slopes: | <u>0.5</u> H:1V |
| Breach Bottom Width (ft): | <u>-7.6</u>   | Upstream Slopes:    | <u>3.0</u> H:1V |
| Time of failure (hrs):    | <u>0.32</u>   | Downstream Slopes:  | <u>3.0</u> H:1V |
|                           |  Storage exceeds 100 ac-ft | Crest Width (ft):   | <u>35</u>       |

#### VALUES USED FOR ANALYSIS (To be Entered by Engineer)

|                           |              |                            |               |
|---------------------------|--------------|----------------------------|---------------|
| Avg. Breach Width (ft):   | <u>132.0</u> | Breach Side Slopes:        | <u>1</u> H:1V |
| Breach Bottom Width (ft): | <u>88.0</u>  | (based on selected values) |               |
| Time of failure (hrs):    | <u>0.20</u>  |                            |               |



Check for: Time of Failure too long



Check for: Time of Failure less than recommended minimum value

#### Notes:

- The average breach width cannot be wider than The width of The stream valley at The particular elevation.
- The check for time of failures are based on minimum reasonable value (based on MDE experience) and the maximum reasonable valuesbased on expected erosion rate (Von Thun & Gillette (1990)).

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

Dam Name: Cove Road Prepared by: EC  
 Location: Pickens County, GA Date: 10/24/2022  
 Breach Scenario: Sunny Day

**Peak Breach Discharge**

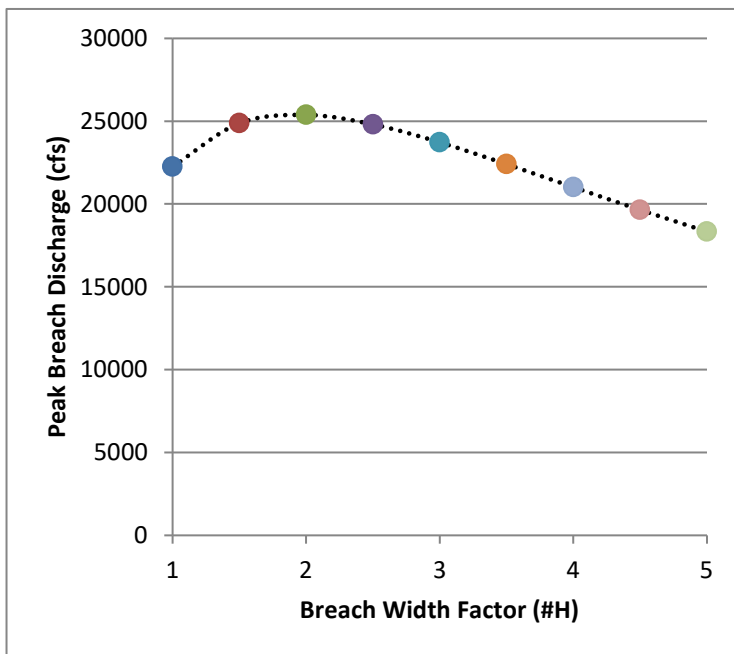
**National Weather Service Simple Dam Break Equation**

|  |             |
|--|-------------|
| Avg. Breach Width (ft) from previous sheet:                    | 132         |
| Time of failure (hrs) from previous sheet:                     | 0.201862415 |
| Height of water above breach bottom (ft):                      | 46          |
| Reservoir Surface Area at Failure (acres):                     | 10.37       |
| Discharge through spillways at failure (Q <sub>o</sub> , cfs): | 0           |

$$Q_b = Q_o + 3.1B_r(C/(T_f + C/\sqrt{H}))^3$$

- Q<sub>b</sub> = Peak breach discharge plus discharge through spillways (cfs)
- Q<sub>o</sub> = Discharge through principal and emergency spillways with water surface at failure level
- B<sub>r</sub> = Avg. Breach Width (ft), typically 1 to 5 times height of dam
- A<sub>s</sub> = Reservoir Surface Area at with water surface at failure level (acres)
- H = Height of water above breach bottom (ft)
- T<sub>f</sub> = Time to failure (hrs)
- C = 23.4\*A<sub>s</sub>/B<sub>r</sub>

| Factor | Breach Width (feet) | C    | Q <sub>b</sub> (cfs) |
|--------|---------------------|------|----------------------|
| [H]    | 46                  | 5.28 | 22265                |
| [1.5H] | 69                  | 3.52 | 24886                |
| [2H]   | 92                  | 2.64 | 25383                |
| [2.5H] | 115                 | 2.11 | 24812                |
| [3H]   | 138                 | 1.76 | 23721                |
| [3.5H] | 161                 | 1.51 | 22404                |
| [4.0H] | 184                 | 1.32 | 21019                |
| [4.5H] | 207                 | 1.17 | 19649                |
| [5.0H] | 230                 | 1.06 | 18338                |



**Peak Breach Discharge: 25383.5 cfs**

Note: This spreadsheet is provided for the convenience of the engineering community in the State of Maryland. All results should be verified as accurate by the user.

**ATTACHMENT E**  
**SETTINGS AND TOLERANCES**



HEC-RAS Unsteady Computation Options and Tolerances

General | **2D Flow Options** | 1D/2D Options | Advanced Time Step Control | 1D Mixed Flow Options

**1D Unsteady Flow Options**

Theta [implicit weighting factor] (0.6-1.0):

Theta for warm up [implicit weighting factor] (0.6-1.0):

Water surface calculation tolerance [max=0.2](ft):

Storage Area elevation tolerance [max=0.2](ft):

Flow calculation tolerance [optional] (cfs):

Max error in water surface solution (Abort Tolerance)(ft):

Maximum number of iterations (0-40):

Maximum iterations without improvement (0-40):

**1D/2D Unsteady Flow Options**

Number of warm up time steps (0 - 100,000):

Time step during warm up period (hrs):

Minimum time step for time slicing (hrs):

Maximum number of time slices:

Lateral Structure flow stability factor (1.0-3.0):

Inline Structure flow stability factor (1.0-3.0):

Weir flow submergence decay exponent (1.0-3.0):

Gate flow submergence decay exponent (1.0-3.0):

DSS Messaging Level (1 to 10, Default = 4)

**Geometry Preprocessor Options**

Family of Rating Curves for Internal Boundaries

Use existing internal boundary tables when possible.

Recompute at all internal boundaries

**1D Numerical Solution**

Finite Difference (classic HEC-RAS methodology)

Finite Difference Matrix Solver

Skyline/Gaussian (Default: faster for dendritic systems)

Pardiso (Optional: may be faster for large interconnected systems)

Finite Volume (new approach)

Number of cores to use with Pardiso solver:

OK Cancel Defaults ...

HEC-RAS Unsteady Computation Options and Tolerances

General | **2D Flow Options** | 1D/2D Options | Advanced Time Step Control | 1D Mixed Flow Options

Use Coriolis Effects (only when using the momentum equation)

Number of cores to use in 2D computations:

| Parameter                                       | (Default)                | Middle                   | North                    | South                    |
|---|--------------------------|--------------------------|--------------------------|--------------------------|
| 1 Theta (0.6-1.0):                              | 1                        | 1                        | 1                        | 1                        |
| 2 Theta Warmup (0.6-1.0):                       | 1                        | 1                        | 1                        | 1                        |
| 3 Water Surface Tolerance [max=0.2](ft)         | 0.01                     | 0.01                     | 0.01                     | 0.01                     |
| 4 Volume Tolerance (ft)                         | 0.01                     | 0.01                     | 0.01                     | 0.01                     |
| 5 Maximum Iterations                            | 20                       | 20                       | 20                       | 20                       |
| 6 Equation Set                                  | Diffusion Wave           | Full Momentum            | Full Momentum            | Full Momentum            |
| 7 Initial Conditions Time (hrs)                 |                          |                          |                          |                          |
| 8 Initial Conditions Ramp Up Fraction (0-1)     | 0.1                      | 0.1                      | 0.1                      | 0.1                      |
| 9 Number of Time Slices (Integer Value)         | 1                        | 1                        | 1                        | 1                        |
| 10 Eddy Viscosity Transverse Mixing Coefficient |                          |                          |                          |                          |
| 11 Boundary Condition Volume Check              | <input type="checkbox"/> | <input type="checkbox"/> | <input type="checkbox"/> | <input type="checkbox"/> |
| 12 Latitude for Coriolis (-90 to 90)            |                          |                          |                          |                          |

OK Cancel Defaults ...

HEC-RAS Unsteady Computation Options and Tolerances

General | 2D Flow Options | 1D/2D Options | Advanced Time Step Control | 1D Mixed Flow Options

Maximum iterations between 1D and 2D (0=off, 1 to 20):

Water surface tolerance (ft):

Flow Tolerance (%):

Minimum flow tolerance (cfs):

OK Cancel Defaults ...

HEC-RAS Unsteady Computation Options and Tolerances

General | 2D Flow Options | 1D/2D Options | Advanced Time Step Control | 1D Mixed Flow Options

Mixed Flow Regime (see menu: "Options/Mixed Flow Options ...")

Exponent for Froude number reduction factor  $m$  ( $m > 0$ ):

Froude number threshold for eliminating acceleration terms:

**Local Partial Inertia Filter**

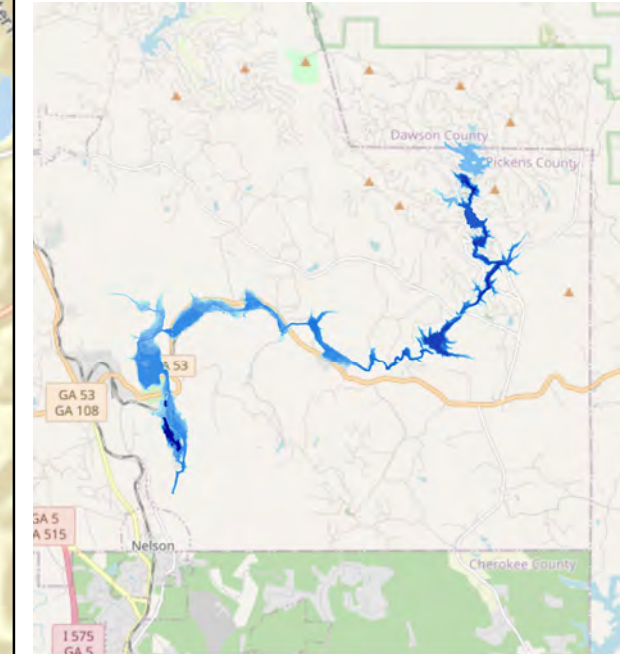
The graph plots the Local Partial Inertia (LPI) Factor on the y-axis (ranging from 0.0 to 1.0) against the Froude Number on the x-axis (ranging from 0 to 1). Multiple curves are shown for different values of the exponent  $m$ :  $m=1$  (blue),  $m=2$  (magenta),  $m=4$  (yellow),  $m=8$  (cyan),  $m=16$  (purple),  $m=32$  (red),  $m=64$  (teal), and  $m=128$  (dark blue). As  $m$  increases, the curves become steeper and shift towards the right, indicating that the LPI factor remains closer to 1.0 for a wider range of Froude numbers before dropping to 0.0.

OK Cancel Defaults ...

**ATTACHMENT F**  
**INUNDATION MAP**

# LAKE PETIT DAM FAILURE

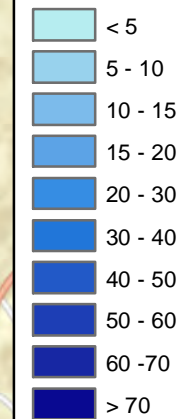
## LOCATION MAP



## LEGEND

Inundated Structures (Total Count: 142)

### Maximum Depth (feet)

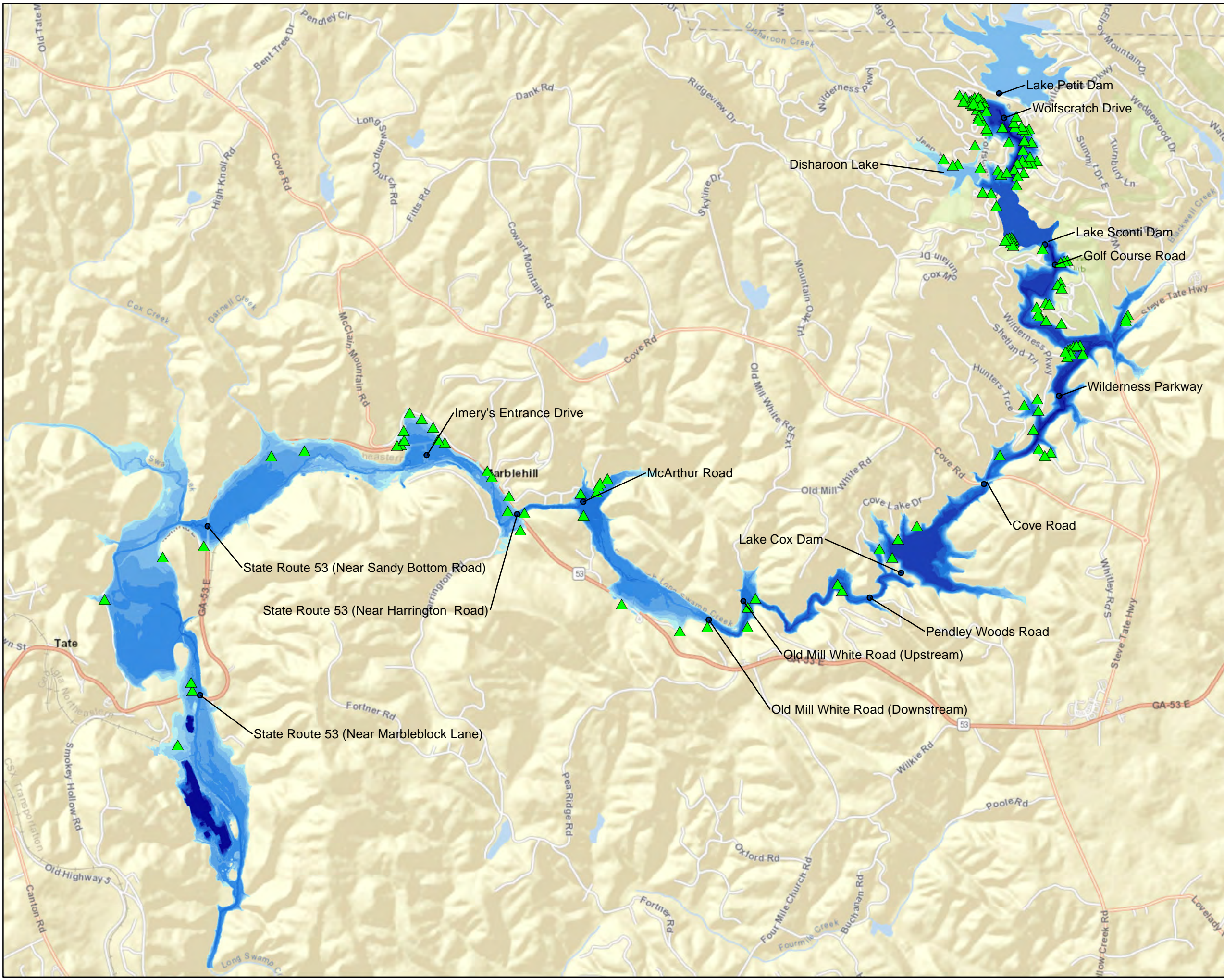
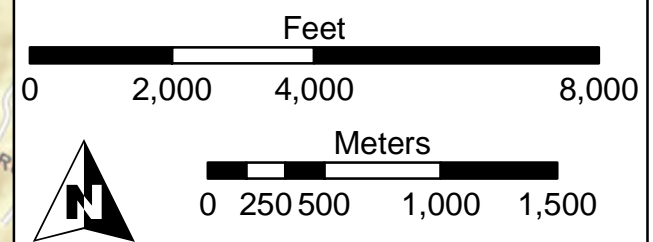


NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.

The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.

**Geosyntec**  
consultants

07-Oct-2022

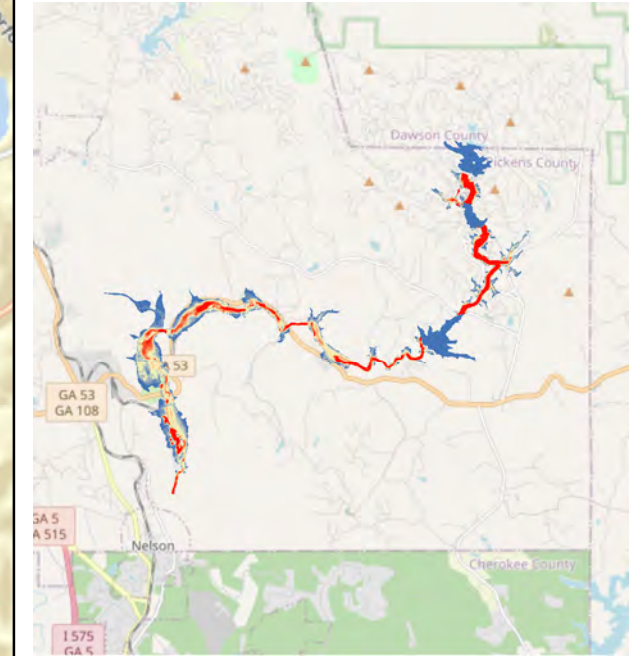


**ATTACHMENT G**

**VELOCITY MAP**

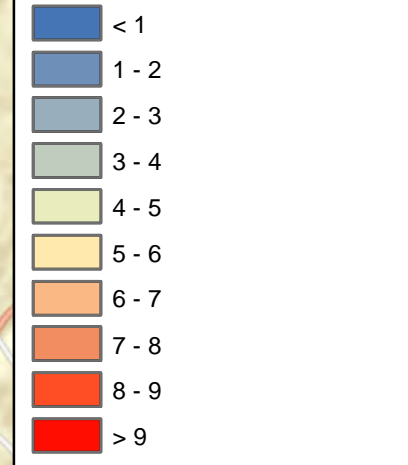
# LAKE PETIT DAM FAILURE

## LOCATION MAP



## LEGEND

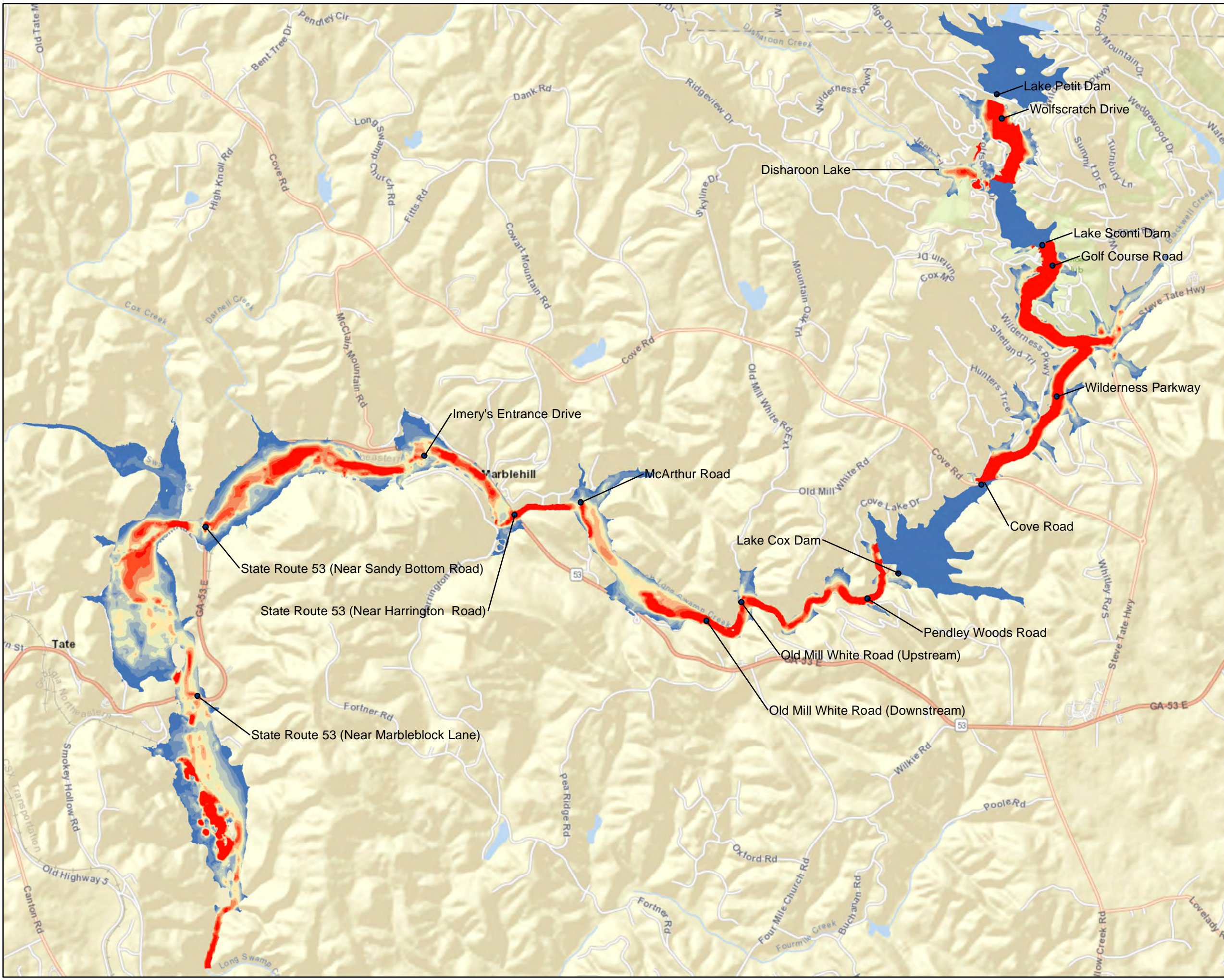
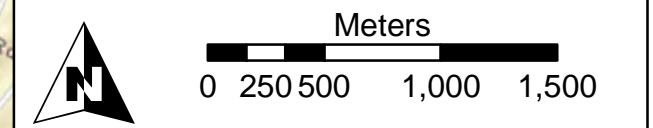
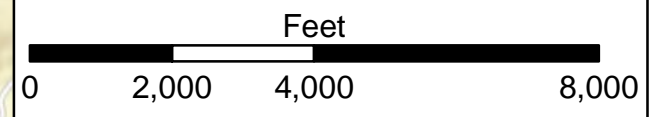
Maximum Velocity (ft/s)



The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.

**Geosyntec**  
consultants

07-Oct-2022



**ATTACHMENT H**  
**SUMMARY OF WARNINGS AND NOTES**

## Model Errors

1. Extrapolated beyond Storage VOL vs EL curve at Storage Area Sconti Lake
2. The maximum storage area wsel error was 0.035 (Cox Lake at 12FEB2020 03:21:16)

## Snip from Plan Computation in HEC-RAS

\*\*\*\*\* Error! Extrapolated beyond Storage VOL vs EL curve \*\*\*\*\*

At Storage Area Sconti Lake  
**Writing Results to DSS**

**The maximum storage area wsel error was 0.035**  
Cox Lake at 12FEB2020 03:21:16



HEC-RAS - River Analysis System

Project File: C:\Office\_Projects\Lake Petit\2022(0927)\_Final Model\LakePetit.prj

Project Name: Lake Petit

Plan Name: 0927(2022)\_Lake Petit Plan

Short ID: Lake Petit 092722

Starting Time: 11Feb2020 2400

Ending Time: 12Feb2020 0600

```
#####
#                               #
#                               #
# 1D and 2D Unsteady Flow Module #
#                               #
#                               #
# HEC-RAS 5.0.7 March 2019      #
#                               #
# 27SEP22 at 17:49:08          #
#                               #
#####
```

Volume Accounting in Acre Feet

External Boundary Flux of Water

| US Inflow  | Lat Hydro | DS Outflow | SA Hydro | Groundwater | 2D Inflow | 2D Outflow |
|------------|-----------|------------|----------|-------------|-----------|------------|
| Diversions |           |            |          |             |           |            |
| *****      | *****     | *****      | *****    | *****       | *****     | *****      |
| *****      |           |            |          |             |           |            |
|            |           |            |          | 3980.       |           |            |

River Reaches, Storage Areas, and 2D Areas

| Start 1D Reach | Starting SA's | Starting 2D | Final 1D Reach | Final SA's | Final 2D Areas |
|----------------|---------------|-------------|----------------|------------|----------------|
| *****          | *****         | *****       | *****          | *****      | *****          |
| 6318.          |               |             | 18308.         |            |                |

Error Percent Error

\*\*\*\*\* \*\*\*\*\*  
15970. 252.8

Volume Accounting for 2D Flow Area in Acre Feet

| 2D Area | Starting Vol | Ending Vol | Cum Inflow | Cum Outflow | Error   | Percent Error |
|---------|--------------|------------|------------|-------------|---------|---------------|
| *****   | *****        | *****      | *****      | *****       | *****   | *****         |
| Middle  | 231.8        | 141168.    | 141804.    | 867.9       | 0.6148  |               |
| North   | 15.10        | 5637.      | 5844.      | 221.8       | 3.935   |               |
| South   | 18061.       | 22038.     | 3980.      | 2.513       | 0.01140 |               |

**ATTACHMENT I**  
**POTENTIAL HAZARD ADDRESSES**

**INUNDATED STRUCTURES  
ADDRESS**

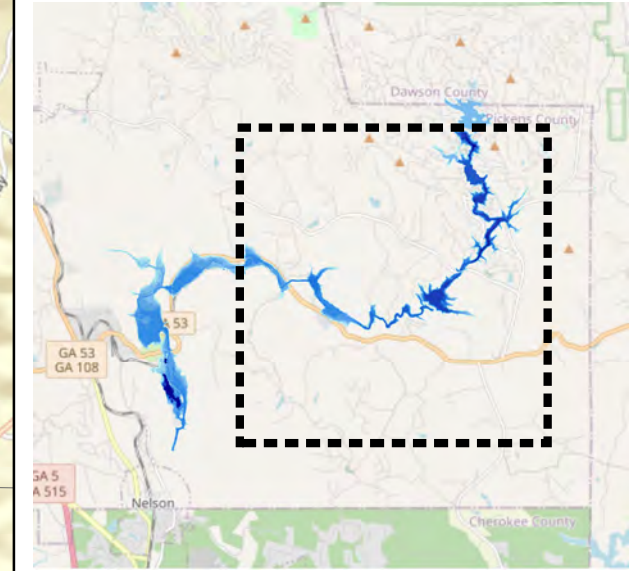
|                         |                             |
|-------------------------|-----------------------------|
| 1136 WOLFSCRATCH DR     | 9399 HIGHWAY 53 E           |
| 1136 WOLFSCRATCH DR     | 25 PULLTIGHT RD             |
| 1136 WOLFSCRATCH DR     | 9077 HIGHWAY 53 E           |
| 1136 WOLFSCRATCH DR     | 9037 HIGHWAY 53 E           |
| 48 SCONTI RDG           | 454 OLD HARBIN RD           |
| 86 SCONTI RDG           | 356 OLD HARBIN RD           |
| 40 TREETOP LN           | 8839 HIGHWAY 53 E           |
| 104 TREETOPPER LN       | 8817 HIGHWAY 53 E           |
| 92 TREETOPPER LN        | 280 OLD HARBIN RD           |
| 52 TREETOPPER LN        | 154 OLD HARBIN RD           |
| 309 BUCKSKULL HOLLOW DR | 34 LIMESTONE LN             |
| 301 BUCKSKULL HOLLOW DR | 9 HARRINGTON RD             |
| 293 BUCKSKULL HOLLOW DR | 9502 HIGHWAY 53 E           |
| 229 BUCKSKULL HOLLOW DR | 1511 SANDY BOTTOM RD        |
| 41 BUCKSKULL HOLLOW DR  | 1150 SANDY BOTTOM RD E      |
| 75 BUCKSKULL HOLLOW DR  | 200 GEORGIA MARBLE LN       |
| 11 BUCKSKULL BROW       | 6361 HIGHWAY 53 E           |
| 11 BUCKSKULL HOLLOW DR  | 66 MARBLEBLOCK DR           |
| 26 BUCKSKULL PT         | 220 SINTI TRL               |
| 28 BUCKSKULL PT         | 104 TWIN CREEKS DR          |
| 32 BUCKSKULL HOLLOW DR  | 22 BUCKSKULL PT             |
| 30 BUCKSKULL PT         | 57 BUCKSKULL HOLLOW DR      |
| 10 BUCKSKULL BROW       | 126 SINTI TRL               |
| 26 BUCKSKULL BROW       | 115 TWIN CREEKS DR          |
| 202 BUCKSKULL HOLLOW DR | 14 E BRANCH LN              |
| 242 BUCKSKULL HOLLOW DR | 11 ISUBA TRL                |
| 110 HIGHLAND CT         | 204 SINTI TRL               |
| 15 HIGHLAND CT          | 180 SINTI TRL               |
| 186 SCONTI KNOLL DR     | 461 WOLFSCRATCH DR          |
| 196 SCONTI KNOLL DR     | 50 WOLFSCRATCH VILLAGE CIR  |
| 200 SCONTI KNOLL DR     | 84 WOLFSCRATCH VILLAGE CIR  |
| 193 SCONTI KNOLL DR     | 100 WOLFSCRATCH VILLAGE CIR |
| 191 SCONTI KNOLL DR     | 800 WOLFSCRATCH DR          |
| 60 SCONTI PT            | 131 TWIN CREEKS DR          |
| 86 SCONTI PT            | 333 CHOCTAW PASS            |
| 17 LAUREL RIDGE WAY     | 315 CHOCTAW PASS            |
| 261 LAUREL RIDGE TRL    | 185 TWIN CREEKS DR          |
| 279 LAUREL RIDGE TRL    | 125 TROTTERS LN             |
| 327 LAUREL RIDGE TRL    | 226 WOLFSCRATCH VILLAGE CIR |

| <b>INUNDATED STRUCTURES<br/>ADDRESS</b> |                             |
|---|-----------------------------|
| 194 TWIN CREEKS DR                      | 35 TREETOP KNOLL DR         |
| 1944 WILDERNESS PKY                     | 151 TWIN CREEKS DR          |
| 1944 WILDERNESS PKY                     | 165 TWIN CREEKS DR          |
| 1944 WILDERNESS PKY                     | 8100 HIGHWAY 53 E           |
| 1944 WILDERNESS PKY                     | 112 TWIN CREEKS DR          |
| 1944 WILDERNESS PKY                     | 298 CLUBHOUSE DR            |
| 1944 WILDERNESS PKY                     | 111 SCONTI PT               |
| 1944 WILDERNESS PKY                     | 1138 SANDY BOTTOM RD        |
| 1944 WILDERNESS PKY                     | 2191 STEVE TATE RD          |
| 1944 WILDERNESS PKY                     | 2193 STEVE TATE RD          |
| 10 SHETLAND TRCE                        | 2195 STEVE TATE RD          |
| 42 OSI WAY                              | 84 HIGHLAND TRL             |
| 48 OSI WAY                              | 57 HIGHLAND CT              |
| 140 SINTI TRL                           | 38 HIGHLAND TRL             |
| 52 NASHOBA TRL                          | 244 WOLFSCRATCH DR          |
| 1543 OLD MILL WHITE RD                  | 400 CAMERON CT              |
| 30 JUSTICE WAY                          | 61 HUNTERS TRCE             |
| 189 TIMBER CREEK DR                     | 835 COVE LAKE DR            |
| 112 OVERLOOK CT                         | 833 COVE LAKE DR            |
| 391 COVE LAKE DR                        | 43 OSI WAY                  |
| 87 TROTTERS LN                          | 30 E BRANCH LN              |
| 100 HUNTERS TRCE                        | 101 TWIN CREEKS DR          |
| 270 BLACKWELL CREEK WAY                 | 250 SINTI TRL               |
| 295 BLACKWELL CREEK WAY                 | 350 WOLFSCRATCH VILLAGE CIR |
| 255 BLACKWELL CREEK WAY                 | 1125 WOLFSCRATCH DR         |
| 482 MARBLEHEAD LN                       | 1127 WOLFSCRATCH DR         |
| 427 MCARTHUR RD                         | 8200 HIGHWAY 53 E           |
| 393 MCARTHUR RD                         | 475 DELL MOORE RD           |
| 1985 OLD MILL WHITE RD                  | 95 TREETOP KNOLL DR         |
| 2192 OLD MILL WHITE RD                  | 1175 WOLFSCRATCH DR         |
| 10322 HIGHWAY 53 E                      | 80 E BRANCH RD              |
| 414 MARBLE HILL CHURCH RD               | 9679 HIGHWAY 53 E           |

Note: Property information was provided by the Pickens County EMA in September 2022 using the mapped inundation area plus an additional 100-foot horizontal buffer outside the extent of the mapped inundation zone.

**Figure 1 - Lake Petit Dam Evacuation Map Vicinity of Big Canoe**

**LOCATION MAP**



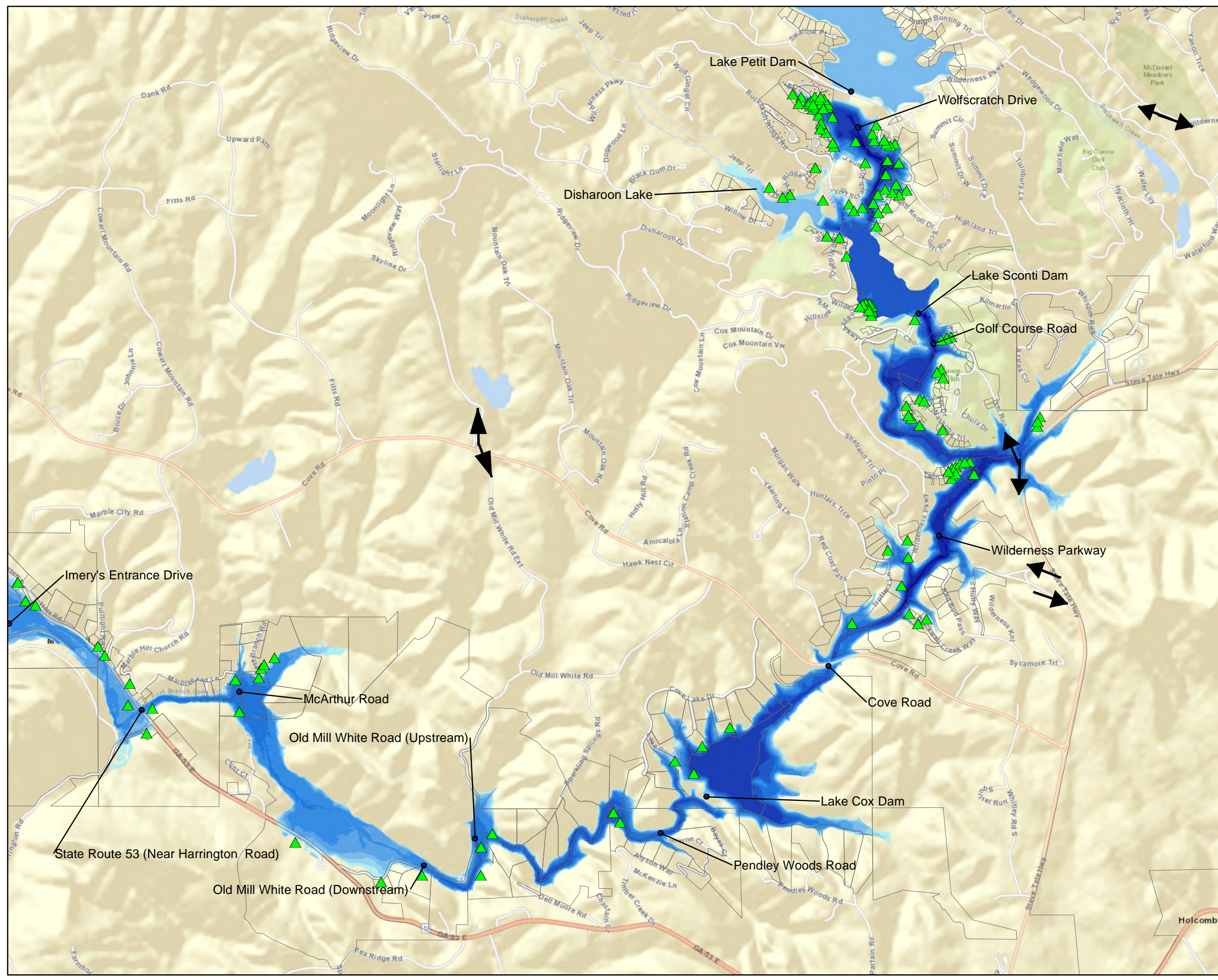
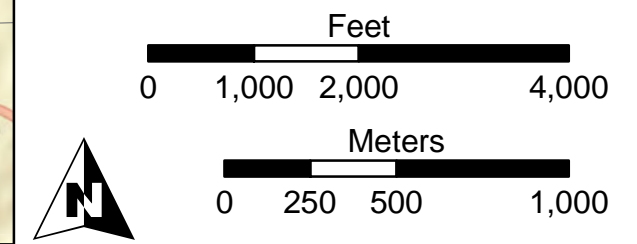
**LEGEND**

- Inundated Structures
  - Site Access
  - Inundated Parcels
- Maximum Depth (feet)**
- < 5
  - 5 - 10
  - 10 - 15
  - 15 - 20
  - 20 - 30
  - 30 - 40
  - 40 - 50
  - 50 - 60
  - 60 - 70
  - > 70

*NOTE: The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.*

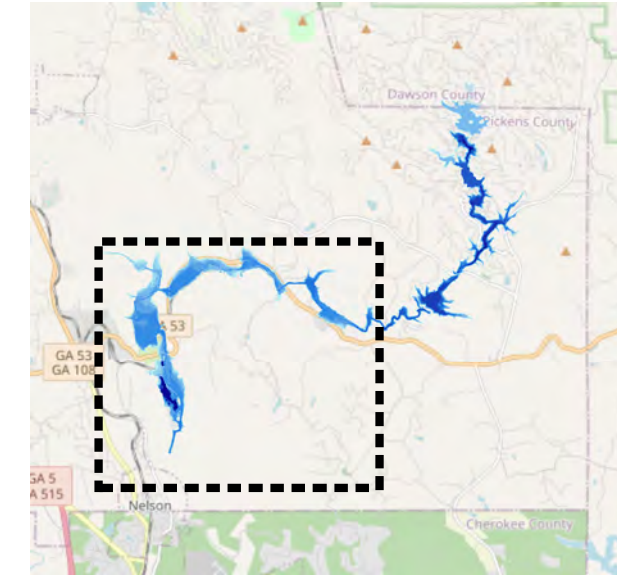
*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

**Geosyntec** consultants  
07-Oct-2022



**Figure 2 - Lake Petit Dam Evacuation Map  
Downstream of Big Canoe**

**LOCATION MAP**



**LEGEND**

- Inundated Structures
  - Site Access
  - Inundated Parcels
- Maximum Depth (feet)**
- < 5
  - 5 - 10
  - 10 - 15
  - 15 - 20
  - 20 - 30
  - 30 - 40
  - 40 - 50
  - 50 - 60
  - 60 - 70
  - > 70

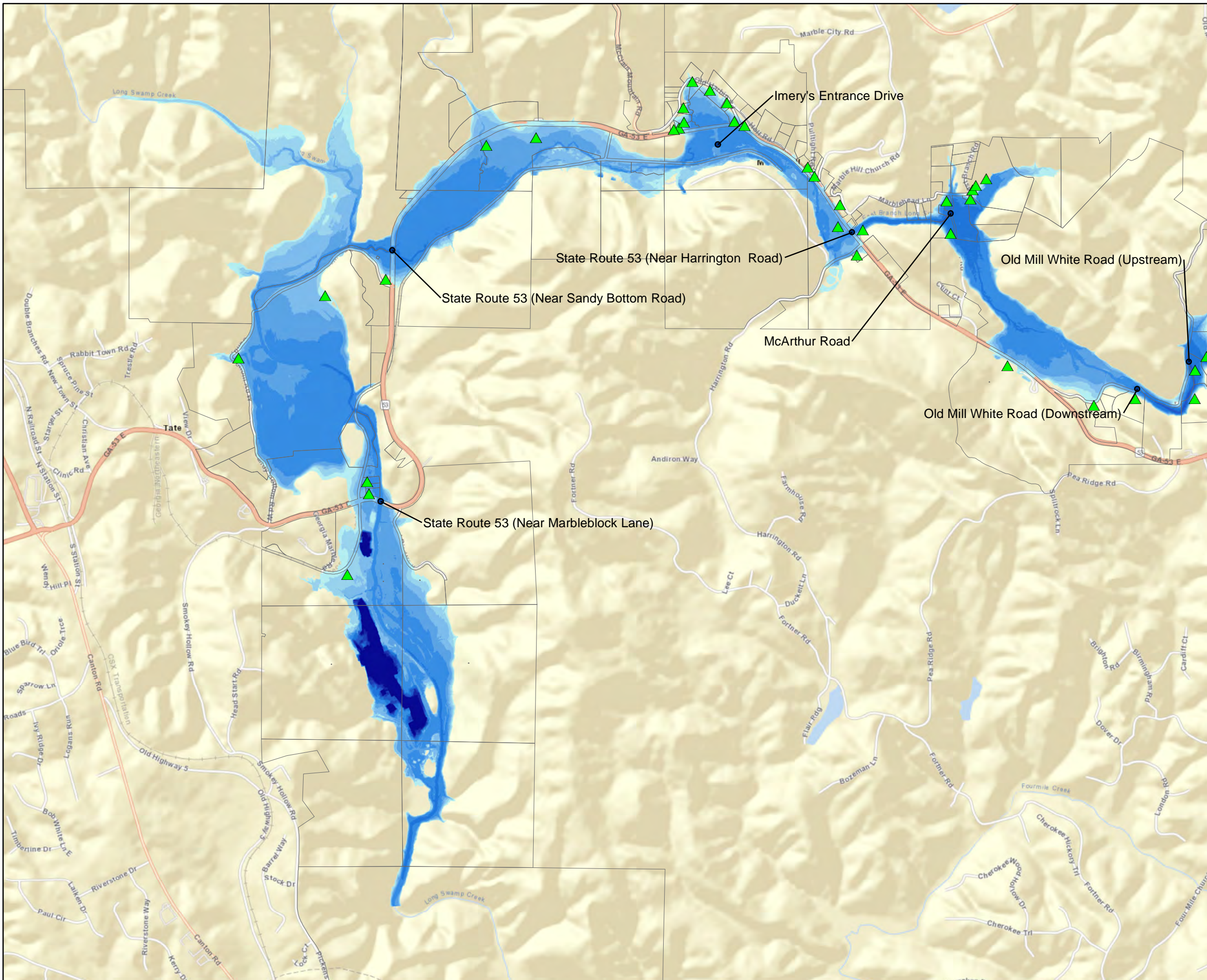
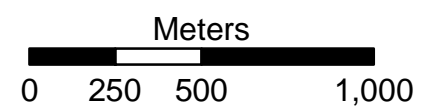
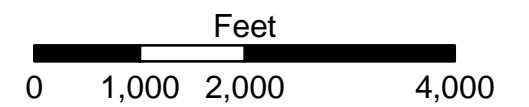
*NOTE: This evacuation map displays the inundated structures and parcels downstream of Big Canoe (i.e., downstream of Figure 1).*

*The list of inundated structure were provided by Pickens County and included structures within a 100 ft buffer outside of the extent of the mapped inundation zone.*

*The method used to develop inundation zones are approximate. Actual areas inundated will depend on actual failure and pre-failure hydrologic conditions and may differ significantly from information shown on maps.*

**Geosyntec**  
consultants

04-Oct-2022

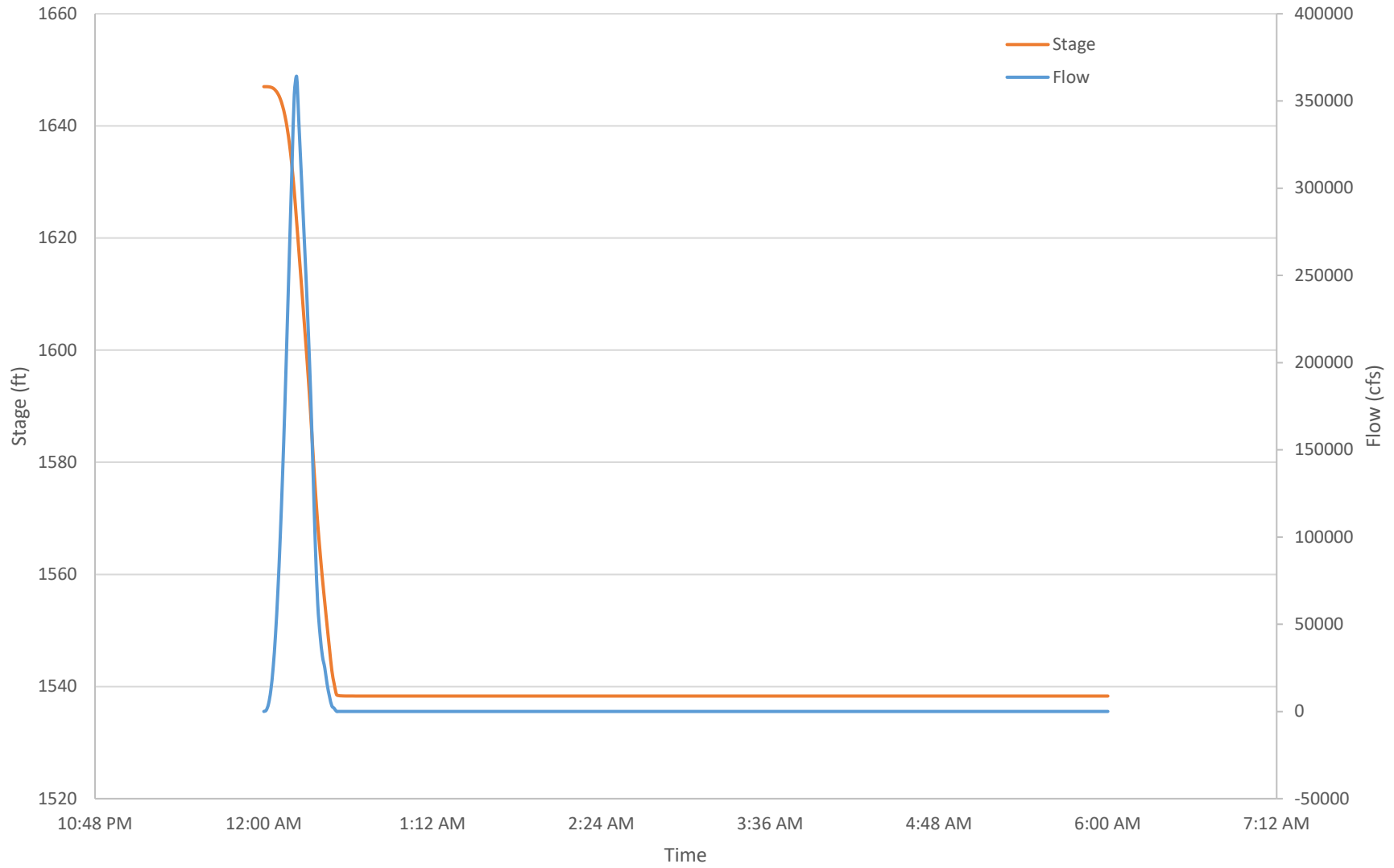


**ATTACHMENT J**

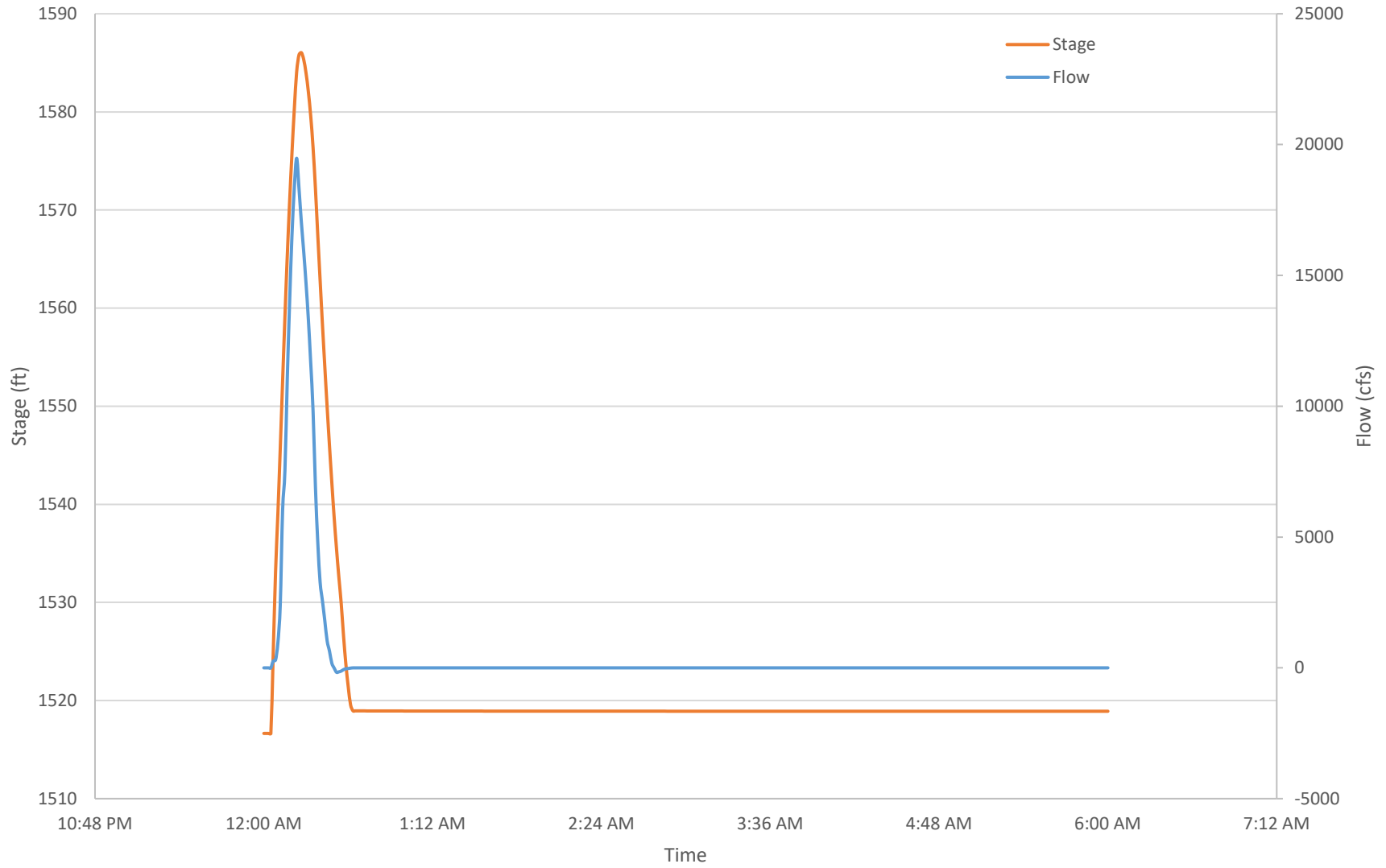
**HYDROGRAPHS**



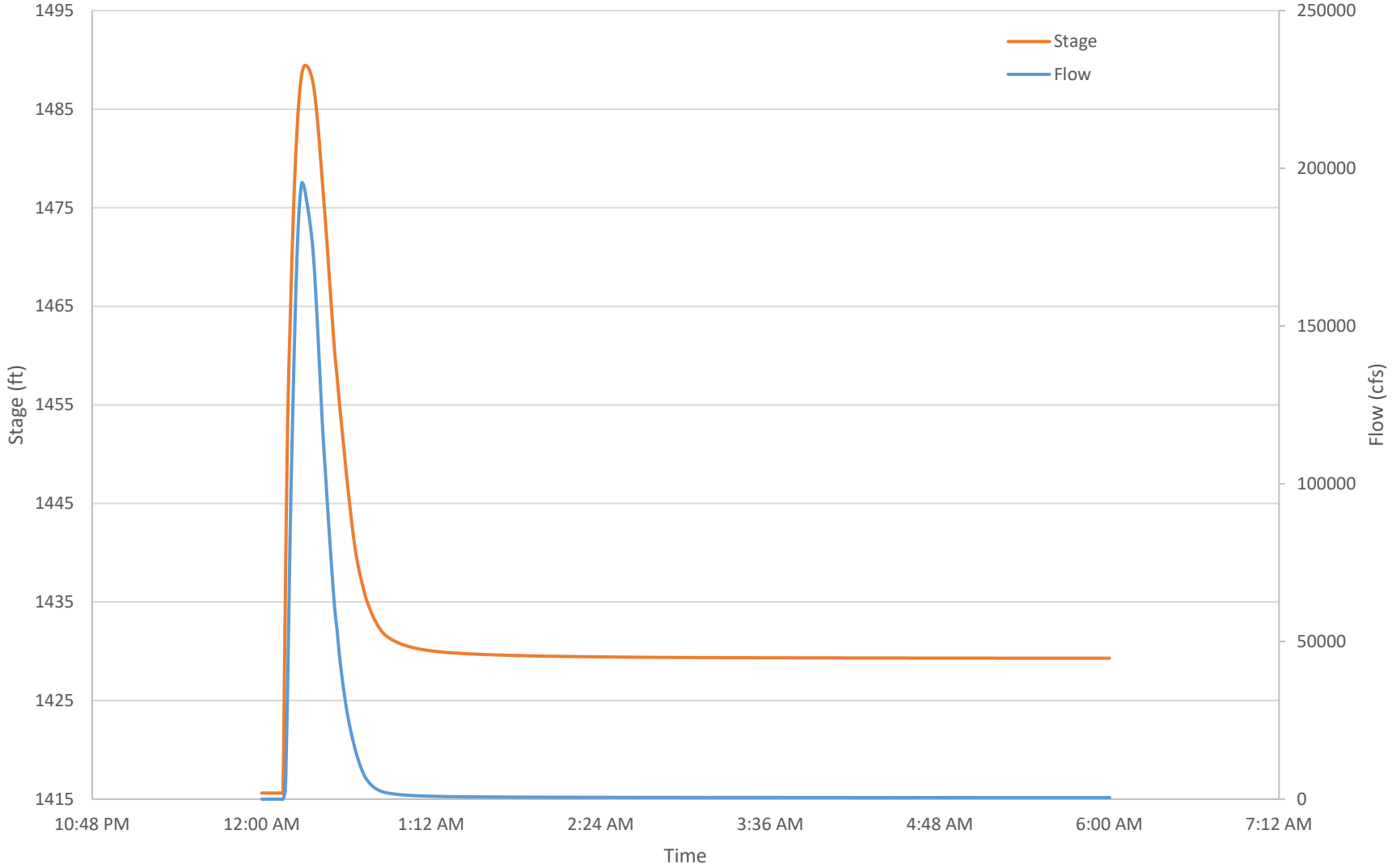
# Lake Petit Dam



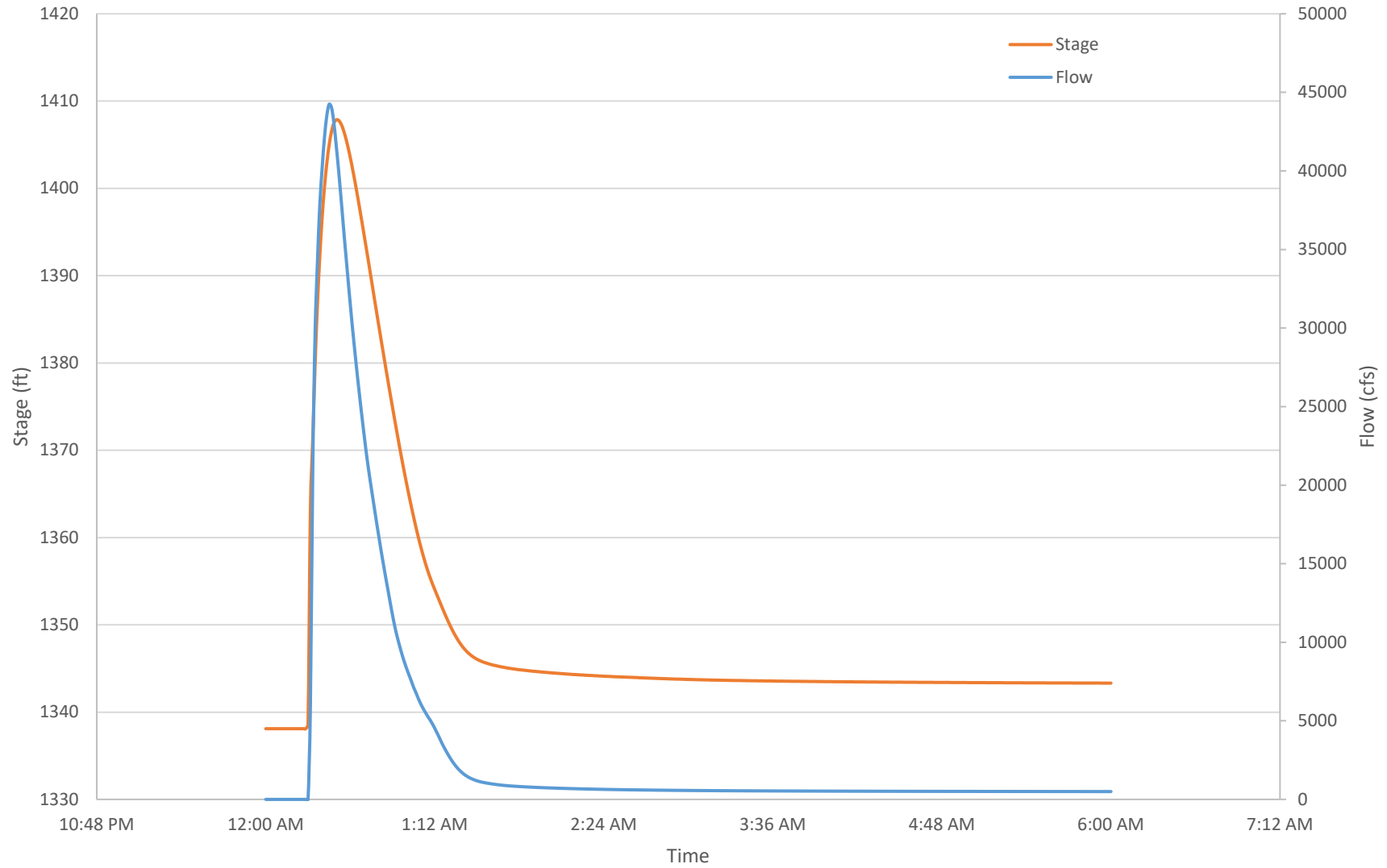
# Wolfscratch Drive



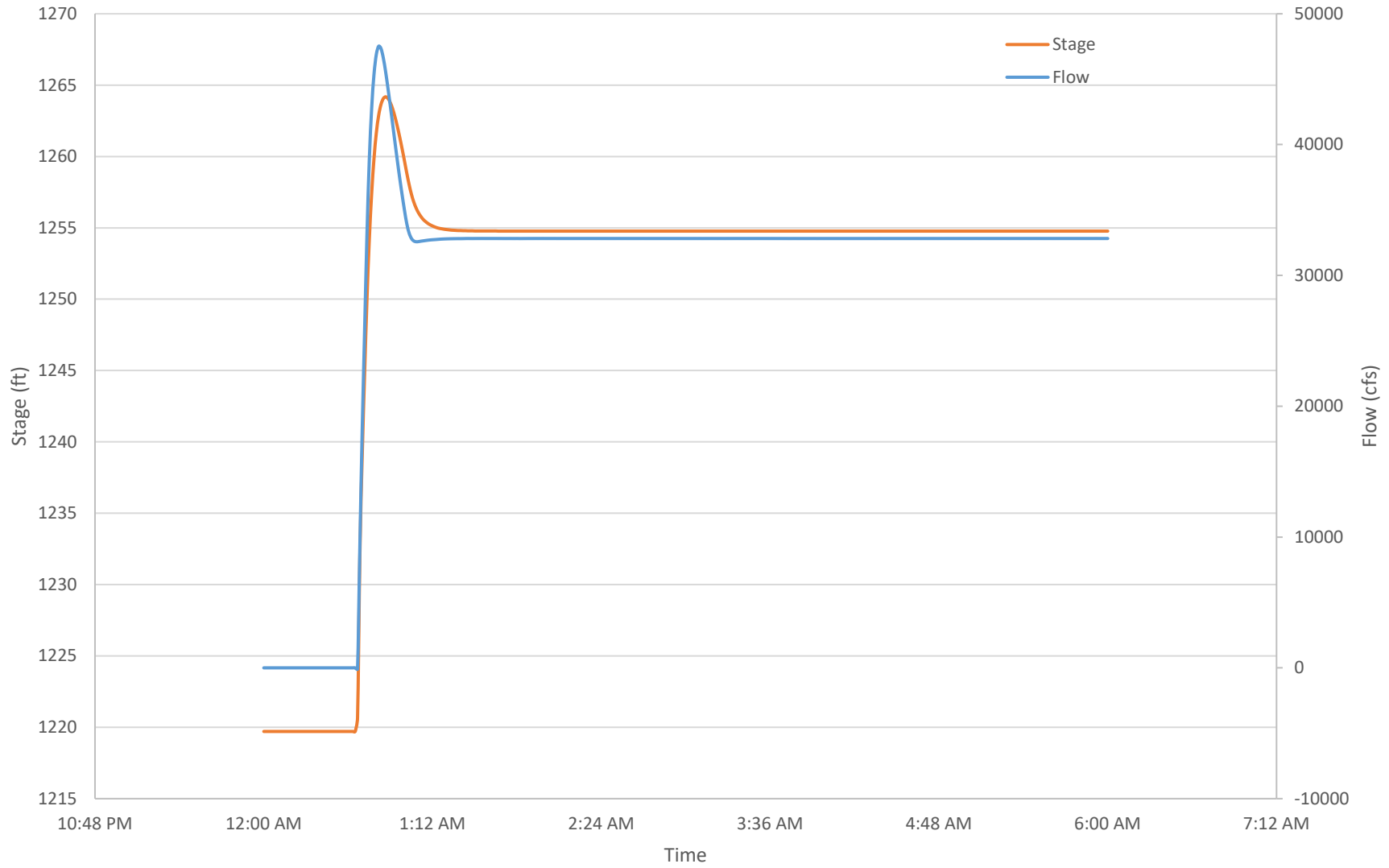
### Golfcourse Road



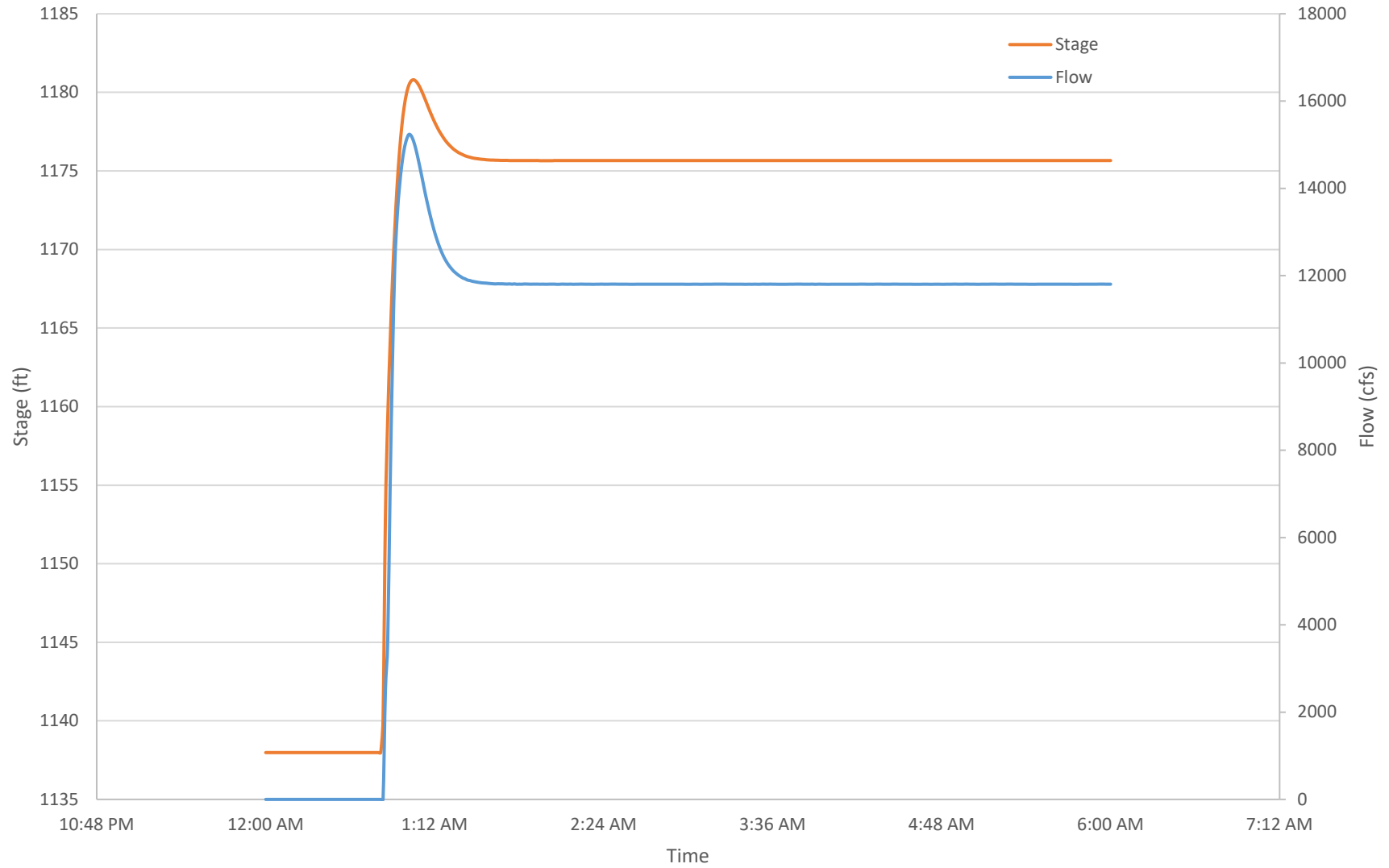
# Wilderness Parkway



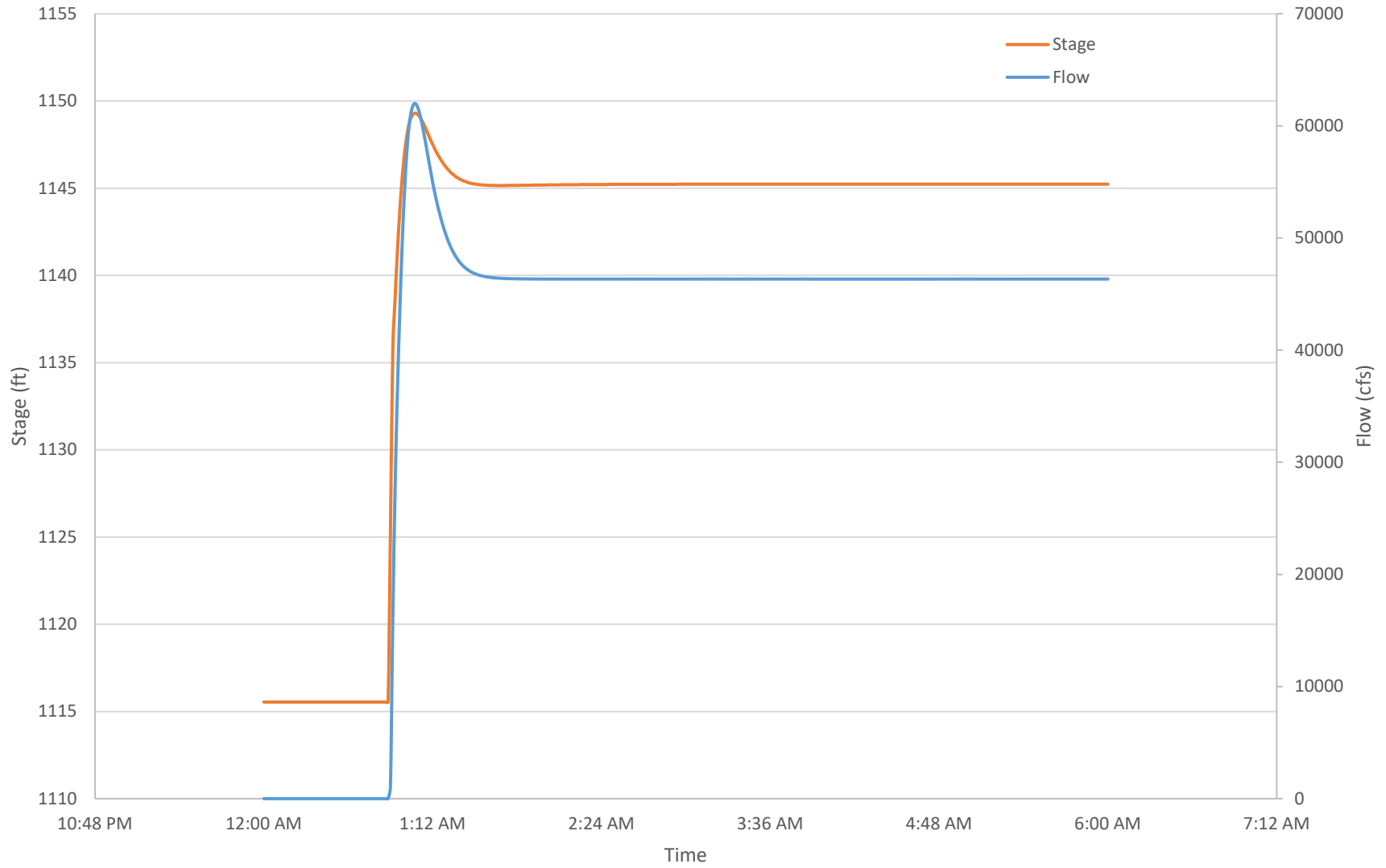
# Pendley Woods



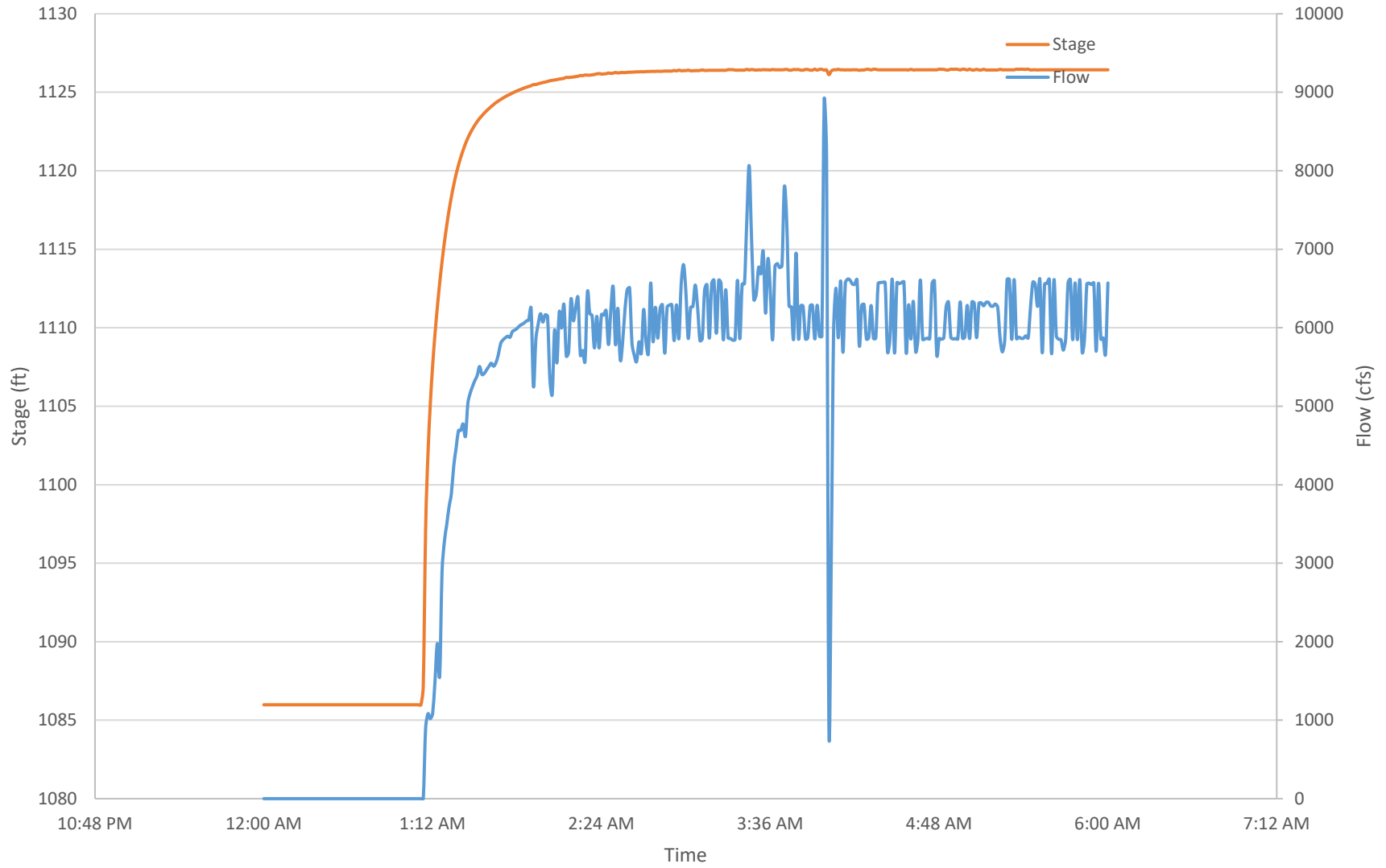
# Old Mill Upstream



# Old Mill Downstream

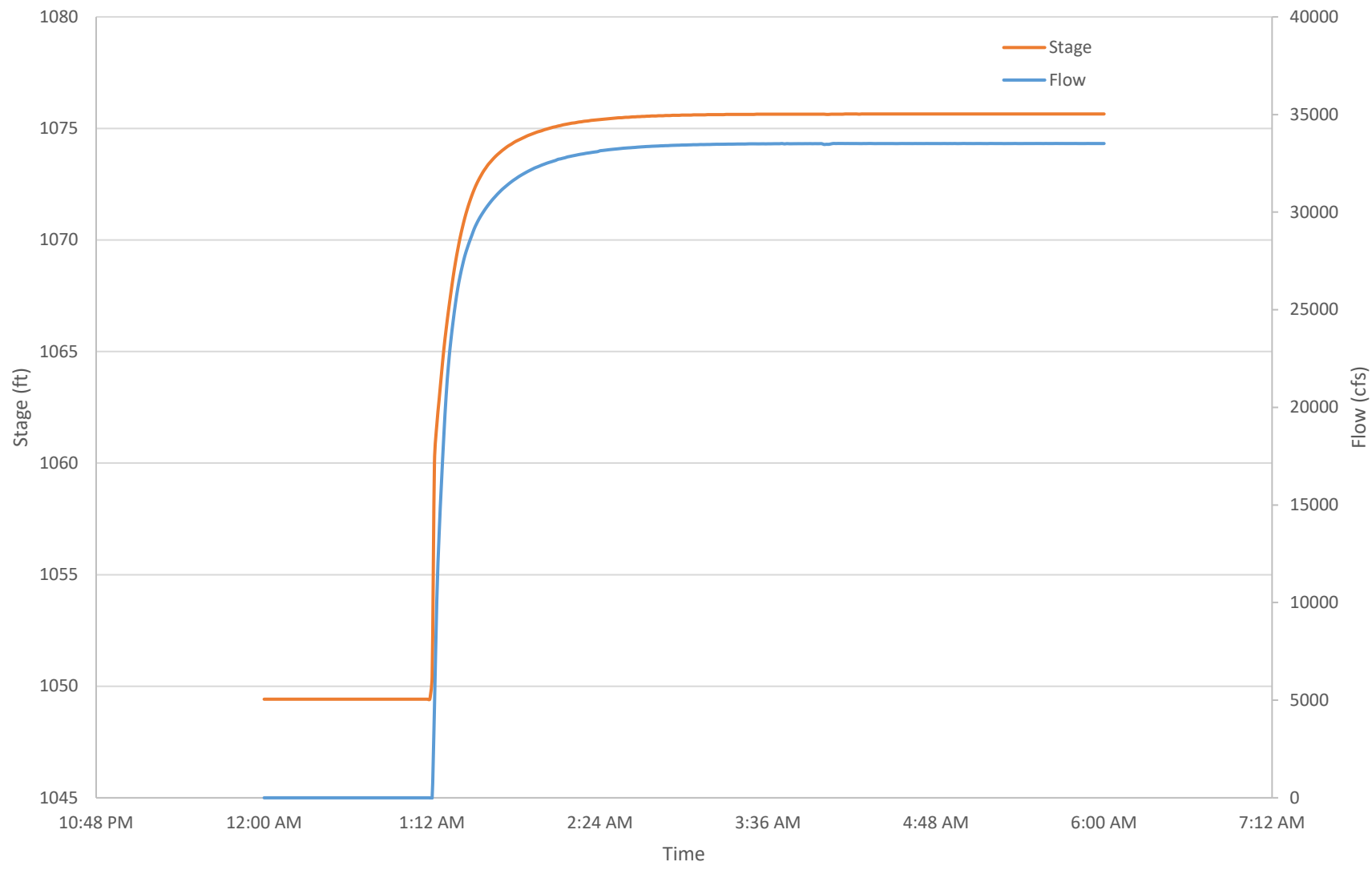


# McArthur Road

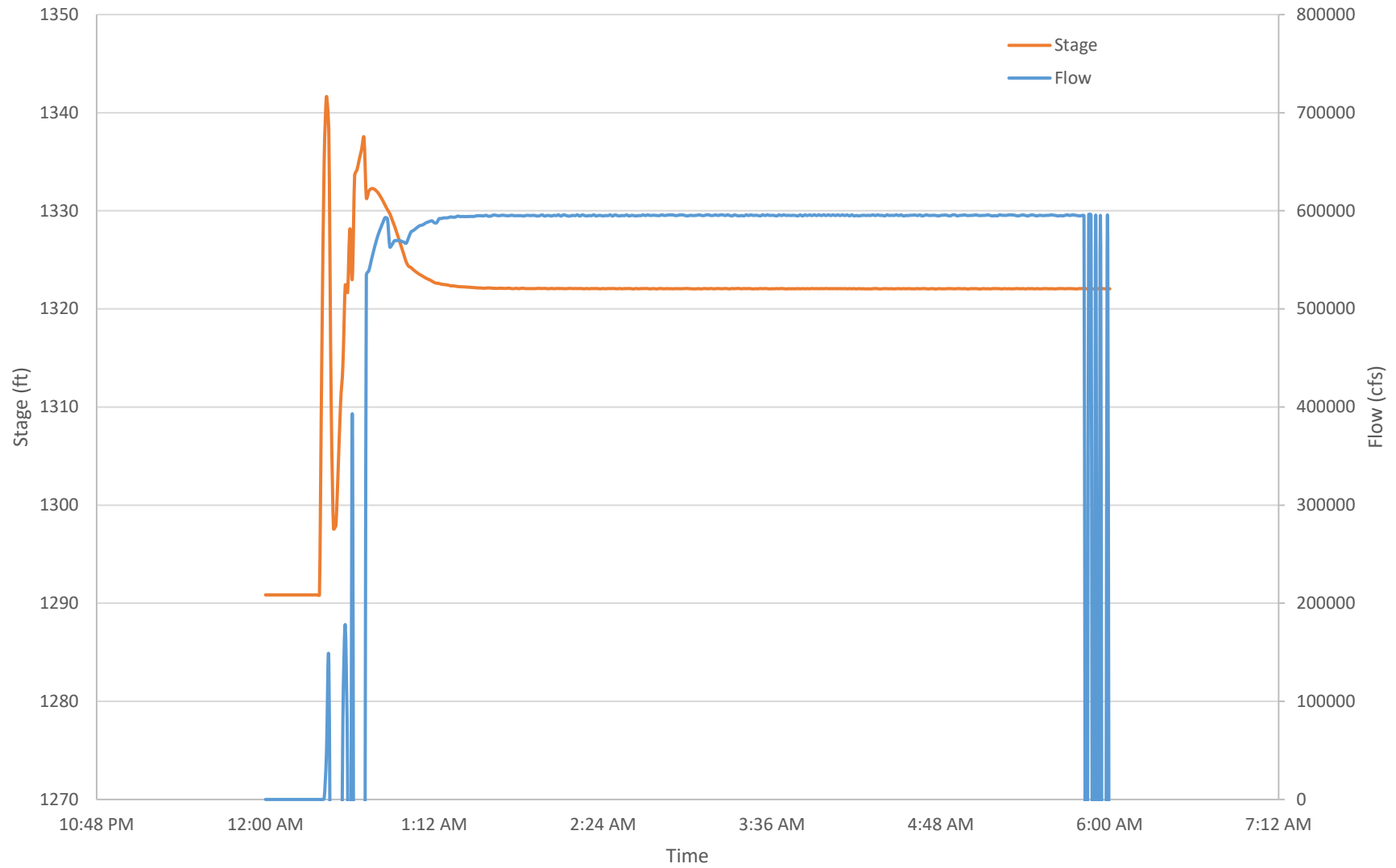




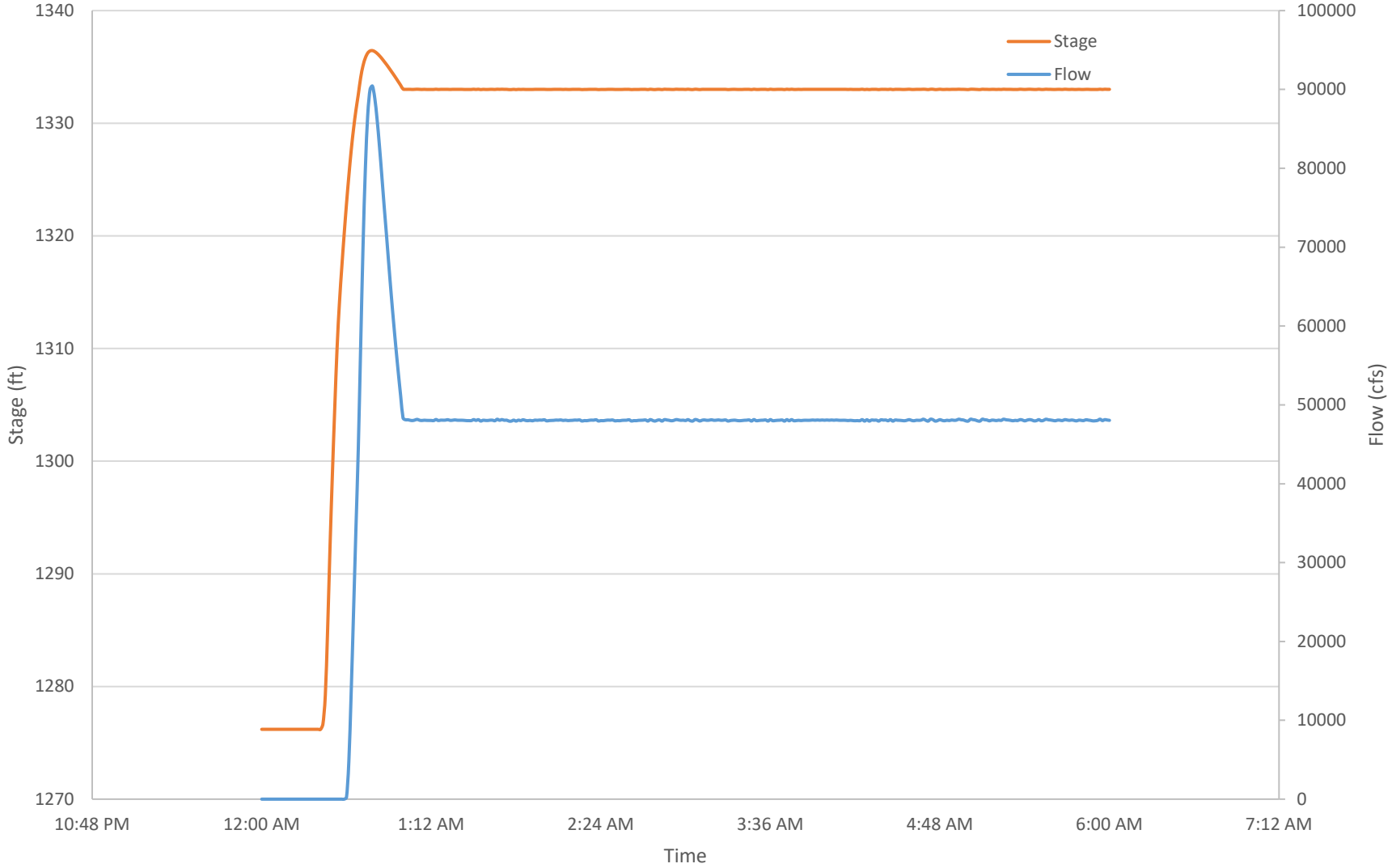
# Harrington Road



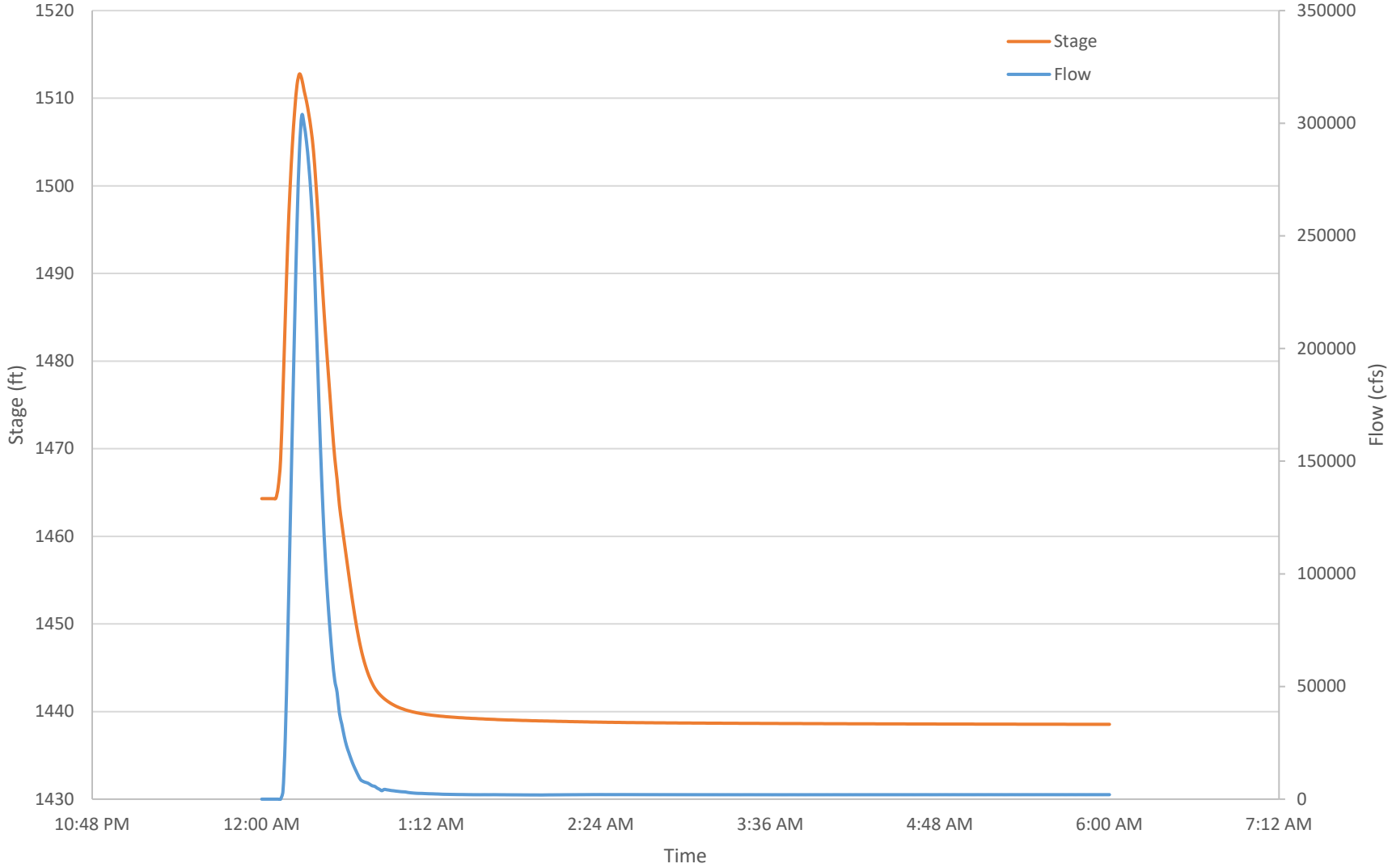
# Cove Road



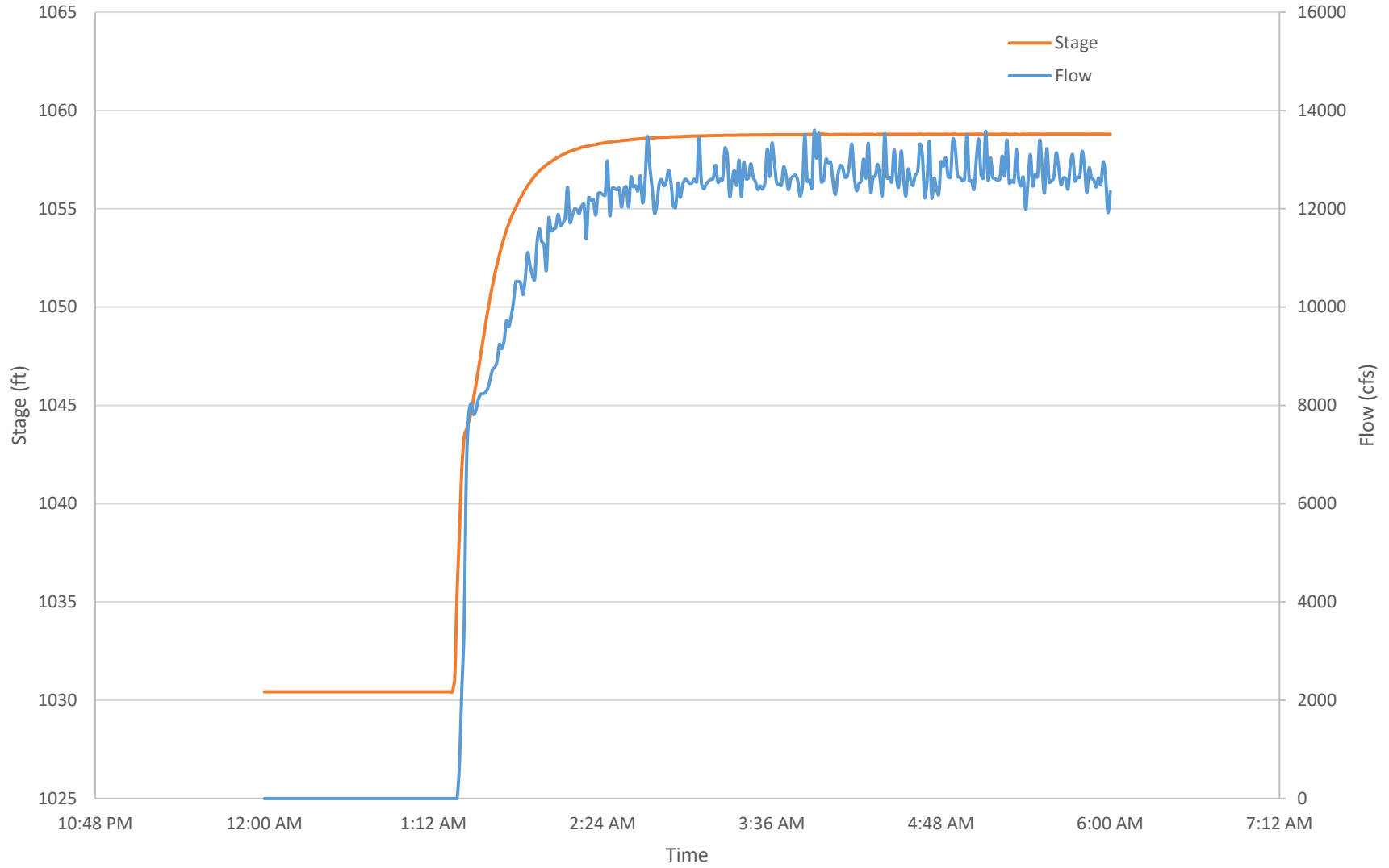
# Lake Cox Dam



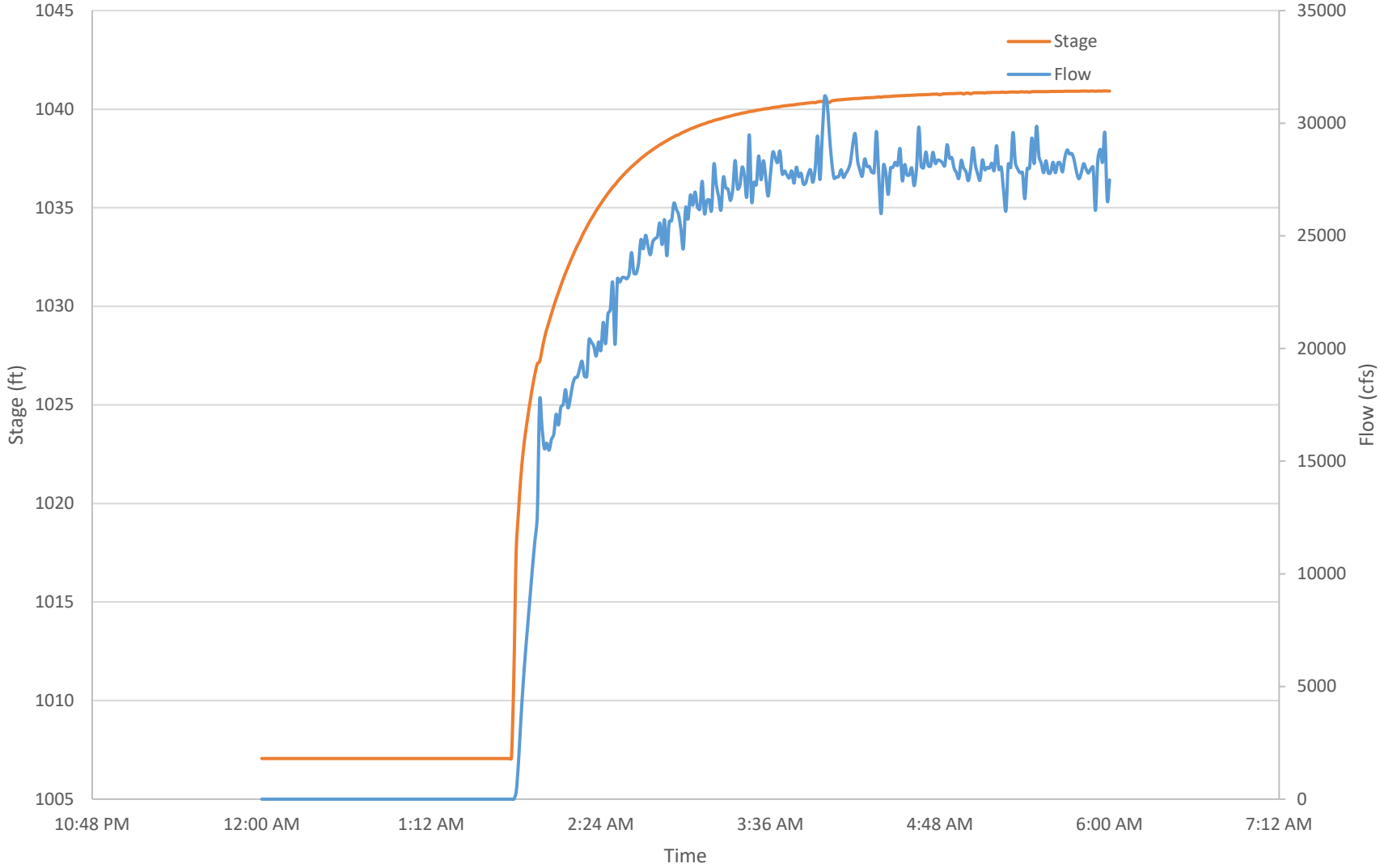
# Lake Sconti Dam



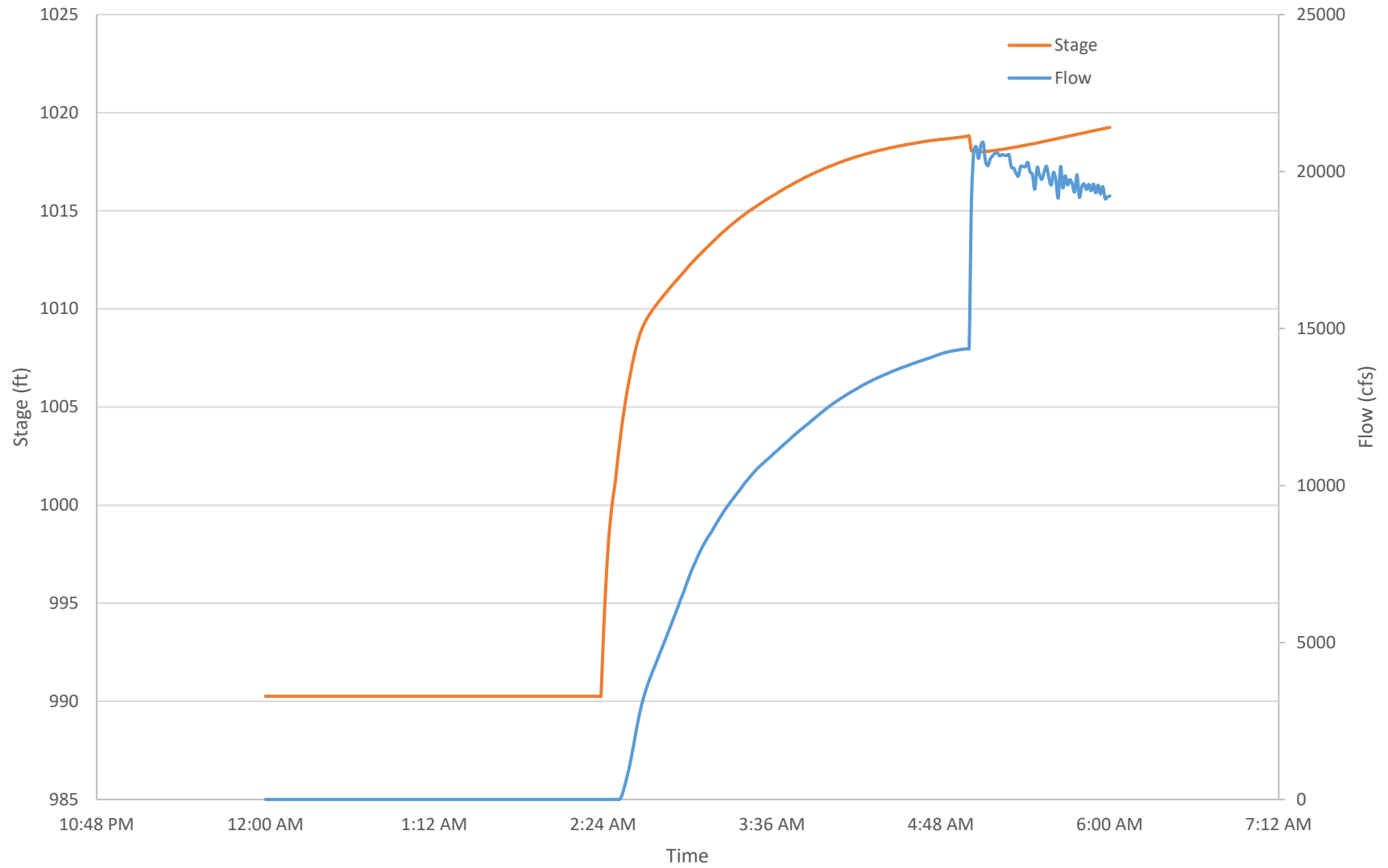
### Imery Entrance Drive



### State Route 53 - Near Sandy Bottom Road



### State Route 53 - Near Marbleblock Lane



**ATTACHMENT K**

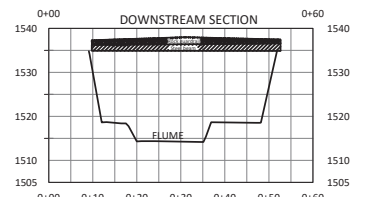
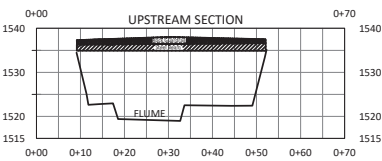
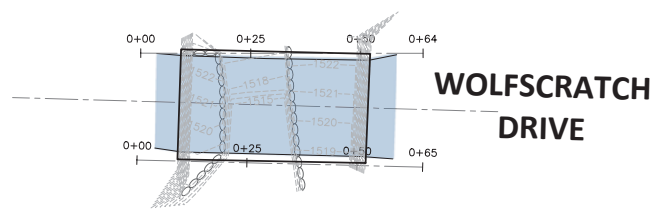
**PHOTOGRAPHS OF THE DAM AND POINTS OF INTEREST**



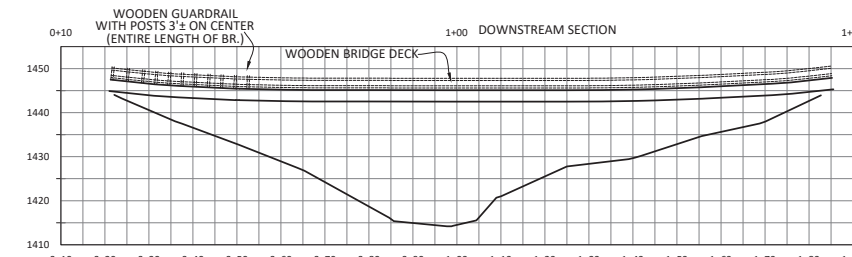
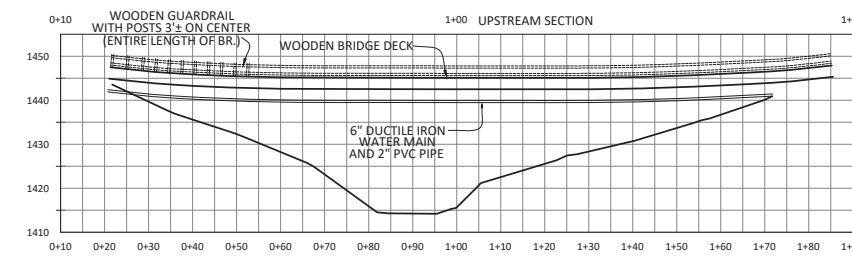
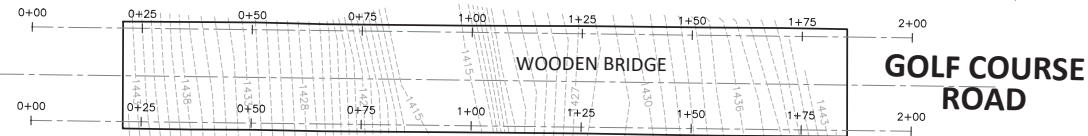


### SITE 1 WOLFSCRATCH DRIVE

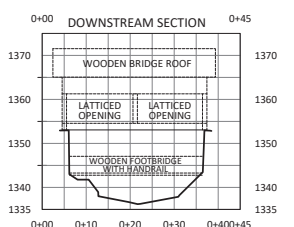
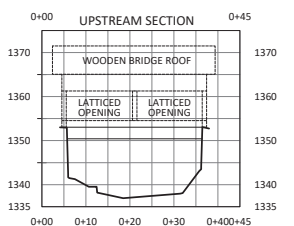
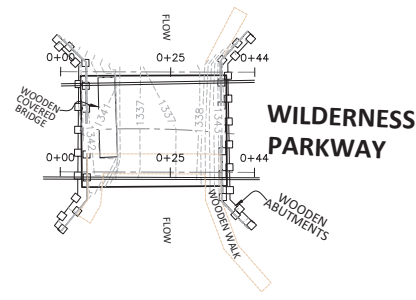
PETTIT CREEK



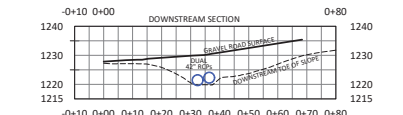
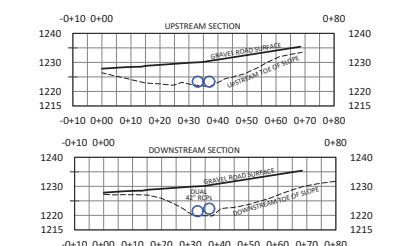
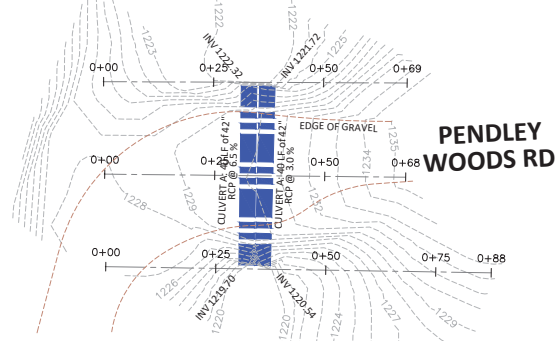
### SITE 5 GOLF COURSE ROAD



### SITE 6 WILDERNESS PARKWAY



### SITE 10 PENDLEY WOODS ROAD



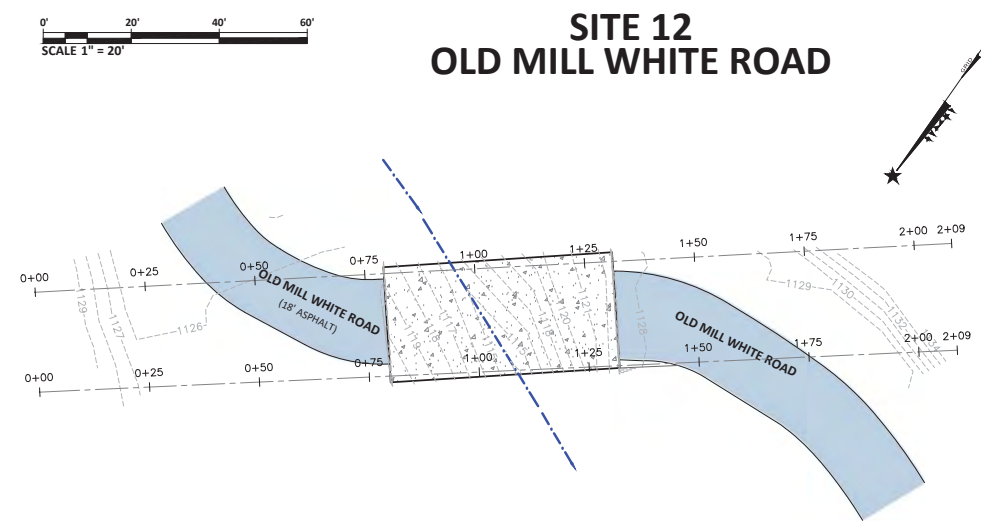
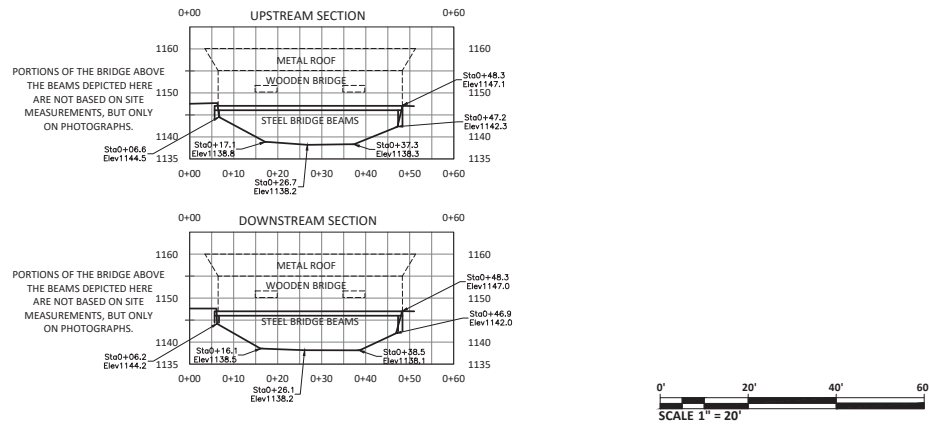
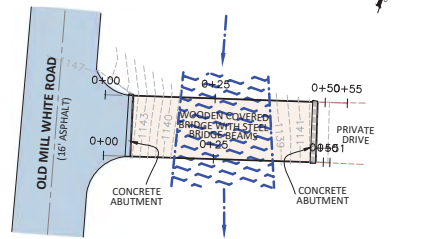
- SITES:**
- 1 - WILDERNESS PARKWAY
  - 5 - GOLF COURSE ROAD
  - 6 - WILDERNESS PARKWAY
  - 10 - PENDLEY WOODS ROAD

**Big Canoe Stream Sections**  
Pickens County, Georgia

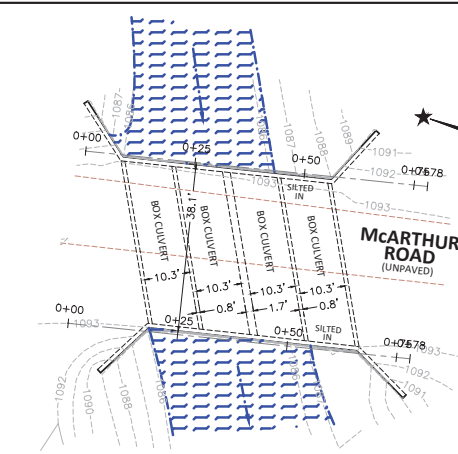
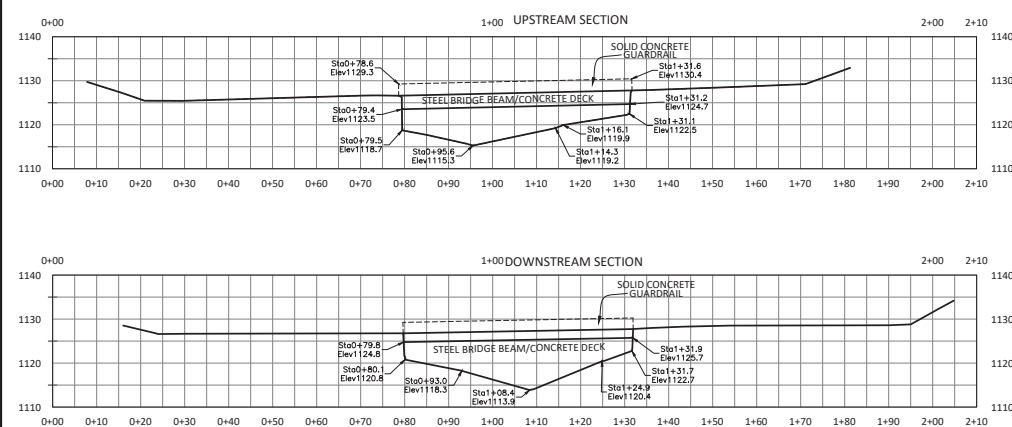
| Rev | Revision Description | Date     |
|-----|----------------------|----------|
| 0   | Initial Issue        | XX/XX/XX |



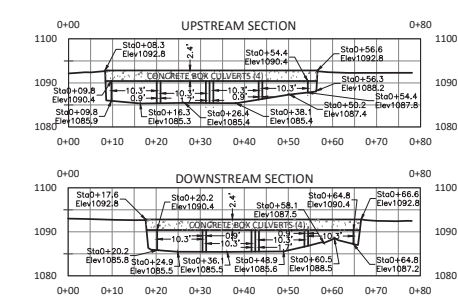
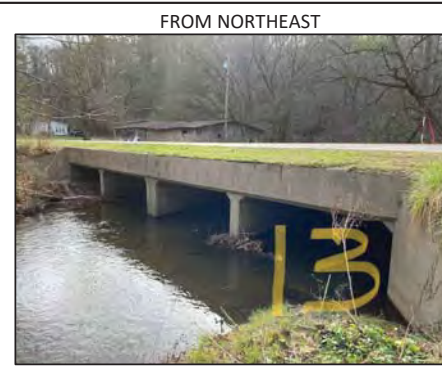
**SITE 11  
OLD MILL WHITE ROAD  
(JUSTICE WAY)**



**SITE 12  
OLD MILL WHITE ROAD**



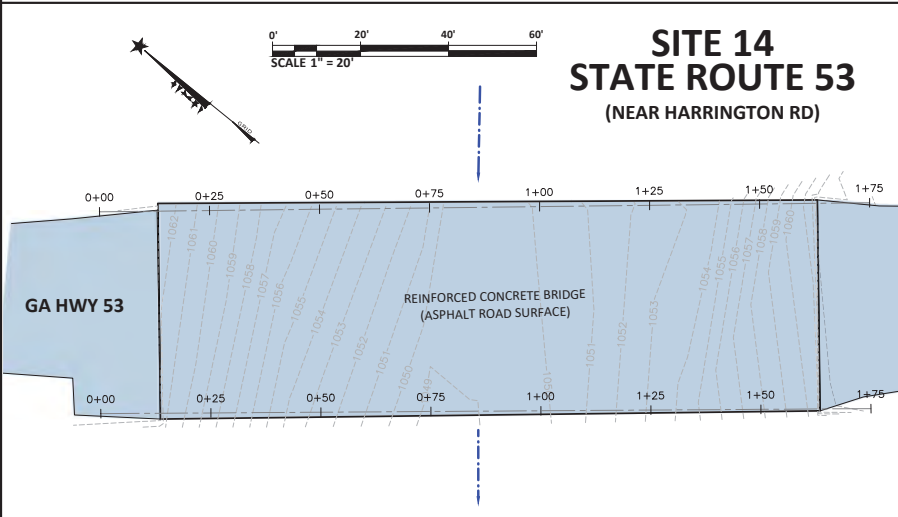
**SITE 13  
McARTHUR ROAD**



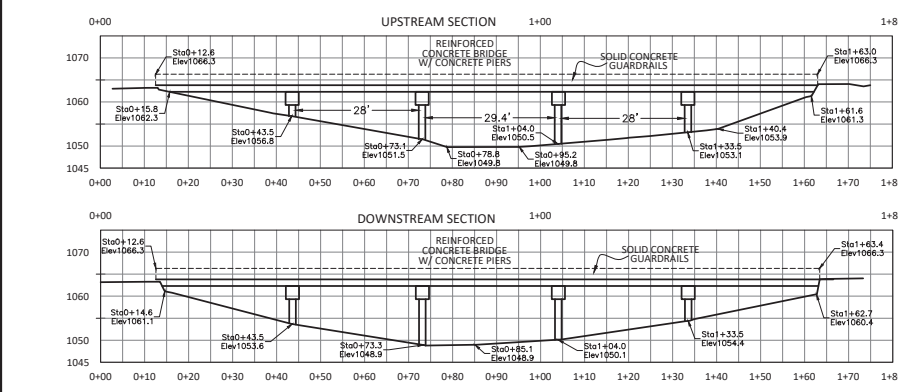
**FROM NORTHWEST**



**FROM NORTH**



**SITE 14  
STATE ROUTE 53  
(NEAR HARRINGTON RD)**



**FROM WEST**



**FROM EAST**

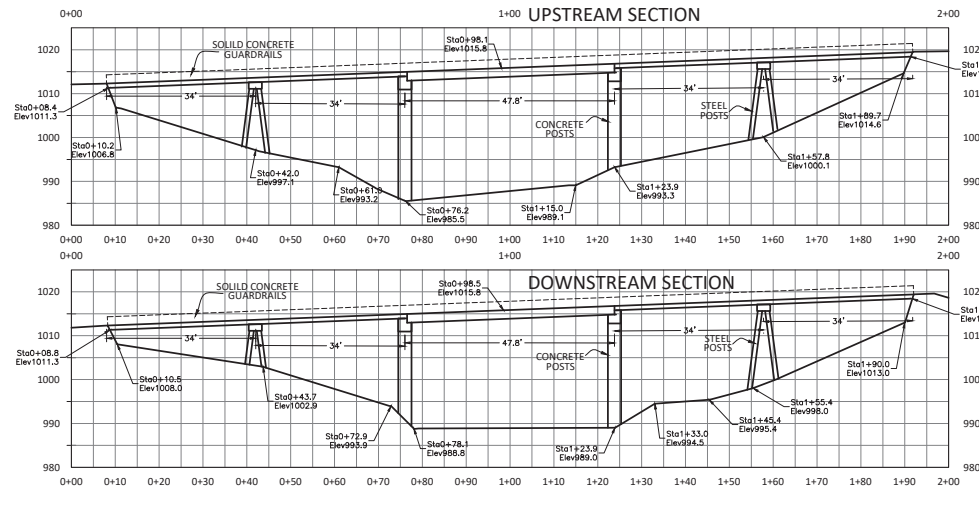
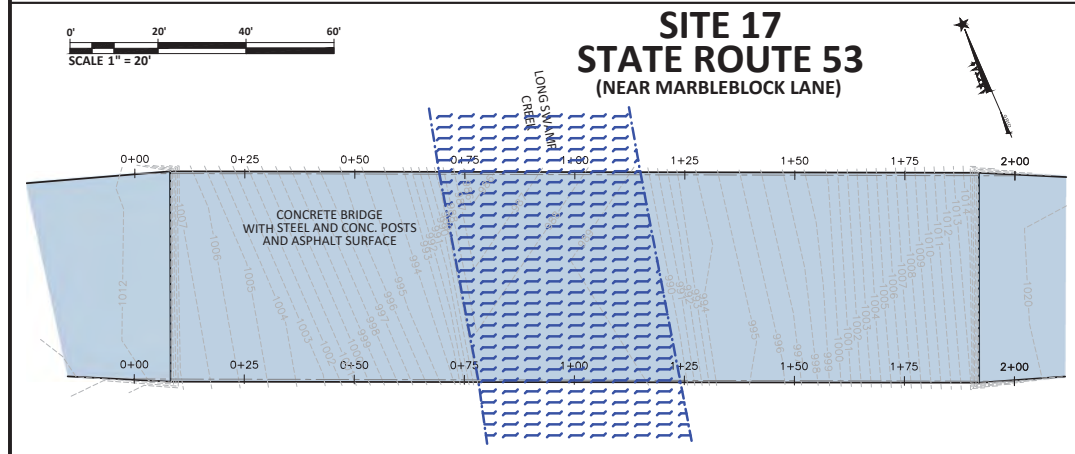
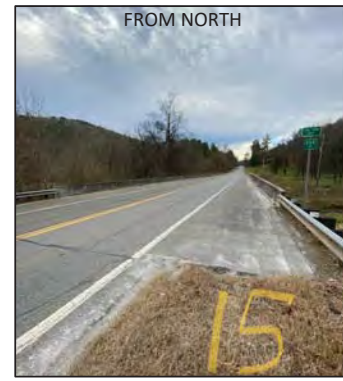
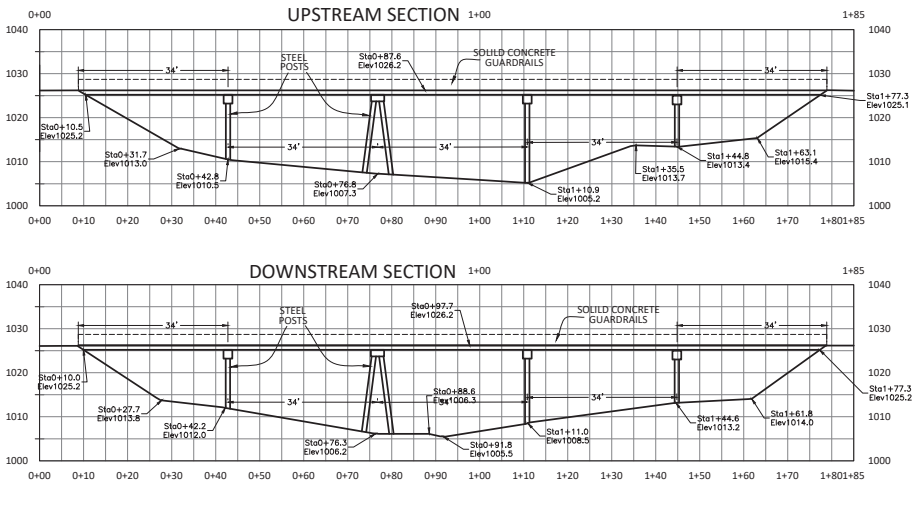
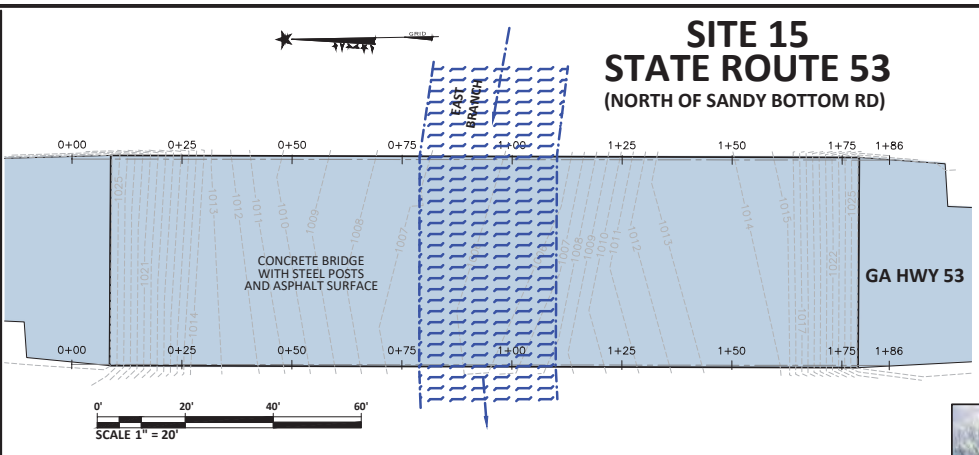
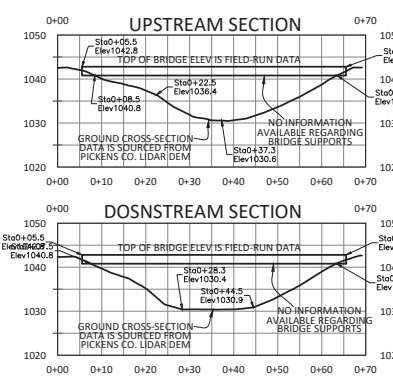
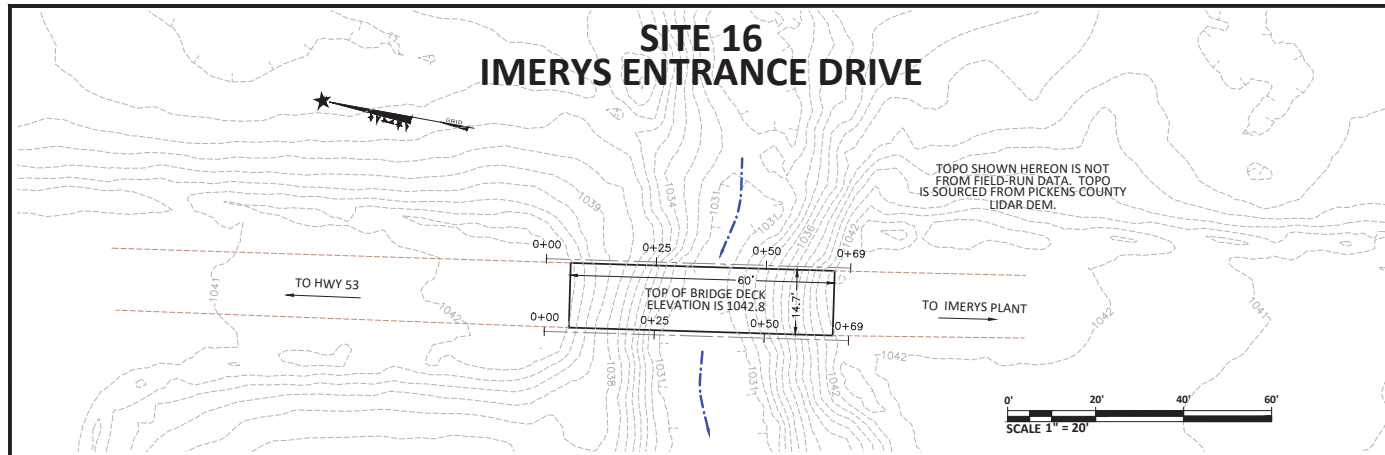


**SITES:**  
11 - OLD MILL WHITE ROAD  
12 - OLD MILL WHITE ROAD  
13 - McARTHUR ROAD  
14 - STATE ROUTE 53 (HARRINGTON RD)

**Big Canoe Stream Sections**  
Pickens County, Georgia

| Rev | Revision Description    | Date     |
|-----|-------------------------|----------|
| 0   | Initial Issue - Sheet 1 | 01/22/21 |

Sheet No.  
**2 of 3**



**SITES:**  
16 - IMERYS ENTRANCE DRIVE  
15 - STATE ROUTE 53  
17 - STATE ROUTE 53

**Big Canoe Stream Sections**  
Pickens County, Georgia

| Rev | Revision Description           | Date     |
|-----|--------------------------------|----------|
| 0   | Initial Issue - Sheet 1        | 01/22/21 |
| 1   | Initial Issue - Sheets 2 and 3 | 01/28/21 |

Sheet No.  
**3 of 3**

**ATTACHMENT L**

**AERIAL PHOTOS OF POINT OF INTERESTS**

Lake Petit Dam



Wolfscratch Drive



Lake Sconti Dam



Golf Course Road





Wilderness Parkway



Cove Road



Lake Cox Dam



Pendley Woods Road



Old Mill White Road (Upstream) – Left and Old Mill White Road (Downstream) - Right



McArthur Road



State Route 53 (Near Harrington Road)



Imerys Entrance Drive





State Route 53 (Near Sandy Bottom Road)



State Route 53 (Near Marbleblock Lane)



# APPENDIX D

## Definitions

## APPENDIX D – DEFINITIONS

The following definitions are frequently used when discussing the physical characteristic of the Dam.

|                                   |  |
|-----------------------------------|--|
| <b>ABUTMENT</b>                   | The part of the valley side against which the dam is constructed. Right and left abutments are those on respective sides as an observer when looking downstream.   |
| <b>ACRE-FOOT</b>                  | A unit of volumetric measure that would cover 1 acre to a depth of 1 foot. One acre-foot is equal to 43,560 cubic feet or 325,850 gallons.   |
| <b>APPURTENANCES</b>              | Structures around a dam that are necessary to the operation of the dam project (i.e., spillways).  |
| <b>BERM</b>                       | A nearly horizontal step (bench) in the upstream or downstream sloping face of the dam.  |
| <b>BOIL</b>                       | A disruption of the soil surface due to water discharging from below the surface. Eroded soil may be deposited in the form of a ring around the disruption.  |
| <b>BREACH</b>                     | An opening through the dam resulting in partial or total failure of the dam.   |
| <b>CATEGORY I DAM</b>             | A dam that is either 25 feet tall or impounds 100 acre-feet of water and where improper operation or dam failure would result in the probable loss of human life.  |
| <b>COMPREHENSIVE EAP EXERCISE</b> | An in-depth exercise of an EAP that involves the interaction of the dam owner with the state and local emergency management agencies in a stressful environment with time constraints. Functional and full-scale EAP exercises are considered comprehensive EAP exercises. |
| <b>CONDUIT</b>                    | A closed channel (round pipe or rectangular box) that conveys water through, around, or under the dam.   |
| <b>CONTROL SECTION</b>            | A usually level segment in the profile of an open channel spillway above which water in the reservoir discharges through the spillway.   |
| <b>CROSS SECTION</b>              | A slice through the dam showing elevation vertically and direction of natural water flow horizontally. Also, a slice through a spillway showing elevation vertically and left and right sides of the spillway looking downstream.  |
| <b>DAM</b>                        | An artificial barrier generally constructed across a watercourse for the purpose of impounding or diverting water.   |

|   |  |
|---|--|
| <b>DAM FAILURE</b>                          | Catastrophic type of failure characterized by the sudden, rapid, and uncontrolled release of impounded water. It is recognized that there are lesser degrees of failure and that any malfunction or abnormality outside the design assumptions and parameters which adversely affect a dam's primary function of impounding water is properly considered a failure. Such lesser degrees of failure can progressively lead to or heighten the risk of a catastrophic failure. They are, however, normally amendable to corrective action. |
| <b>DAM OPERATOR</b>                         | The person(s) or unit(s) of government with responsibility for the operation and maintenance of dam.   |
| <b>DRAIN, TOE, FOUNDATION, OR BLANKET</b>   | A water collection system of sand and gravel and typically pipes along the downstream portion of the dam to collect seepage and convey it to a safe outlet.  |
| <b>DRAINAGE AREA (WATERSHED)</b>            | The geographic area on which rainfall flows into the dam.  |
| <b>DRAWDOWN</b>                             | The lowering or releasing of the water level in a reservoir over time or the volume lowered or released over a particular period of time.  |
| <b>EMBANKMENT</b>                           | Fill material, usually earth or rock, placed with sloping sides.   |
| <b>EMERGENCY</b>                            | A condition that develops unexpectedly, endangers the structural integrity of the dam and/or downstream human life and property, and requires immediate action.  |
| <b>EMERGENCY ACTION PLAN (EAP) EXERCISE</b> | An activity designed to promote emergency preparedness; test or evaluate emergency action plans, procedures or facilities; train personnel in emergency management duties; and demonstrate operational capability. Exercises consist to the performance of duties, tasks or operations very similar to the way they would be performed in a real emergency. However, the exercise performance is in response to a simulated event.   |
| <b>EMERGENCY MANAGEMENT AGENCY</b>          | The State and local agencies responsible for emergency operations, planning, mitigation, preparedness, response, and recovery for all hazards.   |
| <b>EMERGENCY OPERATIONS CENTER</b>          | The location or facility where responsible officials gather during an emergency to direct and coordinate emergency operations, to communicate with other jurisdictions and   |

|                              |  |
|------------------------------|--|
|                              | with field emergency forces, and to formulate protective action decisions and recommendations during an emergency.   |
| <b>EVACUATION MAP</b>        | A map showing the geographic area downstream of a dam that should be evacuated if it is threatened to be flooded by a breach of the dam or other large discharge.  |
| <b>FILTER</b>                | The layers of sand and gravel in a drain that allow seepage through an embankment to discharge into the drain without eroding the embankment soil.   |
| <b>FLOOD ROUTING</b>         | A process of determining progressively over time the amplitude of a flood wave as it moves past a dam or downstream to successive points along a river or stream.  |
| <b>FREEBOARD</b>             | Vertical distance between a stated water level in the reservoir and the top of dam.  |
| <b>GATE, SLIDE, OR SLUID</b> | An operable, watertight valve to manage the discharge of water from the dam.   |
| <b>GROIND</b>                | The area along the intersection of the face of a dam and the abutment.   |
| <b>HAZARD</b>                | A situation which creates the potential for adverse consequences such as loss of life, property damage or other adverse impacts. Impacts may be for a defined area downstream of a dam from floodwaters released through spillways and outlet works of the dam or waters released by partial or complete failure of the dam. |
| <b>HEADWATER</b>             | The water immediately upstream from a dam. The water surface elevation varies due to fluctuations in inflow and the amount of water passed through the dam.  |
| <b>HEIGHT</b>                | The vertical distance between the lowest point along the crest of the dam and the lowest point at the downstream toe, which usually occurs in the bed of the outlet channel.   |
| <b>INSTRUMENTATION</b>       | An arrangement of devices installed into or near dams that provide measurements to evaluate the structural behavior and other performance parameters of the dam and appurtenant structures.  |
| <b>INUNDATION MAP</b>        | A map delineating areas that would be flooded as a result of a dam failure or other unusually large spillway release.  |
| <b>NOTIFICATION</b>          | To immediately inform appropriate individuals, organizations, or agencies about a potentially emergency situation so they can initiate appropriate actions.  |
| <b>OUTLET WORKS</b>          | An appurtenant structure that provides for controlled  |

|  |   |
|--|---|
|  | passage of normal water flows through the dam.  |
| <b>OVERTOP</b>   | Flow of an embankment dam beyond its spillway capacity and over the top of the dam crest, or containment elevation.   |
| <b>PROBABLE MAXIMUM PRECIPITATION (PMP) OR FLOOD (PMF)</b> | The theoretically greatest precipitation or resulting flood that is meteorologically feasible for a given duration over a specific drainage area at a particular geographical location. |
| <b>PIPING</b>  | The progressive destruction of an embankment or embankment foundation by internal erosion of the soil by seepage flows.   |
| <b>PROJECT DESIGN FLOOD</b>                                | The maximum rate of rainfall in which the dam could safely pass or store without overtopping.   |
| <b>RESERVOIR</b>   | The body of water impounded or potentially impounded by the dam.  |
| <b>RIP RAP</b>   | A layer of large rock, precast blocks, or other suitable material, generally placed on an embankment or along a watercourse as protection against wave action, erosion, or scour.       |
| <b>RISK</b>  | A measure of the likelihood and severity of an adverse consequence.   |
| <b>SEEPAGE</b>   | The continuous movement of water from the upstream face of the dam toward its downstream face.  |
| <b>SETTLEMENT</b>  | The downward movement of the ground due to forces (i.e., buildings and other structures) applied to the surface.  |
| <b>SPILLWAY</b>  | A structure over or through which flood flows are discharged.   |
| <b>TAILWATER</b>   | The water downstream from the dam.  |
| <b>TOE OF DAM</b>  | The junction of the upstream or downstream face of an embankment with the ground surface.   |

## **APPENDIX E**

**E-1: Contact Checklist**

**E-2: Condition B (Level 2) or C (Level 3) Event  
Log**

**E-3: Dam Emergency Situation Report Forms**



## APPENDIX E-1 – CONTACT CHECKLIST

Lake Petit Dam, Pickens County, Georgia

Date: \_\_\_\_\_

The following contacts should be made immediately after the Emergency Condition (Level) is determined. The person making the contacts should initial and record the time of the call and who was notified for each contact made. See the Notification Flowcharts for critical contact information and Emergency Services Contacts for contact information for other possible emergency services.

### Emergency Condition A (Level 1)

|                                      | Person Contacted | Time Contacted | Contacted by |
|--------------------------------------|------------------|----------------|--------------|
| ___ Dam Owner's Representative       | _____            | _____          | _____        |
| ___ Owner's Technical Representative | _____            | _____          | _____        |
| ___ GA Safe Dams Program             | _____            | _____          | _____        |

### Emergency Condition B (Level 2)

|                                      | Person Contacted | Time Contacted | Contacted by |
|--------------------------------------|------------------|----------------|--------------|
| ___ Dam Owner's Representative       | _____            | _____          | _____        |
| ___ Owner's Technical Representative | _____            | _____          | _____        |
| ___ GA Safe Dams Program             | _____            | _____          | _____        |
| ___ Pickens County EMA               | _____            | _____          | _____        |

### Emergency Condition C (Level 3)

|                                      | Person Contacted | Time Contacted | Contacted by |
|--------------------------------------|------------------|----------------|--------------|
| ___ 911                              | _____            | _____          | _____        |
| ___ GA Safe Dams Program             | _____            | _____          | _____        |
| ___ Dam Owner's Representative       | _____            | _____          | _____        |
| ___ Owner's Technical Representative | _____            | _____          | _____        |
| ___ Pickens County EMA               | _____            | _____          | _____        |

# APPENDIX E-2 – CONDITION B (LEVEL 2 ) OR C (LEVEL 3) EVENT LOG

(To be Completed During the Emergency)

Lake Petit Dam, Pickens County, Georgia

Date: \_\_\_\_\_

Water Level Elevation: \_\_\_\_\_ Freeboard: \_\_\_\_\_

When and how was the event detected?

---

---

Weather conditions:

---

---

General description of the emergency situation:

---

---

Emergency level determination: \_\_\_\_\_ Made by: \_\_\_\_\_

## Actions and Event Progression

| Date | Time | Action/Event Progression | Taken By |
|------|------|--------------------------|----------|
|      |      |                          |          |

Report prepared by: \_\_\_\_\_

## APPENDIX E-3 – DAM EMERGENCY SITUATION REPORT

(To be Completed Following the Termination of the Emergency)

Lake Petit Dam, Pickens County, Georgia

Date: \_\_\_\_\_

National Inventory of Dams (NID) No.: GA00685

Weather conditions: \_\_\_\_\_

General description of emergency situation:

\_\_\_\_\_  
\_\_\_\_\_

Area(s) of dam affected:

\_\_\_\_\_  
\_\_\_\_\_

Extent of dam damage: \_\_\_\_\_

Possible cause(s): \_\_\_\_\_

Effect on dam's operation: \_\_\_\_\_

Initial reservoir elevation: \_\_\_\_\_ Time: \_\_\_\_\_

Maximum reservoir elevation: \_\_\_\_\_ Time: \_\_\_\_\_

Final reservoir elevation: \_\_\_\_\_ Time: \_\_\_\_\_

Description of area flooded downstream/damages/injuries/loss of life:

\_\_\_\_\_  
\_\_\_\_\_

Other data and comments:

\_\_\_\_\_  
\_\_\_\_\_

Observer's name and telephone number: \_\_\_\_\_

Report prepared by: \_\_\_\_\_

# **APPENDIX F**

## **Locally Available Resources**

## APPENDIX F – LOCALLY AVAILABLE RESOURCES

The Pickens County Commissioner indicated the following heavy equipment may be available in the case of an emergency (for use, contact Mark Harris at Pickens County EMA at 706-253-8809 or Kirk Anderson at the Pickens County Public Works Department at 706-253-8873):

- One (1) Caterpillar D6NXL bulldozer
- One (1) Caterpillar 304C excavator
- One (1) Kobelco SK330 LC excavator
- One (1) Kubota KX057 excavator
- One (1) Kubota KX080 excavator
- Two (2) Kubota SVL 95-2S skid steers
- Three (3) Mack tandem dump trucks
- Six (6) single axle box dump trucks
- One (1) single axle flatbed trucks.

Other locally available resources include:

| Heavy Equipment Service and Rental  | Sand and Gravel Supply  | Ready-mix Concrete Supply  |
|---|---|--|
| <p>Sunbelt Rentals<br/>5290 Lake Pointe Center Drive<br/>Cumming, Georgia<br/>Main: 770-887-9966<br/>Emergency: 800-667-9328</p> <p>United Rentals<br/>1151 Northpoint Parkway SE<br/>Acworth, Georgia 30102<br/>Main: 770-974-3500<br/>Emergency: 844-873-4948</p> | <p>Martin Marietta<br/>Ball Ground Quarry<br/>970 Old Nelson Road<br/>Ball Ground, Georgia 30107<br/>Office: 770-735-4783</p> <p>Vulcan Materials Company<br/>4420 Hightower Road<br/>Ball Ground, Georgia 30107<br/>Main: 678-947-3310<br/>GA Services: 770-454-3691</p> | <p>Wayne Davis Concrete<br/>115 River Mill Drive<br/>Ball Ground, Georgia<br/>Main: 770-345-4454</p> <p>Ernst Concrete<br/>970 Old Nelson Road<br/>Ball Ground, Georgia 30107<br/>Main: 770-422-0103</p> <p>Argos Ready Mix<br/>829 Univeter Road<br/>Canton, Georgia<br/>770-428-7478</p> |

| Pumps   | Diving Service  | Sand Bags  |
|---|---|--|
| <p>Xylem Cartersville<br/>402 Old Mill Road<br/>Cartersville, Georgia 30120<br/>770-415-8814<br/>(On call service available 24/7)</p> <p>United Rentals<br/>5260 Truman Drive<br/>Decatur, Georgia 30035<br/>Main: 404-439-4322<br/>Emergency: 844-873-4948</p> <p>Rain for Rent<br/>2330 Burnt Wood Drive<br/>Kennesaw, Georgia 30152<br/>678-594-6601</p> | <p>Georgia Department of Natural Resources, Law Enforcement Division, Search and Rescue Team<br/>781 Red Top Mountain Road SE, Acworth, Georgia 30102<br/>770-529-2424 (M-F 8am – 4:30pm)<br/>1-800-241-4113 (After hours)</p> <p>Underwater Construction Corporation<br/>8494 Gulf View Drive<br/>Soddy Daisy, TN 37379<br/>423-332-6700</p> | <p>Hanes Geo Components<br/>3105 Sweetwater Road, Shite 200<br/>Lawrenceville, Georgia 30044<br/>Main: 866-961-3565<br/>Emergency: 678-221-7849</p> <p>SouthScape<br/>790 East Church Street<br/>(Hwy 53 Business)<br/>Jasper, Georgia 30143<br/>706-253-0033<br/>770-894-7400</p> |